Bending analysis of an imperfect FGM plates under hygro-thermo-mechanical loading with analytical validation

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(Received April 11, 2016, Revised June 20, 2016, Accepted June 21, 2016)

Abstract. Flexural bending analysis of perfect and imperfect functionally graded materials plates under hygro-thermo-mechanical loading are investigated in this present paper. Due to technical problems during FGM fabrication, porosities and micro-voids can be created inside FGM samples which may lead to the reduction in density and strength of materials. In this investigation, the FGM plates are assumed to have even and uneven distributions of porosities over the plate cross-section. The modified rule of mixture is used to approximate material properties of the FGM plates including the porosity volume fraction. In order the elastic coefficients, thermal coefficient and moisture expansion coefficient of the plate are assumed to be graded in the thickness direction. The elastic foundation is modeled as two-parameter Pasternak foundation. The equilibrium equations are given and a number of examples are solved to illustrate bending response of Metal-Ceramic plates subjected to hygro-thermo-mechanical effects and resting on elastic foundations. The influences played by many parameters are investigated.

Keywords: FGM plate; moisture concentration; thermal field; elastic foundations; higher-order shear deformation theory

1. Introduction

Functionally graded materials(FGMs), possessing spatially varying properties, have been developed for special components such as rocket engine components, aerospace structures, etc. The earliest FGMs were introduced by Japanese scientists in the mid 1980 as ultra-high temperature resistant materials for aerospace applications. Recently, these materials have found other uses in electrical devices, energy transformation, biomedical engineering, optics, etc (Suresh ,1998). However, in FGM fabrication, micro voids or porosities can occur within the materials during the process of sintering. This is because of the large difference in solidification temperatures between material constituents (Zhu 2001). Wattanasakulpong (2012) also gave the discussion on porosities happening inside FGM samples fabricated by a multi-step sequential infiltration technique. Therefore, it is important to take into account the porosity effect when designing FGM structures subjected to dynamic loadings. The degradation in performance of the

ISSN: 2234-0912 (Print), 2234-179X (Online)

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structure due to moisture concentration and high temperature has become increasingly more important with the prolonged use of functionally graded materials (FGMs) in many structural applications. As a result, hygrothermal internal stresses are generated with the change of environment. These stresses generally induce large deformation and could even contribute to the failure of the structure. The deformation and stress analysis of different plate structures subjected to moisture and temperature has been the subject of research interest of many investigators (Patel et al. 2002, Bahrami et al. 2007, Benkhedda et al. 2008, Ait amar et al. 2014, Benoun et al. 2016 and Bourada et al. 2015). The plates supported by an elastic foundation are very common in structural engineering. Many linear bending studies for thick plates subjected to transverse loads with elastic foundations are available in the literature (Voyiadjis et al. 1986). In some of the analyses of plates on elastic foundation, a single parameter is used to describe the foundation behavior according to Winkler's model (Liu 2000).

Based on the open literature, it is found that many researchers have paid their attention on investigating the mechanical or the thermal or the thermo-mechanical responses of FG plates and shells. A comprehensive review is done by Tanigawa (1995). Reddy (2000) has analyzed the static behavior of functionally graded rectangular plates based on his third-order shear deformation plate theory. The static response of FG plate has been investigated by Zenkour (2006) using a generalized shear deformation theory. In a recent study, a new four variable refined plate theory which involves only four unknown functions and yet takes into account shear deformations has been developed (Benachour et al. 2011, Hassaine Daouadji 2012 and 2013, Tlidji et al. 2014, Benferhat et al. 2014, Abdelhak et al. 2015, Bouderba et al. 2013, Habali et al. 2014, Mahi et al. 2015, Ait yahia et al. 2015, Tounsi 2013, Bellifa 2015, Belabed 2014, Hamidi 2015 and Zidi et al. 2014). This four variable refined plate theory is based on the assumption that the in-plane and transverse displacements consist of bending and shear components in which the bending components do not contribute toward shear forces and, likewise, the shear components do not contribute toward bending moments. The most interesting feature of this theory is that it accounts for a quadratic variation of the transverse shear strains across the thickness, and satisfies the zero traction boundary conditions on the top and bottom surfaces of the plate without using shear correction factors. In addition, it has strong similarities with the classical plate theory in some aspects including the structure of the governing equation, boundary conditions and moment expressions.

In this study, is to extend the four variable refined plate theory to the hygro-thermo-mechanical bending behavior of perfect and imperfect FGM plate resting on elastic foundations, motivated by rocket-launch pad structural foundations under thermal propulsion loads. Some other researchers have modeled the foundation with two parameters according to Pasternak's model. This two-parameter model takes into account the effect of shear interaction among the points in the foundation (Shen 1999, zenkour 2009). The objective of this investigation is to present a general hygrothermal formulation for imperfect FGM plates resting on an elastic foundation using the sinusoidal shear deformation theory.

2. Mathematical model and governing equations

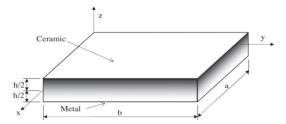


Fig. 1 Geometry of FGM rectangular plate

Consider a plate of total thickness h and composed of imperfect functionally graded material through the thickness (Fig. 1). It is assumed that the material is isotropic and grading is assumed to be only through the thickness. The xy plane is taken to be the undeformed mid plane of the plate with the z axis positive upward from the mid plane.

2.1 Displacement fields and strains

The assumed displacement field is as follows

$$u(x, y, z) = u_0(x, y) - z \frac{\partial w_b}{\partial x} - (z - \sin(\frac{\pi z}{h})) \frac{\partial w_s}{\partial x}$$

$$v(x, y, z) = v_0(x, y) - z \frac{\partial w_b}{\partial y} - (z - \sin(\frac{\pi z}{h})) \frac{\partial w_s}{\partial y}$$

$$w(x, y, z) = w_b(x, y) + w_s(x, y)$$
(1a)

where u_0 and v_0 are the mid-plane displacements of the plate in the x and y direction, respectively; w_b and w_s are the bending and shear components of transverse displacement, respectively, while f(z) represents shape functions determining the distribution of the transverse shear strains and stresses along the thickness (Benferhat *et al.* 2014) and is given as

$$f(z) = z - \sin(\frac{\pi z}{h}) \tag{1b}$$

It should be noted that unlike the first-order shear deformation theory, this theory does not require shear correction factors. The kinematic relations can be obtained as follows

$$\varepsilon_{x} = \varepsilon_{x}^{0} + z k_{x}^{b} + f(z) k_{x}^{s}
\varepsilon_{y} = \varepsilon_{y}^{0} + z k_{y}^{b} + f(z) k_{y}^{s}
\gamma_{xy} = \gamma_{xy}^{0} + z k_{xy}^{b} + f(z) k_{xy}^{s}
\gamma_{yz} = g(z) \gamma_{xz}^{s}
\varepsilon_{z} = 0$$
(2)

where

$$\boldsymbol{\varepsilon}_{x}^{0} = \frac{\partial u_{0}}{\partial x}, \quad \boldsymbol{k}_{x}^{b} = -\frac{\partial^{2} w_{b}}{\partial x^{2}}, \quad \boldsymbol{k}_{x}^{s} = -\frac{\partial^{2} w_{s}}{\partial x^{2}}, \quad \boldsymbol{\varepsilon}_{y}^{0} = \frac{\partial v_{0}}{\partial y}, \quad \boldsymbol{k}_{y}^{b} = -\frac{\partial^{2} w_{b}}{\partial v^{2}}, \quad \boldsymbol{k}_{y}^{s} = -\frac{\partial^{2} w_{s}}{\partial v^{2}}, \quad \boldsymbol{\gamma}_{xy}^{0} = \frac{\partial u_{0}}{\partial y} + \frac{\partial v_{0}}{\partial x}, \quad (3)$$

$$k_{xy}^b = -2\frac{\partial^2 w_b}{\partial x \partial y}$$
, $k_{xy}^s = -2\frac{\partial^2 w_s}{\partial x \partial y}$, $\gamma_{yz}^s = \frac{\partial w_s}{\partial y}$, $\gamma_{xz}^s = \frac{\partial w_s}{\partial x}$, $g(z) = 1 - f'(z)$ and $f'(z) = \frac{df(z)}{dz}$

2.2 Constitutive equations

Consider a rectangular plate of length a, width b and thickness h made of imperfect functionally graded material. The plate is graded from metal to ceramic. The plate is subjected to a sinusoidal distributed load q(x,y) and a temperature field T(x, y, z) as well as a moisture concentration C(x, y, z). The material properties V(z) of the FGM plate, such as Young's modulus E, Poisson's ratio v, and thermal α and moisture expansion β coefficients are given according to the modified rule of mixture as

$$V(z) = V_m + (V_c - V_m) \left(\frac{1}{2} + \frac{z}{h}\right)^p - (V_c + V_m) \frac{\zeta}{2}$$
 (4)

Where V_c and V_m are the corresponding properties of the ceramic and metal, respectively, and p is the volume fraction exponent which takes values greater than or equal to zero, and ζ is the porosity volume fraction of the imperfect FGM plate (ζ <<1) sited in (Wattanasakulpong 2012), perfect FGM plate can be obtained by setting ζ =0. The linear constitutive relations are

$$\begin{cases}
\sigma_{x} \\
\sigma_{y} \\
\tau_{xy}
\end{cases} = \begin{bmatrix}
Q_{11} & Q_{12} & 0 \\
Q_{12} & Q_{22} & 0 \\
0 & 0 & Q_{66}
\end{bmatrix} \begin{cases}
\varepsilon_{x} - \alpha \Delta T - \beta \Delta C \\
\varepsilon_{y} - \alpha \Delta T - \beta \Delta C \\
\gamma_{xy}
\end{cases}$$

$$\begin{cases}
\tau_{yz} \\
\tau_{zx}
\end{cases} = \begin{bmatrix}
Q_{44} & 0 \\
0 & Q_{55}
\end{bmatrix} \begin{cases}
\gamma_{yz} \\
\gamma_{zx}
\end{cases}$$
(5)

Where $(\sigma_x, \sigma_y, \tau_{yz}, \tau_{xz}, \tau_{xy})$ and $(\varepsilon_x, \varepsilon_y, \gamma_{yz}, \gamma_{xz}, \gamma_{xy})$ are the stress and strain components, respectively. Using the material properties defined in Eq. (4), stiffness coefficients, Q_{ij} , can be expressed as

$$Q_{11} = Q_{22} = \frac{E(z)}{1 - v^2} \quad Q_{12} = \frac{v E(z)}{1 - v^2} \qquad Q_{44} = Q_{55} = Q_{66} = \frac{E(z)}{2(1 + v)}$$
 (6)

Where $\Delta T = T - T_0$ and $\Delta C = C - C_0$ in which T_0 is the reference temperature and C_0 is the reference moisture concentration. Now, let us consider a temperature distribution field T(x,y,z) and the moisture concentration C(x,y,z) in the plate domain V (plate volume) in the form

$$T(x, y, z) = T_z(z)T_{\Omega}(x, y) \quad C(x, y, z) = C_z(z)C_{\Omega}(x, y)$$
 (7a)

Where $T_z(z)$ and $C_z(z)$ represent the temperature and the moisture concentration profile across the plate thickness coordinate z, respectively, where as $T_\Omega(x, y)$ and $C_\Omega(x, y)$ are the temperature and the moisture concentration distributions, respectively, over the reference surface domain Ω . In the most general case, the form of $T_z(z)$ is a result of the solution of a heat conduction problem. This calculated $T_z(z)$ turns out to be a nonlinear polynomial of z, often of a transcendental form (Zenkour 2009). In the case of a thick multi-layered structure, $T_z(z)$ would may be require a layerwise description (Tlidji et al. 2014), that is: plate theories with higher-order (or layerwise) displacement fields are required to capture a temperature profile obtained from the solution of a

heat-conduction problem. In the present study, the moisture concentration C(x,y,z) and the temperature distribution field T(x,y,z) are given as (Tlidji *et al.* 2014)

$$T(x, y, z) = T_1(x, y) + \frac{z}{h} T_2(x, y) + \frac{1}{\pi} \sin\left(\frac{\pi z}{h}\right) T_3(x, y)$$
 (7b)

$$C(x, y, z) = C_1(x, y) + \frac{z}{h}C_2(x, y) + \frac{1}{\pi}\sin\left(\frac{\pi z}{h}\right)C_3(x, y)$$
 (7c)

2.3 Governing equations

The governing equations of equilibrium can be derived by using the principle of virtual displacements. The principle of virtual work in the present case yields

$$\int_{-h/2}^{h/2} \int_{\Omega} \left[\sigma_x \delta \varepsilon_x + \sigma_y \delta \varepsilon_y + \tau_{xy} \delta \gamma_{xy} + \tau_{yz} \delta \gamma_{yz} + \tau_{xz} \delta \gamma_{xz} \right] d\Omega dz - \int_{\Omega} (q - f_e) \delta w dz = 0$$
 (8)

Where Ω is the top surface and f_e is the density of reaction force of foundation. For the Pasternak foundation model

$$f_e = K_w w - J_1 \frac{\partial w}{\partial x^2} - J_2 \frac{\partial w}{\partial y^2}$$
(9)

where K_w is the modulus of subgrade reaction (elastic coefficient of the foundation) and J_1 and J_2 are the shear moduli of the subgrade (shear layer foundation stiffness). If the foundation is homogeneous and isotropic, this implies that $J_1 = J_2 = J_0$. If the shear layer foundation stiffness is neglected, Pasternak foundation becomes a Winkler foundation.

Substituting Eqs. (2) and (5) into Eq. (8) and integrating through the thickness of the plate, Eq. (8) can be rewritten as

$$\int_{\Omega} \left[N_{x} \delta \varepsilon_{x}^{0} + N_{y} \delta \varepsilon_{y}^{0} + N_{xy} \delta \gamma_{xy}^{0} + M_{x}^{b} \delta k_{x}^{b} + M_{y}^{b} \delta k_{y}^{b} + M_{xy}^{b} \delta k_{xy}^{b} + M_{x}^{s} \delta k_{x}^{s} + M_{y}^{s} \delta k_{y}^{s} \right] d\Omega - \int_{\Omega} (q - f_{e}) (\delta w_{b} + \delta w_{s}) d\Omega = 0$$
(10)

The stress resultants N, M, and S are defined by

$$\begin{cases}
N_{x} & N_{y} & N_{xy} \\
M_{x}^{b} & M_{y}^{b} & M_{xy}^{b} \\
M_{x}^{s} & M_{y}^{s} & M_{xy}^{s}
\end{cases} = \int_{-\frac{h}{2}}^{\frac{h}{2}} (\sigma_{x}, \sigma_{y}, \tau_{xy}) \begin{cases} 1 \\ z \\ f(z) \end{cases} dz \tag{11a}$$

$$(S_{xz}^{s}, S_{yz}^{s}) = \int_{-\frac{h}{2}}^{\frac{h}{2}} (\tau_{xz}, \tau_{yz}) g(z) dz$$
 (11b)

Substituting Eq. (5) into Eq. (11) and integrating through the thickness of the plate, the stress resultants are given as

$$\begin{cases}
S_{yz}^{s} \\
S_{xz}^{s}
\end{cases} = \begin{bmatrix}
A_{44}^{s} & 0 \\
0 & A_{55}^{s}
\end{bmatrix} \begin{Bmatrix} \gamma_{yz}^{s} \\
\gamma_{xz}^{s}
\end{Bmatrix}$$
(12b)

Where

$$N = \{N_x, N_y, N_{xy}\}^t, \quad M^b = \{M_x^b, M_y^b, M_{xy}^b\}^t, M^s = \{M_x^s, M_y^s, M_{xy}^s\}^t$$
(13a)

$$N^{T} = \left\{ N_{x}^{T}, N_{y}^{T}, 0 \right\}^{t}, M^{bT} = \left\{ M_{x}^{bT}, M_{y}^{bT}, 0 \right\}^{t}, M^{sT} = \left\{ M_{x}^{sT}, M_{y}^{sT}, 0 \right\}^{t}$$
(13b)

$$N^{C} = \left\{ N_{x}^{C}, N_{y}^{C}, 0 \right\}^{t}, M^{bC} = \left\{ M_{x}^{bC}, M_{y}^{bC}, 0 \right\}^{t}, M^{sC} = \left\{ M_{x}^{sC}, M_{y}^{sC}, 0 \right\}^{t}$$
(13c)

$$\varepsilon = \left\{ \varepsilon_{x}^{0}, \varepsilon_{y}^{0}, \gamma_{xy}^{0} \right\}^{t}, \quad k^{b} = \left\{ k_{x}^{b}, k_{y}^{b}, k_{xy}^{b} \right\}^{t}, \quad k^{s} = \left\{ k_{x}^{s}, k_{y}^{s}, k_{xy}^{s} \right\}^{t}$$

$$(13d)$$

$$A = \begin{bmatrix} A_{11} & A_{12} & 0 \\ A_{12} & A_{22} & 0 \\ 0 & 0 & A_{66} \end{bmatrix}, \quad B = \begin{bmatrix} B_{11} & B_{12} & 0 \\ B_{12} & B_{22} & 0 \\ 0 & 0 & B_{66} \end{bmatrix} \quad D = \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{12} & D_{22} & 0 \\ 0 & 0 & D_{66} \end{bmatrix}$$
(13e)

$$B^{s} = \begin{bmatrix} B_{11}^{s} & B_{12}^{s} & 0 \\ B_{12}^{s} & B_{22}^{s} & 0 \\ 0 & 0 & B_{66} \end{bmatrix}, \quad D^{s} = \begin{bmatrix} D_{11}^{s} & D_{12}^{s} & 0 \\ D_{12}^{s} & D_{22}^{s} & 0 \\ 0 & 0 & D_{66} \end{bmatrix}, \quad H^{s} = \begin{bmatrix} H_{11}^{s} & H_{12}^{s} & 0 \\ H_{12}^{s} & H_{22}^{s} & 0 \\ 0 & 0 & H_{66} \end{bmatrix}$$
(13f)

Where A_{ij} , B_{ij} , D_{ij} etc., are the plate stiffness, defined by

$$\begin{cases}
A_{11} & B_{11} & D_{11} & B_{11}^{s} & D_{11}^{s} & H_{11}^{s} \\
A_{12} & B_{12} & D_{12} & B_{12}^{s} & D_{12}^{s} & H_{12}^{s} \\
A_{66} & B_{66} & D_{66} & B_{66}^{s} & D_{66}^{s} & H_{66}^{s}
\end{cases} = \int_{-h/2}^{+h/2} Q_{11}(1, z, z^{2}, f(z), zf(z), f^{2}(z)) \begin{cases} 1 \\ v \\ \frac{1-v}{2} \end{cases} dz$$
(14a)

$$(A_{22}, B_{22}, D_{22}, B_{22}^{s}, D_{22}^{s}, H_{22}^{s}) = (A_{11}, B_{11}, D_{11}, B_{11}^{s}, D_{11}^{s}, H_{11}^{s})$$
(14b)

$$A_{44}^{s} = A_{55}^{s} = \int_{-h/2}^{h/2} \frac{E(z)}{2(1+v)} [g(z)]^{2} dz$$
 (14c)

The stress and moment resultants,

$$N_{x}^{T} = N_{y}^{T}, M_{x}^{bT} = M_{y}^{bT}, M_{x}^{sT} = M_{y}^{sT}, N_{x}^{C} = N_{y}^{C}, M_{x}^{bC} = M_{y}^{bC}, M_{x}^{sC} = M_{y}^{sC}$$

due to thermal and hygroscopic loading are defined respectively by

$$\begin{cases}
N_{x}^{T} \\
M_{x}^{bT} \\
M_{x}^{sT}
\end{cases} = \int_{-\frac{h}{2}}^{\frac{h}{2}} \frac{E(z)}{1-\nu} \alpha(z) T \begin{cases} 1 \\ z \\ f(z) \end{cases} dz , \qquad \begin{cases}
N_{x}^{C} \\
M_{x}^{bC} \\ M_{x}^{sC}
\end{cases} = \int_{-\frac{h}{2}}^{\frac{h}{2}} \frac{E(z)}{1-\nu} \beta(z) C \begin{cases} 1 \\ z \\ f(z) \end{cases} dz$$
(15)

The governing equations of equilibrium can be derived from Eq. (10) by integrating the displacement gradients by parts and setting the coefficients δu_0 , δv_0 , δw_b and δw_s zero separately. Thus one can obtain the equilibrium equations associated with the present shear deformation theory,

$$\delta \mathbf{u}_{0}: \frac{\partial N_{x}}{\partial x} + \frac{\partial N_{xy}}{\partial y} = 0$$

$$\delta \mathbf{v}_{0}: \frac{\partial N_{xy}}{\partial x} + \frac{\partial N_{y}}{\partial y} = 0$$

$$\delta \mathbf{w}_{b}: \frac{\partial^{2} M_{x}^{b}}{\partial x^{2}} + 2 \frac{\partial^{2} M_{xy}^{b}}{\partial x \partial y} + \frac{\partial^{2} M_{y}^{b}}{\partial y^{2}} - f_{e} + q = 0$$

$$\delta \mathbf{w}_{s}: \frac{\partial^{2} M_{x}^{s}}{\partial x^{2}} + 2 \frac{\partial^{2} M_{xy}^{s}}{\partial x \partial y} + \frac{\partial^{2} M_{y}^{s}}{\partial y^{2}} + \frac{\partial S_{xz}^{s}}{\partial x} + \frac{\partial S_{yz}^{s}}{\partial y} - f_{e} + q = 0$$

$$(16)$$

Substituting from Eq. (12) into Eq. (16), we obtain the following equation,

$$A_{11}d_{11}u_0 + A_{66}d_{22}u_0 + (A_{12} + A_{66})d_{12}v_0 - B_{11}d_{111}w_b - (B_{12} + 2B_{66})d_{122}w_b - (B_{12}^s + 2B_{66}^s)d_{122}w_s - B_{11}^sd_{111}w_s = p_1$$
(17a)

$$A_{22}d_{22}v_0 + A_{66}d_{11}v_0 + (A_{12} + A_{66})d_{12}u_0 - B_{22}d_{222}w_b - (B_{12} + 2B_{66})d_{112}w_b - (B_{12}^s + 2B_{66}^s)d_{112}w_s - B_{22}^sd_{222}w_s = p_2$$

$$(17b)$$

$$B_{11}d_{111}u_0 + (B_{12} + 2B_{66})d_{122}u_0 + (B_{12} + 2B_{66})d_{112}v_0 + B_{22}d_{222}v_0 - D_{11}d_{1111}w_b - 2(D_{12} + 2D_{66})d_{1122}w_b - D_{22}d_{2222}w_b - D_{11}^sd_{1111}w_s - 2(D_{12}^s + 2D_{66}^s)d_{1122}w_s$$

$$-D_{22}^sd_{2222}w_s = p_3$$
(17c)

$$B_{11}^{s} d_{111} u_{0} + (B_{12}^{s} + 2 B_{66}^{s}) d_{122} u_{0} + (B_{12}^{s} + 2 B_{66}^{s}) d_{112} v_{0} + B_{22}^{s} d_{222} v_{0} - D_{11}^{s} d_{1111} w_{b}$$

$$-2(D_{12}^{s} + 2 D_{66}^{s}) d_{1122} w_{b} - D_{22}^{s} d_{2222} w_{b} - H_{11}^{s} d_{1111} w_{s} - 2(H_{12}^{s} + 2 H_{66}^{s}) d_{1122} w_{s}$$

$$-H_{22}^{s} d_{2222} w_{s} + A_{55}^{s} d_{11} w_{s} + A_{44}^{s} d_{22} w_{s} = p_{4}$$

$$(17d)$$

Where $\{p\}=\{p_1, p_2, p_3, p_4\}^t$ is a generalized force vector, d_{ij} , d_{ijl} and d_{ijlm} are the following differential operators

$$d_{ij} = \frac{\partial^{2}}{\partial x_{i} \partial x_{j}}, \quad d_{ijl} = \frac{\partial^{3}}{\partial x_{i} \partial x_{j} \partial x_{l}}, \quad d_{ijlm} = \frac{\partial^{4}}{\partial x_{i} \partial x_{i} \partial x_{l} \partial x_{m}}, \quad d_{i} = \frac{\partial}{\partial x_{i}} \quad (i, j, l, m = 1, 2)$$
(18a)

The components of the generalized force vector $\{p\}$ are given by

$$p_{1} = \frac{\partial N_{x}^{T}}{\partial x} + \frac{\partial N_{x}^{C}}{\partial x}, \qquad p_{3} = f_{e} + q - \frac{\partial^{2} \left(M_{x}^{bT} + M_{x}^{bC}\right)}{\partial x^{2}} - \frac{\partial^{2} \left(M_{y}^{bT} + M_{y}^{bC}\right)}{\partial y^{2}}$$

$$p_{2} = \frac{\partial N_{y}^{T}}{\partial y} + \frac{\partial N_{y}^{C}}{\partial y}; \qquad p_{4} = f_{e} + q - \frac{\partial^{2} \left(M_{x}^{sT} + M_{x}^{sC}\right)}{\partial x^{2}} - \frac{\partial^{2} \left(M_{y}^{sT} + M_{y}^{sC}\right)}{\partial y^{2}}$$

$$(18b)$$

3. Exact solutions

Following Navier procedure, the uniform external force as well as the transverse uniform temperature and moisture concentration loads are presented in the form of a double trigonometric series. We are here concerned with the exact solution of Eqs. (17) for a simply supported FGM plate. To solve this problem, Navier assumed that the transverse mechanical, temperature and moisture loads, q, T_i and C_i in the form of a in the double Fourier series as

Where $\lambda = \pi/a$, $\mu = \pi/b$, q_0 , t_i and c_i are constants and T_i and C_i are defined in Eq. (7). Following the Navier procedure, we assume the following solution form for u_0 , v_0 , w_b and w_s that satisfies the boundary conditions,

Where U, V, W_b and W_s are arbitrary parameters to be determined subjected to the condition that the solution in Eq. (20) satisfies governing equation (17). One obtains the following operator equation,

$$[K]\{\Delta\} = \{P\} \tag{21}$$

Where $\{\Delta\}=\{U,V,W_h,W_s\}^t$ and [K] is the symmetric matrix given by

In which

$$k_{11} = -(A_{11}\lambda^{2} + A_{66}\mu^{2}), \quad k_{22} = -(A_{66}\lambda^{2} + A_{22}\mu^{2}), \quad k_{33} = -(D_{11}\lambda^{4} + 2(D_{12} + 2D_{66})\lambda^{2}\mu^{2} + D_{22}\mu^{4} + K_{w} + J_{1}\lambda^{2} + J_{2}\mu^{2});$$

$$k_{12} = -\lambda \mu \left(A_{12} + A_{66}\right), \quad k_{13} = \lambda \left[B_{11}\lambda^{2} + (B_{12} + 2B_{66})\mu^{2}\right], \quad k_{14} = \lambda \left[B_{11}^{s}\lambda^{2} + (B_{12}^{s} + 2B_{66}^{s})\mu^{2}\right];$$

$$k_{23} = \mu \left[(B_{12} + 2B_{66})\lambda^{2} + B_{22}\mu^{2}\right], \quad k_{24} = \mu \left[(B_{12}^{s} + 2B_{66}^{s})\lambda^{2} + B_{22}^{s}\mu^{2}\right];$$

$$k_{34} = -(D_{11}^{s}\lambda^{4} + 2(D_{12}^{s} + 2D_{66}^{s})\lambda^{2}\mu^{2} + D_{22}^{s}\mu^{4} + K_{w} + J_{1}\lambda^{2} + J_{2}\mu^{2})$$

$$k_{44} = -(H_{11}^{s}\lambda^{4} + 2(H_{12}^{s} + 2H_{66}^{s})\lambda^{2}\mu^{2} + H_{22}^{s}\mu^{4} + A_{55}^{s}\lambda^{2} + A_{44}^{s}\mu^{2} + D_{22}^{s}\mu^{4} + K_{w} + J_{1}\lambda^{2} + J_{2}\mu^{2})$$

The components of the generalized force vector $\{P\} = \{P_1, P_2, P_3, P_4\}^t$ are given by

$$P_{1} = \lambda \left[(A^{T} t_{1} + B^{T} t_{2} + {}^{a} B^{T} t_{3}) + (A^{C} c_{1} + B^{C} c_{2} + {}^{a} B^{C} c_{3}) \right]$$

$$P_{2} = \mu \left[(A^{T} t_{1} + B^{T} t_{2} + {}^{a} B^{T} t_{3}) + (A^{C} c_{1} + B^{C} c_{2} + {}^{a} B^{C} c_{3}) \right]$$

$$P_{3} = -q_{0} - h(\lambda^{2} + \mu^{2}) \left[(B^{T} t_{1} + D^{T} t_{2} + {}^{a} D^{T} t_{3}) + (B^{C} c_{1} + D^{C} c_{2} + {}^{a} D^{C} c_{3}) \right]$$

$$P_{4} = -q_{0} - h(\lambda^{2} + \mu^{2}) \left[({}^{s} B^{T} t_{1} + {}^{s} D^{T} t_{2} + {}^{s} F^{T} t_{3}) + ({}^{s} B^{C} c_{1} + {}^{s} D^{C} c_{2} + {}^{s} F^{C} c_{3}) \right]$$

$$(24)$$

Where

$$\left\{A^{T}, B^{T}, D^{T}\right\} = \int_{-h/2}^{h/2} \frac{E(z)}{1 - \nu} \alpha(z) \left\{1, \overline{z}, \overline{z}^{2}\right\} dz$$
 (25a)

$$\left\{A^{C}, B^{C}, D^{C}\right\} = \int_{-b/2}^{b/2} \frac{E(z)}{1 - v} \beta(z) \left\{1, \overline{z}, \overline{z}^{2}\right\} dz$$
 (25b)

$$\left\{ {}^{a}B^{T}, {}^{a}D^{T} \right\} = \int_{-h/2}^{h/2} \frac{E(z)}{1-v} \alpha(z) \overline{\psi}(z) \left\{ 1, \overline{z} \right\} dz \tag{25c}$$

$$\left\{ {}^{a}B^{C}, {}^{a}D^{C} \right\} = \int_{-h/2}^{h/2} \frac{E(z)}{1 - v} \beta(z) \overline{\psi}(z) \left\{ 1, \overline{z} \right\} dz$$
 (25d)

$$\left\{ {}^{s}B^{T}, {}^{s}D^{T}, {}^{s}F^{T} \right\} = \int_{-h/2}^{h/2} \frac{E(z)}{1 - \nu} \alpha(z) \, \bar{f}(z) \left\{ 1, \, \overline{z}, \, \overline{\psi}(z) \right\} dz$$
 (25e)

$$\left\{ {}^{s}B^{C}, {}^{s}D^{C}, {}^{s}F^{C} \right\} = \int_{-h/2}^{h/2} \frac{E(z)}{1 - \nu} \beta(z) \, \bar{f}(z) \left\{ 1, \, \overline{z}, \, \overline{\psi}(z) \right\} dz$$
 (25f)

In which

$$\overline{z} = z / h$$
, $\overline{f}(z) = f(z) / h$, $\overline{\psi}(z) = \frac{1}{\pi} \sin\left(\frac{\pi z}{h}\right)$

Table 1 Effect of the volume fraction exponent and elastic foundation parameters on the dimensionless deflection and stresses of an FGM rectangular plate with and without porosities (a = 10h, b = 3a, $q_0 = 100$, T = C = 0).

p	K_0	\boldsymbol{J}_0	Theory	ζ	\overline{w}	$\overline{\sigma}_{\scriptscriptstyle x}$	$\overline{ au}_{xy}$	$\overline{ au}_{xz}$
			Present Model	$\zeta = 0$	0.85885	0.51364	0.72787	-0.44328
	0	0	Reddy model	$\zeta = 0$	0.85891	0.51545	0.72797	-0.42956
			FSDT model	$\zeta = 0$	0.85892	0.51065	0.72949	-0.34377
			Present Model	$\zeta = 0$	0.46203	0.27632	0.39157	-0.23847
	100	0	Reddy model	$\zeta = 0$	0.46206	0.27620	0.39162	-0.23109
0			FSDT model	$\zeta = 0$	0.46206	0.27471	0.39246	-0.18493
U			Present Model	$\zeta = 0$	0.08964	0.05361	0.07597	-0.04627
	0	100	Reddy model	$\zeta = 0$	0.08965	0.05358	0.07599	-0.04485
			FSDT model	$\zeta = 0$	0.08965	0.05330	0.07616	-0.03588
			Present Model	$\zeta = 0$	0.08227	0.04920	0.06972	-0.04246
	100	100	Reddy model	$\zeta = 0$	0.08228	0.04919	0.06972	-0.04116
			FSDT model	$\zeta = 0$	0.08228	0.04890	0.06986	-0.03293
		100		$\zeta = 0$	0.08364	0.04671	0.05812	-0.03609
	100		Present Model	$\zeta = 0.1$	0.08430	0.04308	0.05290	-0.03291
0.5				$\zeta = 0.2$	0.08496	0.03938	0.04758	-0.02968
			Reddy model	$\zeta = 0$	0.08366	0.04671	0.05812	-0.03498
			FSDT model	$\zeta = 0$	0.08366	0.04643	0.05824	-0.02828
		100		$\zeta = 0$	0.08456	0.04541	0.04769	-0.03049
	100		Present Model	$\zeta = 0.1$	0.08523	0.04162	0.04222	-0.02715
2				$\zeta = 0.2$	0.08590	0.03768	0.03661	-0.02378
			Reddy model	$\zeta = 0$	0.08457	0.04539	0.04770	-0.02951
			FSDT model	$\zeta = 0$	0.08457	0.04515	0.04781	-0.02303
	100	100		$\zeta = 0$	0.08584	0.02905	0.04116	-0.02507
			Present Model	$\zeta = 0.1$	0.08651	0.02522	0.03574	-0.02177
Metal				$\zeta = 0.2$	0.08720	0.02134	0.03024	-0.01842
			Reddy model	$\zeta = 0$	0.08584	0.02905	0.04115	-0.02428
			FSDT model	$\zeta = 0$	0.08584	0.02888	0.04126	-0.01943

4. Numerical results

To illustrate the proposed approach, the Titanium/Zerconia of perfect and imperfect FG plate is considered. Young's modulus, Poisson's ratio, coefficient of thermal expansion, and moisture concentration expansion for

Titanium, *Ti-6Al-4V*:
$$E_m = 66.2$$
 GPa ; $v = 1/3$; ; $\beta_m = 0.33$ Zirconia, ZrO_2 : $E_c = 117.0$ GPa ; $v = 1/3$; $\alpha_c = 7.11 \times (10^{-6} / {}^{\circ}C)$; $\beta_c = 0$

Table 2 Effect of the volume fraction exponent and elastic foundation parameters on the dimensionless deflection and stresses of an FGM rectangular plate (a = 10h, b = 3a, $q_0 = 100$; $t_1 = t_3 = 0$, $t_2 = 10$, $t_3 = 0$, $t_4 = 10$).

p	K_0	${\pmb J}_0$	Theory	ζ	\overline{w}	$\overline{\sigma}_{_{\scriptscriptstyle X}}$	$\overline{ au}_{xy}$	$\overline{ au}_{xz}$
			Present Model	$\zeta = 0$	1.80707	0.47204	1.55974	-0.44328
	0	0	Reddy model	$\zeta = 0$	1.80712	0.47187	1.55982	-0.42955
			FSDT model	$\zeta = 0$	1.80713	0.46900	1.56139	-0.34377
			Present Model	$\zeta = 0$	0.97214	-0.02729	0.85215	-0.01235
	100	0	Reddy model	$\zeta = 0$	0.97216	-0.02740	0.85211	-0.01197
Ceramic			FSDT model	$\zeta = 0$	0.97216	-0.02750	0.85216	-0.00957
Cerannic			Present Model	$\zeta = 0$	0.18861	-0.49588	0.18810	0.39206
	0	100	Reddy model	$\zeta = 0$	0.18861	-0.49570	0.18806	0.37990
			FSDT model	$\zeta = 0$	0.18860	-0.49322	0.18667	0.30404
			Present Model	$\zeta = 0$	0.17309	-0.50516	0.17495	0.40007
	100	100	Reddy model	$\zeta = 0$	0.17309	-0.50498	0.17490	0.38766
			FSDT model	$\zeta = 0$	0.17309	-0.50245	0.17351	0.31024
				$\zeta = 0$	0.18411	-0.51999	0.18301	0.45727
	100	100	Present Model	$\zeta = 0.1$	0.16549	-0.42187	0.15147	0.36625
0.5				$\zeta = 0.2$	0.14872	-0.33279	0.12405	0.28426
			Reddy model	$\zeta = 0$	0.18410	-0.51975	0.18299	0.44334
			FSDT model	$\zeta = 0$	0.18408	-0.51623	0.18190	0.35898
				$\zeta = 0$	0.18504	-0.51475	0.15636	0.45986
			Present Model	$\zeta = 0.1$	0.16621	-0.41652	0.12578	0.36814
1	100	100		$\zeta = 0.2$	0.14925	-0.32731	0.09924	0.28549
			Reddy model	$\zeta = 0$	0.18504	-0.51450	0.15631	0.44545
			FSDT model	$\zeta = 0$	0.18505	-0.51084	0.15494	0.35542
				$\zeta = 0$	0.18559	-0.50363	0.13461	0.45337
			Present Model	$\zeta = 0.1$	0.16659	-0.40543	0.10501	0.36169
2	100	100		$\zeta = 0.2$	0.14945	-0.31618	0.79451	0.27908
			Reddy model	$\zeta = 0$	0.18560	-0.50336	0.13451	0.43831
			FSDT model	$\zeta = 0$	0.18567	-0.49980	0.13244	0.33958
			Present Model	$\zeta = 0$	0.18840	-0.43117	0.12092	0.47465
Metal	100	100	Reddy model	$\zeta = 0$	0.18840	-0.43095	0.12087	0.45993
			FSDT model	$\zeta = 0$	0.18840	-0.42794	0.11921	0.36808

In this section, numerical examples are presented and discussed for verifying the accuracy of the present theory in predicting the hygro-thermo-mechanical bending responses of perfect and imperfect FGM plates. Comparisons are made with various plate theories available in the scientific literature. The reference temperature and moisture concentration are taken by T_0 =25°C (room temperature) and C_0 =0%, numerical results are presented in terms of non-dimensional stresses and

Table 3 Effect of the volume fraction exponent and elastic foundation parameters on the dimensionless deflection and stresses of an FGM rectangular plate with and without porosities (a=10h, b=3a, $q_0=100$, $t_1=0$, $t_2=t_3=10$, $c_1=0$, $c_2=c_3=100$).

v	T	TD1	۶-	_		-	
\mathbf{K}_0	\boldsymbol{J}_0	Theory	5	W	σ_{x}	τ_{xy}	$\overline{ au}_{\scriptscriptstyle XZ}$
0	0	Present Model	$\zeta = 0$	2.54067	0.52554	2.20366	-0.43753
		Reddy model	$\zeta = 0$	2.54076	0.52522	2.20374	-0.42454
100	0	Present Model	$\zeta = 0$	1.36680	-0.17649	1.20881	0.16834
		Reddy model	$\zeta = 0$	1.36682	-0.17643	1.20877	0.16257
Λ	100	Present Model	$\zeta = 0$	0.26517	-0.83532	0.27518	0.73692
U	100	Reddy model	$\zeta = 0$	0.26518	-0.83500	0.27507	0.71354
100	100	Present Model	$\zeta = 0$	0.24336	-0.84837	0.25669	0.74817
100	100	Reddy model	$\zeta = 0$	0.24336	-0.84804	0.25658	0.72442
100	100		$\zeta = 0$	0.26196	-0.87282	0.28041	0.84587
		Present Model	$\zeta = 0.1$	0.22844	-0.71238	0.22844	0.68075
			$\zeta = 0.2$	0.19820	-0.56627	0.18386	0.53174
		Reddy model	$\zeta = 0$	0.26195	-0.87239	0.28034	0.81947
100	100		$\zeta = 0$	0.26330	-0.86250	0.23772	0.84866
		Present Model	$\zeta = 0.1$	0.22940	-0.70179	0.18756	0.68224
			$\zeta = 0.2$	0.19878	-0.55537	0.14466	0.53196
		Reddy model	$\zeta = 0$	0.26330	-0.86205	0.23762	0.82148
	100		$\zeta = 0$	0.26394	-0.84183	0.20154	0.83481
100		Present Model	$\zeta = 0.1$	0.22972	-0.68110	0.15319	0.66835
			$\zeta = 0.2$	0.19878	-0.53449	0.11213	0.51806
		Reddy model	$\zeta = 0$	0.26396	-0.84138	0.20133	0.80652
100	100		$\zeta = 0$	0.26599	-0.81790	0.18486	0.83228
		Present Model	$\zeta = 0.1$	0.23148	-0.65816	0.13785	0.66489
			$\zeta = 0.2$	0.20025	-0.51226	0.09805	0.51369
		Reddy model	$\zeta = 0$	0.26601	-0.81745	0.18459	0.80297
100	100	Present Model	$\zeta = 0$	0.26775	-0.71694	0.18299	0.86749
	100	Reddy model	$\zeta = 0$	0.26774	-0.71656	0.18286	0.84003
	100 0 100 100 100	0 0 100 0 0 100 100 100 100 100 100 100 100 100	0 0 Present Model Reddy model Reddy model Present Model Reddy model Reddy model Present Model Reddy model Reddy model Present Model Reddy model Present Model Reddy model Present Model Reddy model Reddy model Present Model Present Model Reddy model	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

deflection. The various non-dimensional parameters used are

$$\overline{w} = \frac{10^{2} D}{a^{4} q_{0}} w \left(\frac{a}{2}, \frac{b}{2}\right); \qquad \overline{\sigma}_{x} = \frac{1}{10^{2} q_{0}} \sigma_{x} \left(\frac{a}{2}, \frac{b}{2}, \frac{h}{2}\right); \qquad \overline{\tau}_{xy} = \frac{1}{10 q_{0}} \tau_{xy} \left(0, 0, \frac{-h}{3}\right)$$

$$\overline{\tau}_{xz} = -\frac{1}{10 q_{0}} \tau_{xz} \left(0, \frac{b}{2}, 0\right); \qquad K_{0} = \frac{a^{4} K_{w}}{D}; \qquad J_{0} = \frac{a^{2} J_{1}}{D} = \frac{b^{2} J_{2}}{D}; \qquad D = \frac{h^{3} E_{c}}{12(1 - v^{2})}$$

Numerical results are presented in tabulated in Tables 1-4 and graphically plotted in Figs. 2-10 using the present model . It is assumed, unless otherwise stated, that $q_0=100~GPa$, a/h=10~, b/a=3, p=2, $t_1=t_2=0$, $c_1=c_2=0$.

The correlation between the present model and the higher-order theory Reddy model (2000) and first-order shear deformation theories FSDT is illustrated in Tables 1-3. These tables give also the effects of the volume fraction exponent ratio *P* and elastic foundation parameters on the dimensionless deflection and stresses of perfect and imperfect FGM rectangular plate. Table 1

Table 4 Effects of side-to-thickness ratio and elastic foundation parameters on the dimensionless deflection of an FGM square plate (b=a, $q_0=100$, $t_1=0$, $t_2=t_3=10$, $t_1=0$, $t_2=t_3=10$, .).

P	K_0	1	ζ	a/h					
r	11 0	${J}_0$	<u> </u>	5	10	20	50	100	
0.5			$\zeta = 0$	5.29023	1.55628	0.62189	0.36020	032281	
	0	0	$\zeta = 0.1$	4.77653	1.45251	0.62069	0.38772	0.35444	
			$\zeta = 0.2$	4.26990	1.35652	0.62744	0.42324	0.39407	
			$\zeta = 0$	3.83159	1.17192	0.47301	0.27474	0.24632	
	100	0	$\zeta = 0.1$	3.36205	1.06588	0.46039	0.28846	0.26381	
			$\zeta = 0.2$	2.90407	0.96489	0.45148	0.30554	28462	
			$\zeta = 0$	0.62132	0.20823	0.08622	0.05045	0.04529	
	0	100	$\zeta = 0.1$	0.51335	0.17800	0.07884	0.04975	0.04555	
			$\zeta = 0.2$	0.41521	0.15053	0.07218	0.04919	0.04587	
			$\zeta = 0$	0.59473	0.19948	0.08261	0.04834	0.04339	
	100	100	$\zeta = 0.1$	0.49114	0.17043	0.07550	0.04765	0.04362	
			$\zeta = 0.2$	0.39705	0.14404	0.06908	0.04709	0.04391	
			$\zeta = 0$	6.07957	1.79155	0.71642	0.41516	0.37211	
	0	0	$\zeta = 0.1$	5.57581	1.69972	0.72762	0.45521	0.41629	
			$\zeta = 0.2$	5.08437	1.62145	0.75267	0.50920	0.47441	
			$\zeta = 0$	4.20771	1.29862	0.52553	0.30557	0.27402	
	100	0	$\zeta = 0.1$	3.71360	1.19036	0.51624	0.32416	0.29660	
2			$\zeta = 0.2$	3.22777	1.08752	0.51207	0.34783	0.32425	
2		100	$\zeta = 0$	0.62155	0.21095	0.08769	0.05139	0.04613	
	0		$\zeta = 0.1$	0.51162	0.17993	0.08011	0.05069	0.04643	
			$\zeta = 0.2$	0.41156	0.15166	0.07326	0.05013	0.04678	
			$\zeta = 0$	0.59451	0.20193	0.08396	0.04920	0.04417	
	100	100	$\zeta = 0.1$	0.48911	0.17213	0.07666	0.04851	0.04443	
			$\zeta = 0.2$	0.39325	0.14500	0.07005	0.04794	0.04474	
10			$\zeta = 0$	6.81971	2.00812	0.80056	0.46210	0.41374	
	0	0	$\zeta = 0.1$	6.32870	1.92700	0.82192	0.51216	0.46790	
			$\zeta = 0.2$	5.85622	1.86429	0.86163	0.58054	0.54038	
			$\zeta = 0$	4.54370	1.41069	0.57016	0.33039	0.29598	
	100	0	$\zeta = 0.1$	4.02487	1.29937	0.56272	0.35216	0.32192	
			$\zeta = 0.2$	3.51079	1.19327	0.56102	0.37984	0.35380	
		100	$\zeta = 0$	0.62637	0.21455	0.89184	0.52102	0.04673	
	0		$\zeta = 0.1$	0.51458	0.18292	0.81441	0.51379	0.04702	
			$\zeta = 0.2$	0.41279	0.15407	0.74427	0.50790	0.04736	
			$\zeta = 0$	0.59882	0.20526	0.08534	0.04986	0.04472	
	100	100	$\zeta = 0.1$	0.49170	0.17490	0.07789	0.04914	0.04497	
	100	100	2	0.12170	0.17 170	0.07707	0.0 1711	0.01171	

gives the effects of the volume fraction exponent ratio P and elastic foundation parameters on the dimensionless displacements and stresses of perfect and imperfect FGM plate subjected to a mechanical load. It can be shown that the deflection and stresses are decreasing with the existence of the elastic foundations. The inclusion of the Winkler foundation parameter yields higher magnitude results than those with the inclusion of Pasternak foundation parameters. As the volume fraction exponent increases for perfect and imperfect FGM plates, the deflection will increase. The stresses are also sensitive to the variation of p. A table 2 and 3 presents similar results as those given in Table 1 including the effect of the temperature and moisture fields. The obtained results are compared with those predicted by FSDT, Reddy (2000). An excellent agreement is obtained between the present theory and Reddy theory for all values of power law index p and with or without the presence of the elastic foundation. It is important to observe that the stresses for a fully ceramic plate are not the same as that for a fully metal plate with elastic foundations. This is

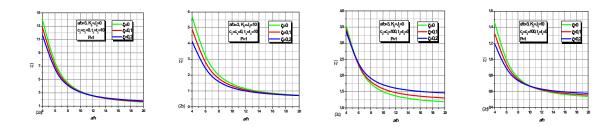


Fig. 2 Dimensionless center deflection \overline{w} versus side-to-thickness ratio a/h for imperfect and perfect FGM rectangular plates on elastic foundations

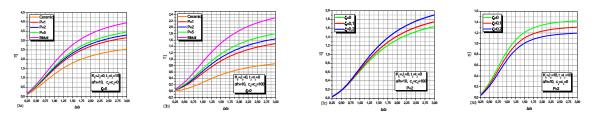


Fig. 3 Dimensionless center deflection \overline{w} versus plate aspect ratio b/a for imperfect and perfect FGM plates on elastic foundations

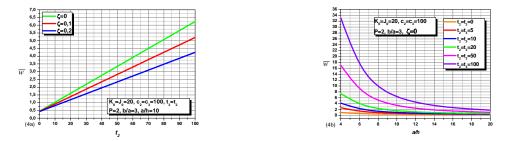
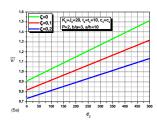


Fig. 4 Dimensionless center deflection \overline{w} of imperfect and perfect FGM plates on elastic foundations versus side-to-thickness ratio a/h for different values of temperatures



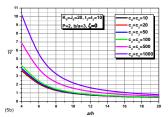
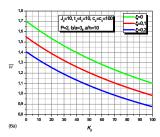


Fig. 5 Dimensionless center deflection \overline{w} of imperfect and perfect FGM plates on elastic foundations versus side-to-thickness ratio a/h for different values of moistures



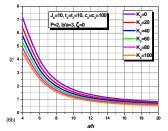
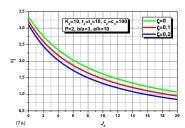


Fig. 6 Effect of elastic modulus of Winkler foundation K_0 on dimensionless center deflection \overline{W} of imperfect and perfect FGM plates



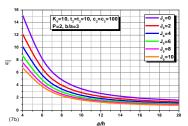
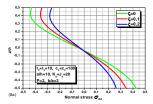


Fig. 7 Effect of Pasternak shear modulus parameter J_0 on dimensionless center deflection \overline{w} of FGM plates



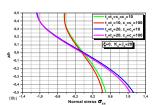


Fig. 8 Dimensionless axial stress $\overline{\sigma}_x$ through-the-thickness of imperfect and perfect FGM plates on elastic foundations for different values of moistures and temperatures.

because the plate here is affected with the inclusion of the temperature field. This has an important bearing in rocket-launch foundation structures as via judicious selection of the spatial structure of FGM, one can effectively prolong the serviceability life of the structure. In considering the results presented in tables 1 to 3, it should be noted that the quantity of unknown variables in the present formulation is four, whereas the number unknown function in FSDT and Reddy is five. It can be concluded that the present theory is not only accurate but also comparatively simple and quite

elegant in predicting the hygro-thermo-mechanical bending response of perfect and imperfect FGM plates resting on Winkler's or Pasternak's elastic foundations. Table 4 documents the effects of side-to-thickness ratio and elastic foundation parameters on the dimensionless deflection of imperfect FGM square plate under hygro-thermo-mechanical loads using the present model. It can be seen from Table 4 (and also Fig. 2) that the deflection decreases as the side-to-thickness ratio a/h increases and this, is due to the considerable effect of temperature and moisture on the extensional behavior of the plate comparatively the flexural behavior. This observation is also confirmed by Benferhat *et al.*.(2014). In addition, all displacements observed to decrease with the presence of the elastic foundations. The inclusion of the Winkler foundation parameter effectively generates greater magnitudes in solutions compared with solutions yielded from the inclusion of Pasternak foundation parameters.

A Figs. 2 and 3 shows the variation of the center deflection with aspect ratio for different types of imperfect FGM plates. The deflection of the metallic plate is found to be of the largest magnitude and that of the ceramic plate, of the smallest magnitude irrespective of the values of temperature, moisture, and elastic foundation parameters. All the plates with intermediate properties undergo corresponding intermediate values of center deflection. In addition, the deflection is increasing in the absence of the foundations and the effect of moisture parameter may be less than that of the temperature one. Figs. 4 and 5 shows the variation of the center deflection with side-to-thickness ratio for imperfect FG plates subjected to different parameters. The deflection increases as the moisture parameter increases, and the plate is very sensitive to the variation of elastic foundation parameters. In Figs. 6 and 7 show the effect of foundation parameters k_0 and J_0 on the variation of non-dimensional center deflection \overline{w} of imperfect FGM plate. The results show that the non-dimensional center deflection decreases with the increase of foundation parameters k_0 and J_0 , and the Winkler parameter k_0 has more effect on decreasing the

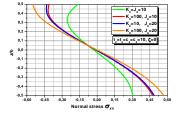


Fig. 9 Dimensionless axial stress σ_x through-the-thickness of perfect FGM plate for different values of elastic foundations parameters.

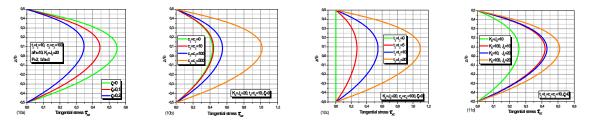


Fig. 10 Dimensionless shear stress $\bar{\tau}_{xz}$ through-the-thickness of FGM plates on elastic foundations

non-dimensional center deflection \overline{w} than the shear Pasternak parameter J_0 . Decreases of deflections indicate that increasing the foundation stiffness will certainly enhance the deformation rigidity of the plate. It is observed from the results; that effects of k_0 and J_0 on the non-dimensional center deflection \overline{w} of imperfect FGM plates are more significant for thicker plates. The longitudinal stress σ_x and the transverse shear stress τ_{xz} are plotted in Figs. 8, 9 and 10 through-the-thicknesses of FGM plates. Under the application of the uniform loading and elastic foundations, the longitudinal stress is tensile through the plate thickness and very sensitive to the variation of temperature and moisture parameters.

Figs. 8 and 9 depict the through-the-thickness distributions of the non-dimensional axial stress $\overline{\sigma}_x$ in the imperfect FGM rectangular plates. The effect of moisture and temperature fields on $\overline{\sigma}_x$ is shown in Fig. 8, while the effect of elastic foundations parameters is shown Fig. 9. As exhibited in these figures, the maximum compressive stresses occur at a point on the top surface and the maximum tensile stresses occur, of course, at a point on the bottom surface of the imperfect FGM plates. As illustrated in Fig. 9 the elastic foundation has a significant effect on the maximum values of the non-dimensional axial stress $\overline{\sigma}_x$.

Fig. 10 show that the maximum value occurs at a point above the mid-plane of the imperfect FGM plate and its magnitude increases with the inclusion of the temperature, moisture, and foundation parameters. The non-dimensional transverse shear stress $\bar{\tau}_{xz}$ is plotted through-the-thickness of imperfect FGM plates. It is evident that the maximum value of $\bar{\tau}_{xz}$ arises at a point above the mid-plane of the imperfect FGM plate and its magnitude increases with the inclusion of the foundation parameters.

5. Conclusion

In this paper, the hygro-thermo-mechanical bending response of perfect and imperfect FGM plates resting on elastic foundations by using the sinusoidal higher order shear deformation theory to plate is studied. The stress and displacement response of the plates have been analyzed under uniform loading. Dimensionless stresses and deflection are computed for perfect and imperfect FG plates subjected to the hygrothermal effects. Illustrating examples are carried out, with the most important conclusions that the effect of moisture concentration parameter on thermo- mechanical responses of the imperfect FGM plates is considerably different. The influence of moisture concentration as well as other parameter is highly significant. The influence of moisture concentration as well as other parameters is demonstrated to be significant. The bending response of the perfect and imperfect FG plate deteriorates considerably with the increase in temperature and moisture concentration. The results are verified with available results in the literature. It can be concluded that the proposed theory is accurate and simple in solving the hygro-thermo-mechanical bending behavior of perfect and imperfect functionally graded plates.

Acknowledgments

The authors thank the referees for their valuable comments.

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