

An investigation of the thermodynamic effect on the response of FG beam on elastic foundation

Rabbab Bachir Bouiadjra^{1,2a}, Attia Bachiri^{1a}, Samir Benyoucef^{*1}, Bouazza Fahsi^{3a} and Fabrice Bernard^{4b}

¹Material and Hydrology Laboratory, University of Sidi Bel Abbes, Faculty of Technology, Algeria

²Department of Civil Engineering, University Mustapha Stambouli of Mascara, Algeria

³Laboratoire de Modélisation et Simulation Multi-échelle, Université de Sidi Bel Abbès, Algeria

⁴Laboratoire de Génie Civil et Génie Mécanique, INSA de Rennes, France

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Abstract. This study presents an analytical approach to investigate the thermodynamic behavior of functionally graded beam resting on elastic foundations. The formulation is based on a refined deformation theory taking into consideration the stretching effect and the type of elastic foundation. The displacement field used in the present refined theory contains undetermined integral forms and involves only three unknowns to derive. The mechanical characteristics of the beam are assumed to be varied across the thickness according to a simple exponential law distribution. The beam is supposed simply supported and therefore the Navier solution is used to derive analytical solution. Verification examples demonstrate that the developed theory is very accurate in describing the response of FG beams subjected to thermodynamic loading. Numerical results are carried out to show the effects of the thermodynamic loading on the response of FG beams resting on elastic foundation.

Keywords: FGMs beams; Three-dimensional theory; undetermined integral forms; elastic foundation; thermodynamic effect

1. Introduction

Beam-type structures are typically used in various engineering applications such as aerospace, civil and mechanical. In recent decades, functionally graded materials (FGMs) are used in beam forms (Gul *et al.* 2019). It is well known that FGM offers certain opportunities such as uninterrupted variation of the material properties in one or more directions. This makes it possible to have structures with very interesting characteristics such as high resistance to temperature shocks, lower transverse shear stresses and high strength to weight ratio (Barati and Shahverdi 2016, Lal *et al.* 2017, Rezaiee-Pajand *et al.* 2018, Faleh *et al.* 2018, Avcar 2019, Ahmed *et al.* 2019, Balubaid *et al.* 2019, Dash *et al.* 2019, Rahmani *et al.* 2020, Kaddari *et al.* 2020). Therefore, understanding the behavior of the beams in various loading conditions is paramount in order to have a reliable design (Ghiasian *et al.* 2015). Stability analysis of beams under thermodynamic effect has great importance in their design process.

Analyze of FGM beams under various loading and considerations have been studied through the past decade by various investigators.

In the literature, many beam theories have been proposed to study the behavior of beam-type structures. The conventional beam theories, including the Euler and

Timoshenko beam theories have been extensively used in modeling the static and dynamic behavior of beams (Chen *et al.* 2018).

Caliò and Greco (2013) have studied the free vibration of the Timoshenko beam-columns on the elastic Pasternak foundations using the dynamic stiffness matrix method. Using the Euler–Bernoulli beam theory Fu *et al.* (2012) analyzed the dynamic stability and the thermo-piezoelectric buckling of FG beams subjected to steady heat conduction. Also, Ghiasian *et al.* (2013) studied the static and dynamic buckling of FG beam on non-linear elastic foundation subjected to uniform temperature rise across thickness. Using the Rayleigh-Ritz method, Pradhan and Chakraverty (2013) studied the free vibration response of Euler-Bernoulli and Timoshenko FG beams having different boundary conditions.

It should be noted that the Euler beam theory also called classical beam theory (CBT) ignores shear deformation and applies only to slender beams. Therefore, it underestimates deflection and overestimates buckling load and natural frequencies (Fahsi *et al.* 2019, Nguyen and Nguyen 2015). The Timoshenko beams theory or called the first order beam theory (FSBT) accounts for the shear deformation effect, but requires a shear correction factor. Sakar and Ganguli (2014) studied the free vibration of axially FG clamped Timoshenko beams. Ranjan *et al.* (2019) used first shear deformation theory for thermo-elastic free vibration analysis of functionally graded flat panel with temperature gradient along thickness. Draoui *et al.* (2019) employed also FSDT for static and dynamic behavior of nanotubes-reinforced sandwich plates.

In order to avoid the use of a shear correction factor and

*Corresponding author, Professor
E-mail: samir.benyoucef@gmail.com

^a Ph.D.

^b Professor

to take into account the transverse shear deformation for better accuracy, researchers have developed higher order shear theories for the study of FG beams (HSBT). However the efficiency of the HSBT depends on the appropriate choice of displacement field which is an interesting subject that attracted many research (Nguyen and Nguyen 2015, Mehar *et al.* 2018abcd, Dash *et al.* 2018, Ramteke *et al.* 2019, Singh *et al.* 2019, Sahu *et al.* 2020ab, Sahla *et al.* 2019, Batou *et al.* 2019, Salah *et al.* 2019, Mehar *et al.* 2019abc and 2020, Mehar and Panda 2020).

Tounsi and his co-workers have proposed several HSDT (higher order shear deformation theories) for the study and analysis of FG structures with or without elastic foundations (Bachir Bouiadjra *et al.* 2013 and 2018, Ait Atmane *et al.* 2017, Chaabane *et al.* 2019, Zarga *et al.* 2019, Berghouti *et al.* 2019, Boutaleb *et al.* 2019, Tlidji *et al.* 2019, Ahmed *et al.* 2019, Bourada *et al.* 2019, Medani *et al.* 2019, Boussoula *et al.* 2020, Tounsi *et al.* 2020, Refrafi *et al.* 2020). Giunta *et al.* (2013) studied the thermal static behavior of FG beams subjected to thermo-mechanical loading on the base of Carrera unified formulation. Şimşek (2010) have compared results of various HSDT of FG beams subjected to vibration. Mantari and Yarasca (2015) developed an efficient 4-unknown quasi-3D hybrid theory for the bending analysis of FG beams.

The behavior of beams on elastic foundations is an important area of research in engineering. It is necessary to take the beam-foundation-soil interaction into account in a simple way to serve properly to the purposes of the application (Avcar and Mohammed 2018). Therefore, many studies have been conducted. Yas *et al.* (2017) examined the free vibration of FG beams resting on Pasternak foundation using the Euler–Bernoulli theory and by means of Generalized Differential Quadrature (GDQ) method. Avcar and Mohammed (2018) presented an analytical solution for the vibration of the FG beam on Pasternak foundation and under different boundary conditions. Duy *et al.* (2014) presented an analytical formulation to obtain eigen solutions of the FG beams resting on elastic foundation. Zhong *et al.* (2016) analyzed force vibration of FG beams resting on elastic foundation under heat conduction. Tossapanon and Wattanasakulpong (2016) studied dynamic analysis of FG beam on two parameters elastic foundation. Bellal *et al.* (2020) investigated the buckling behavior of a single-layered graphene sheet resting on viscoelastic medium via nonlocal four-unknown integral model.

Studies on the thermodynamic behavior of FG structures are very limited. Through the literature we find only the works of Zenkour and Sobhy (2013), Bachiri *et al.* (2018) and Mekerbi *et al.* (2019) as new studies dealing with this behavior. Moreover, there is no work available in the literature related to thermodynamic behavior of FG beam on elastic foundation by employing a quasi-3D theory according to the knowledge of the authors. Addou *et al.* (2019) examined the effects of porosity on dynamic response of FG plates resting on Winkler/Pasternak/Kerr foundation using quasi 3D HSDT. Boukhelif *et al.* (2019) presented a simple quasi-3D HSDT for the dynamics analysis of FG thick plate on elastic foundation. Also, Boulefrakh *et al.* (2019) discussed the effect of parameters

of visco-Pasternak foundation on the bending and vibration properties of a thick FG plate. Zaoui *et al.* (2019) employed new 2D and quasi-3D shear deformation theories for free vibration of functionally graded plates on elastic foundations. Karami *et al.* (2019a) investigated the wave propagation of FG anisotropic nanoplates resting on Winkler-Pasternak foundation.

In this paper, thermodynamic behavior of FG beams based on Winkler–Pasternak foundation is studied by using an efficient analytical method. The equations of motion are derived using a quasi-3D theory for the case of plates but modified to make it applicable for the case of beams. The formulation takes both the thermodynamic effect through the thickness of the beam as well as the effect of the elastic foundation.

The mechanical characteristics of the FG beam are assumed to be varied across the thickness according to a simple exponential law distribution of the volume fraction of the constituents. Validation of the present method is demonstrating the good agreement between our results and those available in literature. A detailed parametric study is presented to show the effect of the different parameters on the thermodynamic response of FG beam.

2. Quasi-3D theory for functionally graded beams

Consider a functionally graded (FG) beam of thickness h and length L . The beam is assumed to rest on a Winkler-Pasternak elastic foundation. The mechanical characteristics of the beam are assumed to be varied according to thickness as (Zenkour and Sobhy 2013):

$$P(z) = P_m e^{\beta \left(\frac{z+1}{h} \right)^p}, \quad \beta = \ln \left(\frac{P_c}{P_m} \right) \quad (1)$$

$P(z)$ is the effective material properties like Young's modulus E , density ρ , and thermal expansion coefficient α . The subscripts m and c refer to metal and ceramic. p is the power law index.

2.1 Kinematics

The displacement field satisfying the conditions of transverse shear stresses (and hence strains) vanishing at $(x, y, \pm h/2)$ on the outer (top) and inner (bottom) surfaces of the beam, is given as follows:

$$\begin{aligned} u(x, z, t) &= u_0(x, t) - z \frac{\partial w_0}{\partial x} + f(z) \phi_x(x, t) \\ w(x, z, t) &= w_0(x, t) + g(z) \theta(x, t) \end{aligned} \quad (2)$$

u_0, w_0 , and ϕ_x are the three-unknown displacement of the mid-plane of the beam.

By considering that $\phi_x = \int \theta(x, t) dx$, the displacement fields mentioned above can be written as follows:

$$u(x, z, t) = u_0(x, t) - z \frac{\partial w_0}{\partial x} + k_1 f(z) \int \theta(x, t) dx \quad (3)$$

$$w(x, z, t) = w_0(x, t) + g(z)\theta(x, t)$$

The integrals defined in the above equations shall be resolved by a Navier type method and the displacement fields can be rewritten as:

$$u(x, z, t) = u_0(x, t) - z \frac{dw_0}{dx} + k_1 A' f(z) \frac{d\theta}{dx} \quad (4)$$

$$w(x, z, t) = w_0(x, t) + g(z)\theta(x, t)$$

where

$$g(z) = \frac{2}{15} \frac{df}{dz}, \quad k_1 = -\lambda^2; \quad A' = -\frac{I}{\lambda^2}; \quad \lambda = \frac{m\pi}{L} \quad (5a)$$

$$k_1 = \alpha^2 \quad (5b)$$

The coefficient A' is expressed according to the Navier type solution and they are given by:

$$A' = -\frac{1}{\alpha^2}, \quad (6a)$$

And

$$\alpha = \frac{m\pi}{a} \quad (6b)$$

It can be seen that the displacement field in Eqs. (3)-(4) contains only three unknowns u_0, w_0 and θ

The shape function $f(z)$ is given as follows:

$$f(z) = z - \frac{h}{\pi} \sin\left(\frac{\pi z}{h}\right) \quad (7)$$

The kinematic relations can be obtained as follows:

$$\begin{aligned} \varepsilon_x &= \varepsilon_x^0 - z k_x^b + f(z) k_x^\theta \\ \varepsilon_z &= g'(z) \varepsilon_z^0 \\ \gamma_{xz} &= (g(z) + k_1 A' f'(z)) \gamma_{xz}^s \end{aligned} \quad (8)$$

where

$$\begin{aligned} \varepsilon_0 &= \frac{du_0}{dx} \\ k_x^b &= -\frac{d^2 w_0}{dx^2}; \\ k_x^\theta &= k_1 A' \frac{d^2 \theta}{dx^2}; \\ \gamma_{xz}^s &= \frac{d\theta}{dx}; \\ \varepsilon_z^0 &= \theta(x, t) \text{ and } g'(z) = \frac{dg}{dz} \end{aligned} \quad (9)$$

2.2 Constitutive relations

By assuming that the material of FG beam obeys

Hooke's law, the stresses in the beam become:

$$\begin{Bmatrix} \sigma_x \\ \sigma_z \\ \tau_{xz} \end{Bmatrix} = \begin{bmatrix} Q_{11} & Q_{13} & 0 \\ Q_{13} & Q_{11} & 0 \\ 0 & 0 & Q_{66} \end{bmatrix} \begin{Bmatrix} \varepsilon_x - \alpha T \\ \varepsilon_z - \alpha T \\ \gamma_{xz} \end{Bmatrix} \quad (10a)$$

$$Q_{11} = \frac{E(z)}{(1-\nu^2)}; \quad Q_{13} = \nu Q_{11}, \quad Q_{66} = \frac{E(z)}{2(1+\nu)} \quad (10b)$$

where $(\sigma_x, \sigma_z, \tau_{xz})$ and $(\varepsilon_x, \varepsilon_z, \gamma_{xz})$ are the stress and strain components, respectively. $\alpha(z)$ is the coefficient of thermal expansion, and T is the distribution of the temperature load.

2.3 Equations of motion

Hamilton's principle is used herein to derive the equations of motion. The principle can be stated in analytical form as (Behera and Kumari 2018, Fenjan *et al.* 2019ab, Faleh *et al.* 2020):

$$\delta \int_0^T (\delta U + \delta K - \delta T) dt \quad (11)$$

where δU is the variation of strain energy and δK is the variation of potential energy; δT is the variation of kinetic energy.

The variation of strain energy of the beam can be stated as:

$$\delta U = \int_{-h/2}^{h/2} \int_0^L [\sigma_x \delta \varepsilon_x + \sigma_z \delta \varepsilon_z + \tau_{xz} \delta \gamma_{xz}] dx dz \quad (12)$$

Substituting Eqs. (8)-(10) into Eq. (12) and integrating through the thickness of the beam, Eq. (12) can be rewritten as

$$\delta U = \int_A \left\{ N_x \frac{d\delta u_0}{dx} - M_x^b \frac{d^2 \delta w_0}{dx^2} + k_1 A' M_x^s \frac{d^2 \delta \theta}{dx^2} + N_z \delta \theta + Q \frac{d\delta \theta}{dx} \right\} dA \quad (13)$$

where:

$$\begin{Bmatrix} N_x \\ M_x^b \\ M_x^s \end{Bmatrix} = \int_{-h/2}^{h/2} \sigma_x \begin{Bmatrix} 1 \\ z \\ f(z) \end{Bmatrix} dz \quad (14a)$$

$$Q = \int_{-\frac{h}{2}}^{\frac{h}{2}} \tau_{xz} (g(z) - k_1 A' f'(z)) dz \quad (14b)$$

$$N_z = \int_{-\frac{h}{2}}^{\frac{h}{2}} \sigma_z g'(z) dz \quad (14c)$$

Using Eq. (10) in Eq. (14), the stresses resultants, for the

present FG beam, can be related to the displacements as:

$$\begin{Bmatrix} N_x \\ M_x^b \\ M_x^s \end{Bmatrix} = \begin{Bmatrix} A & B & B^s \\ B & D & D^s \\ B^s & D^s & H^s \end{Bmatrix} \begin{Bmatrix} \varepsilon_x^0 \\ k_x^b \\ k_x^s \end{Bmatrix} - \begin{Bmatrix} N_x^T \\ M_x^{b^T} \\ M_x^{s^T} \end{Bmatrix} + \begin{Bmatrix} L \\ L^a \\ R \end{Bmatrix} \varepsilon_z^0, Q = A^s \gamma, \quad (15)$$

$$N_z = L \varepsilon_x^0 + L^a k_x^b + R k_x^s + R^a \varepsilon_z^0 \quad (16)$$

where A , B , D , etc., are the beam stiffness, defined by

$$\{A, B, B^s, D, D^s, H^s\} = \int_{-h/2}^{h/2} Q_{11} \{1, z, f(z), z^2, zf(z), f(z)^2\} dz \quad (17a)$$

$$\{L, L^a, R, R^a\} = \int_{-h/2}^{h/2} Q_{13} \{g'(z), zg'(z), g'(z)f(z), g'(z)^2\} dz \quad (17b)$$

$$A_s = \int_{-h/2}^{h/2} Q_{66} (g(z) - k_1 A' f'(z))^2 dz \quad (17c)$$

The stress and moment resultants, N_x^T , $M_x^{b^T}$, $M_x^{s^T}$ due to thermal loading are defined by

$$\begin{Bmatrix} N_x^T \\ M_x^{b^T} \\ M_x^{s^T} \end{Bmatrix} = \int_{-h/2}^{h/2} \frac{E(z)}{(1-\nu)} \begin{Bmatrix} 1 \\ z \\ f(z) \end{Bmatrix} \alpha T dz \quad (18)$$

$$N_z^T = \int_{-h/2}^{h/2} \frac{E(z)}{(1-\nu)} g'(z) \alpha T dz$$

The potential energy of the foundation and distributed load is expressed as:

$$\delta K = \int_{-h/2}^{h/2} \int_0^L \left(k_w w - k_s \frac{d^2 w}{dx^2} \right) \delta (w_0 + g(z)\theta) - q \delta (w_0 + g(z)\theta) dx dz \quad (19)$$

Also, the variation of the kinetic energy can be expressed as

$$\delta T = - \int_{-h/2}^{h/2} \int_0^L \left[\ddot{u} \delta u + (\ddot{w}_0 + g(z)\ddot{\theta}) \delta (w_0 + g(z)\theta) \right] \rho(z) dx dz \quad (20)$$

$$\delta T = - \int_0^L \left[\begin{aligned} & I_0 (\ddot{u}_0 \delta u_0 + \ddot{w}_0 \delta w_0) + I_1 \left(-\ddot{u}_0 \frac{dw_0}{dx} - \frac{d\ddot{w}_0}{dx} \delta u_0 \right) \\ & + I_2 \left(\frac{d\ddot{w}_0}{dx} \frac{d\delta \ddot{w}_0}{dx} \right) + J_0 (\ddot{w}_0 \delta \theta + \ddot{\theta} \delta w_0) \\ & + J_1 (K_1 A' \ddot{u}_0 \frac{d\delta \theta}{dx} + K_1 A' \frac{d\ddot{\theta}}{dx} \delta u_0) \\ & + J_2 (-K_1 A' \frac{d\ddot{w}_0}{dx} \frac{d\delta \theta}{dx} - K_1 A' \frac{d\ddot{\theta}}{dx} \frac{d\delta w_0}{dx}) \\ & + K_0 \ddot{\theta} \delta \theta + K_2 (K_1 A')^2 \frac{d\ddot{\theta}}{dx} \frac{d\delta \theta}{dx} \end{aligned} \right] dx \quad (21)$$

Where dot-superscript convention indicates the differentiation with respect to the time variable t ; $\rho(z)$ is the mass density; and $(I_0, I_1, J_0, J_1, I_2, J_2, K_0, K_2)$ are mass inertias defined as:

$$(I_0, I_1, I_2) = \int_{-h/2}^{h/2} \rho(z) (1, z, z^2) dz \quad (22a)$$

$$(J_0, J_1, J_2) = \int_{-h/2}^{h/2} (g(z), f(z), zf(z)) dz \quad (22b)$$

$$(K_0, K_2) = \int_{-h/2}^{h/2} (1, z, f, z^2, zf, f^2) dz \quad (22c)$$

Substituting the expressions for δU , δK and δT from Eqs. (13)-(19) and (21) into Eq. (11) and integrating by parts and collecting the coefficients of δu_0 , δw_0 and $\delta \theta$, the following equations of motion of the beam are obtained

$$\begin{aligned} \delta u_0 : \frac{dN_x}{dx} &= I_0 \ddot{u}_0 - I_1 \frac{d\ddot{w}_0}{dx} + J_1 K_1 A' \frac{d\ddot{\theta}}{dx} \\ \delta w_0 : \frac{d^2 M_x^b}{dx^2} + q - k_w w_0 + k_s \frac{d^2 w_0}{dx^2} &= I_0 \ddot{w}_0 + I_1 \frac{d\ddot{u}_0}{dx} \\ &- I_2 \frac{d^2 \ddot{w}_0}{dx^2} + J_0 \ddot{\theta} + J_2 K_1 A' \frac{d^2 \ddot{\theta}}{dx^2} \end{aligned} \quad (23)$$

$$\begin{aligned} \delta \theta : -K_1 A' \frac{d^2 M_x^s}{dx^2} - N_z + q g_0 - k_w w_0 g_0 + k_s \frac{d^2 w_0}{dx^2} g_0 \\ + \frac{dQ}{dx} = J_0 \ddot{w}_0 - J_1 K_1 A' \frac{d\ddot{u}_0}{dx} + J_2 K_1 A' \frac{d^2 \ddot{w}_0}{dx^2} + K_0 \ddot{\theta} \\ - K_2 (k_1 A')^2 \frac{d^2 \ddot{\theta}}{dx^2} \end{aligned}$$

By substituting Eqs. (15)-(16) into Eq. (23), the equations of motion can be expressed in terms of displacements (u_0, w_0, θ) as

$$\begin{aligned} \delta u_0 : A \frac{d^2 u_0}{dx^2} - B \frac{d^3 w_0}{dx^3} + B^s (K_1 A') \frac{d^3 \theta}{dx^3} + L \frac{d\theta}{dx} &= I_0 \ddot{u}_0 \\ - I_1 \frac{d\ddot{w}_0}{dx} + J_1 K_1 A' \frac{d\ddot{\theta}}{dx} \end{aligned} \quad (24a)$$

$$\begin{aligned} \delta w_0 : A \frac{d^3 u_0}{dx^3} - D \frac{d^4 w_0}{dx^4} + D^s (K_1 A') \frac{d^4 \theta}{dx^4} + L^a \frac{d^2 \theta}{dx^2} \\ - q - k_w w_0 + k_s \frac{d^2 w_0}{dx^2} = I_0 \ddot{w}_0 + I_1 \frac{d\ddot{u}_0}{dx} - I_2 \frac{d^2 \ddot{w}_0}{dx^2} \\ + J_0 \ddot{\theta} + J_2 K_1 A' \frac{d^2 \ddot{\theta}}{dx^2} \end{aligned} \quad (24b)$$

$$\begin{aligned} \delta \theta : -K_1 A' (B^s \frac{d^3 u_0}{dx^3} - D^s \frac{d^4 w_0}{dx^4} + H^s (K_1 A') \frac{d^4 \theta}{dx^4} + R \frac{d^2 \theta}{dx^2}) \\ - L \frac{du_0}{dx} + L^a \frac{d^2 w_0}{dx^2} - R K_1 A' \frac{d^2 \theta}{dx^2} - R^a \theta + A^s \frac{d^2 \theta}{dx^2} + q g_0 \\ - k_w w_0 g_0 + k_s \frac{d^2 w_0}{dx^2} g_0 = J_0 \ddot{w}_0 - J_1 K_1 A' \frac{d\ddot{u}_0}{dx} + J_2 K_1 A' \frac{d^2 \ddot{w}_0}{dx^2} \\ + K_0 \ddot{\theta} - K_2 (k_1 A')^2 \frac{d^2 \ddot{\theta}}{dx^2} \end{aligned} \quad (24c)$$

The variation of temperature is assumed to occur in the thickness direction according to a power law form. The

temperature field variation through the thickness is assumed to be (Zenkour and Sobhy (2013))

$$T(x, z, t) = \hat{t}(z) \hat{T}(x, t), \quad (25a)$$

where

$$\hat{t}(z) = T^- e^{\gamma \left(\frac{z+1}{h+2} \right)}, \quad \gamma = \ln \left(\frac{T^+}{T^-} \right), \quad 0 \leq \eta \leq \infty \quad (25b)$$

$$\hat{T}(x, z, t) = \bar{t} \sin(\lambda x) e^{i\omega t} \quad (25c)$$

where \bar{t} is arbitrary parameter and T^+ and T^- are the top and the bottom temperature.

In which η is the temperature exponent. Note that $\eta = 0$ represents a top surface temperature of the beam while $\eta = \infty$ represents a bottom surface temperature.

3. Analytical solution

In this paragraph, the Navier solution for simply supported beams will be used to solve the problem. The variables u_0 , w_0 and θ can be written by assuming the following variations:

$$\begin{pmatrix} u_0 \\ w_0 \\ \theta \end{pmatrix} = \sum_{m=1}^{\infty} \begin{pmatrix} U_m e^{i\omega t} \cos(\lambda x) \\ W_m e^{i\omega t} \sin(\lambda x) \\ X_m e^{i\omega t} \sin(\lambda x) \end{pmatrix} \quad (26)$$

where: U_m , W_m , X_m are arbitrary parameters to be determined, ω is the eigen frequency associated with m^{th} eigen mode, and $\frac{m\pi}{L}$. The transverse load q is also expanded in Fourier series as:

$$q(x) = \sum_{m=1}^{\infty} Q_m \sin\left(\frac{m\pi}{L}x\right) e^{i\omega t} \quad (27)$$

where Q_m is the load amplitude calculated from:

$$Q_m = \int_0^L q(x) \sin(\lambda x) \quad (28)$$

For the case of uniform distributed load, the coefficient Q_m is given as:

$$Q_m = \frac{4q_0}{m\pi} \quad (m=1, 3, 5, \dots) \quad (29)$$

Substituting Eqs. (26)-(27) into Eq. (24), the analytical solutions can be obtained by:

- For the free vibration problem

$$([K] - \omega^2 [M])\{\Delta\} = 0 \quad (30)$$

- For the case of the static problems

$$[K]\{\Delta\} = \{F\} \quad (31)$$

- For the case of the thermodynamic

$$([K] - \omega^2 [M])\{\Delta\} = \{F\} \quad (32)$$

where $[M]$ is the mass matrix, $[K]$ is the stiffness matrix, $\{\Delta\}$ is the displacement of the nodal value, and $\{F\}$ is the distributive force vector, and ω^2 is the eigen values of dynamic system (natural frequencies).

$$[K] = \begin{bmatrix} a_{11} & a_{12} & a_{13} \\ a_{12} & a_{22} & a_{23} \\ a_{13} & a_{23} & a_{33} \end{bmatrix}; \quad [M] = \begin{bmatrix} m_{11} & m_{12} & m_{13} \\ m_{12} & m_{22} & m_{23} \\ m_{13} & m_{23} & m_{33} \end{bmatrix}; \quad (33a)$$

$$\{\Delta\} = \begin{Bmatrix} U_m \\ W_m \\ X_m \end{Bmatrix}; \quad \{F\} = \begin{Bmatrix} -A^t \bar{t} \\ B^t \bar{t} + q_0 \\ -k_1 A' C^t \bar{t} \lambda^2 + D^t \bar{t} + q_0 g_0 \end{Bmatrix} \quad (33b)$$

$$\begin{aligned} a_{11} &= A \lambda^2 \\ a_{12} &= -B \lambda^3 \\ a_{13} &= k_1 A' B^s \lambda^3 - L \lambda \\ a_{22} &= D \lambda^4 + k_s \lambda^2 + k_w \\ a_{23} &= -k_1 A' D^s \lambda^4 + k_s g_0 \lambda^2 + k_w g_0 + L^a \lambda^2 \\ a_{33} &= (k_1 A')^2 H^s \lambda^4 + A^s \lambda^2 - 2k_1 A' R \lambda^2 \\ &\quad + k_s g_0^2 \lambda^2 + k_w g_0^2 + R^a \end{aligned} \quad (33c)$$

where $g_0 = g(0)$

$$\begin{aligned} m_{11} &= I_0 \\ m_{12} &= -I_1 \lambda \\ m_{13} &= k_1 A' J_1 \lambda \\ m_{22} &= I_0 + I_2 \lambda^2 \\ m_{23} &= J_0 - k_1 A' J_2 \lambda^2 \\ m_{33} &= K_2 (k_1 A')^2 \lambda^2 + K_0 \end{aligned} \quad (33d)$$

4. Results and discussion

In this section, numerical computations of the simply supported FG beams resting on elastic foundation by the present method are suggested for investigation.

In absence of works relating to FG beams subjected to thermodynamic loading in literature, the results of this model have been validated for the static case in the presence of a mechanical load and in the dynamic case for free vibration.

Numerical calculations are performed for a mixture made of Aluminum and Alumina, where the top region is Alumina rich and the bottom region is Aluminum rich with the following material properties:

Table 1 Comparisons of the mid-span non-dimensional deflection (\bar{w}) of an isotropic homogeneous beam on elastic foundations using various beam theories

Foundation parameters		L/h=120			L/h=15			L/h=5		
K_w	K_s	Ying <i>et al.</i> (2008)	Ait Atmane <i>et al.</i> (2015)	Present	Ying <i>et al.</i> (2008)	Ait Atmane <i>et al.</i> (2015)	Present	Ying <i>et al.</i> (2008)	Ait Atmane <i>et al.</i> (2015)	Present
			$\varepsilon_z \neq 0$	$\varepsilon_z \neq 0$		$\varepsilon_z \neq 0$	$\varepsilon_z \neq 0$		$\varepsilon_z \neq 0$	$\varepsilon_z \neq 0$
	0	1.3023	1.3009	1.3021	1.3153	1.3022	1.3105	1.4202	1.3133	1.4091
0	10	0.6448	0.6446	0.6448	0.6483	0.6532	0.6473	0.6745	0.7217	0.6730
	25	0.3661	0.3662	0.3661	0.3674	0.3796	0.3671	0.3767	0.4842	0.3763
	0	1.1806	1.1794	1.1803	1.1913	1.1817	1.1875	1.2773	1.2003	1.2688
10	10	0.6133	0.6131	0.6132	0.6165	0.6223	0.6156	0.6403	0.6945	0.6389
	25	0.3557	0.3557	0.3556	0.3568	0.3694	0.3566	0.3657	0.4755	0.3653
	0	0.6401	0.6398	0.6400	0.6434	0.6486	0.6425	0.6685	0.7177	0.6670
100	10	0.4256	0.4256	0.4255	0.4272	0.4380	0.4268	0.4388	0.5344	0.4383
	25	0.2829	0.2830	0.2828	0.2836	0.2981	0.2835	0.2894	0.4148	0.2891

- Metal (Aluminium, Al): $E_M = 70 \times 10^9$ N/m²; $\nu = 0.3$; $\rho_M = 2702$ kg/m³;

- Ceramic (Alumina, Al₂O₃): $E_C = 380 \times 10^9$ N/m²; $\nu = 0.3$; $\rho_C = 3960$ kg/m³;

In all examples, the foundation parameters are presented in the non-dimensional form of $k_w = K_w L^4 / EI$ and $k_p = K_p L^2 / EI$.

For simplicity, the following non-dimensional parameters are used in the numerical examples:

$$\bar{w} = \frac{100 E_c I}{q L^4} w \left(\frac{L}{2} \right); \bar{\omega} = \sqrt{\frac{\rho_c A L^4 \omega^2}{EI}}; K_w = \frac{k_w L^4}{E_c I}; K_s = \frac{k_s L^2}{E_c I}.$$

$$\bar{\sigma}_x = -\frac{10^{-3} E_c I}{L^3} \sigma_x \left(\frac{L}{2}, \frac{h}{2} \right); \bar{\tau}_{xz} = \frac{10^{-1} E_c I}{L^2} \tau_{xz} (0, 0);$$

where $I = \frac{b h^3}{12}$ (beam stiffness)

4.1 Comparison studies

The accuracy and efficiency of the present method to characterize well the free vibration and the static bending behaviors is firstly demonstrated.

Tables 1-2 present respectively the comparisons of the non-dimensional natural frequency and the mid-span deflection obtained from the present computational model with other reported methods (Ying *et al.* (2008), Ait Atmane *et al.* (2017)). Results are presented for different values of foundation parameters, and thickness-to-length ratio.

In general, good agreements are observed between the present results and the exact two-dimensional theory of elasticity by Ying *et al.* (2008) and shear and normal deformation beam theory of Ait Atmane *et al.* (2017).

Table 1 Comparisons of the mid-span non-dimensional deflection (\bar{w}) of an isotropic homogeneous beam on elastic foundations using various beam theories.

Another comparison is presented in Table 3 where the

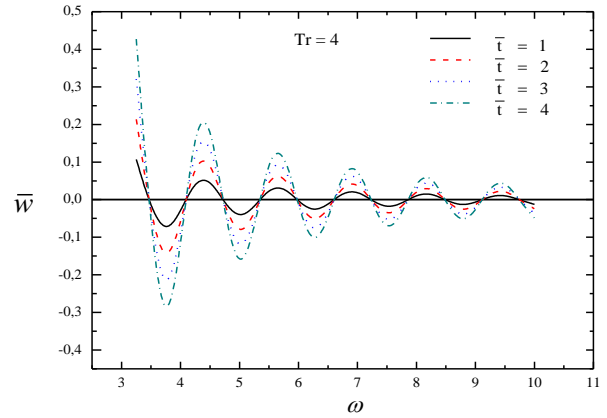


Fig. 1 Variation of deflection \bar{w} versus the natural frequency ω ($L/h = 5$, $t = 5$, $\eta = 1$, $T^- = 25$, $k = 1$, $K_w = 100$, $K_p = 10$)

results of present method are compared with those of Li *et al.* (2010) and Ould Larbi *et al.* (2015).

The results are presented in terms of displacements and stress for different values of the power index and thickness-to-length ratio.

In this case too, an excellent agreement between results is observed. The difference is found to be relatively small while the thickness-to-length $a/h=5$ compared to the other two solutions.

This is due to the fact that the two solutions are 2D theories, whereas the present is a quasi 3D theory so it takes into account the stretching effect.

4.2 Parametric studies

In this section, a series of parametric studies are conducted to investigate the effects of beam parameters, elastic foundation and thermal loading on the thermodynamic behavior of simply supported FG beam resting on elastic foundation.

Table 2 Comparisons of the non-dimensional-fundamental frequency ($\bar{\omega}$) parameter of an isotropic homogeneous beam on elastic foundations using various beam theories

Foundation parameters		L/h=120			L/h=15			L/h=5		
K_w	K_s/π^2	Ying <i>et al.</i> (2008)	Ait Atmane <i>et al.</i> (2017) $\varepsilon_z \neq 0$	Present $\varepsilon_z \neq 0$	Ying <i>et al.</i> (2008)	Ait Atmane <i>et al.</i> (2017) $\varepsilon_z \neq 0$	Present $\varepsilon_z \neq 0$	Ying <i>et al.</i> (2008)	Ait Atmane <i>et al.</i> (2017) $\varepsilon_z \neq 0$	Present $\varepsilon_z \neq 0$
0	0	3.1415	3.1421	3.1414	3.1323	3.1309	3.1329	3.0637	3.0484	3.0552
	1	3.7359	3.7363	3.7358	3.7278	3.7270	3.7278	3.6665	3.6598	3.6644
	2.5	4.2969	4.2972	4.2968	4.2889	4.2884	4.2887	4.2232	4.2249	4.2296
10^2	0	3.7482	3.7486	3.7482	3.7401	3.7393	3.7402	3.6788	3.6724	3.6769
	1	4.1436	4.1438	4.1438	4.1356	4.1350	4.1354	4.0720	4.0712	4.0758
	2.5	4.5823	4.5825	4.5822	4.5741	4.5738	4.5738	4.5028	4.5097	4.5153
10^4	0	10.0240	10.0240	10.0240	9.9958	10.0066	10.0059	7.3408	7.5525	7.7604
	1	10.0481	10.0481	10.0481	10.0197	10.0306	10.0299	7.3410	7.5525	7.7604
	2.5	10.0839	10.0839	10.0839	10.0552	10.0663	10.0656	7.3412	7.5525	7.7604

Table 3 Comparisons of the non-dimensional $\bar{w}, \bar{\sigma}_x, \bar{\tau}_{xz}$ of an FGM beam on using various beam theories

p	Theory	\bar{w}		\bar{u}		$\bar{\sigma}_x$		$\bar{\tau}_{xz}$	
		a/h=5	a/h=20	a/h=5	a/h=20	a/h=5	a/h=20	a/h=5	a/h=20
0	Li <i>et al.</i> (2010)	3.1653	2.8962	0.9402	0.2306	3.8018	15.0128	0.7321	0.7435
	Ould Larbi <i>et al.</i> (2015)	3.1653	2.8962	0.9406	0.2305	3.8018	15.0128	0.7330	0.7436
	Present	3.2986	2.9148	0.9293	0.2273	3.9572	15.5018	0.8006	0.7754
0.5	Li <i>et al.</i> (2010)	4.8285	4.4644	1.6603	0.4087	4.9922	19.7002	0.7493	0.7604
	Ould Larbi <i>et al.</i> (2015)	4.8285	4.4644	1.6608	0.4087	4.9476	19.6891	0.7501	0.7605
	Present	4.9851	4.4640	1.6089	0.3959	5.2021	20.3442	0.8427	0.8426
1	Li <i>et al.</i> (2010)	6.2594	5.8049	2.3045	0.5686	5.8834	23.2051	0.7321	0.7435
	Ould Larbi <i>et al.</i> (2015)	6.2594	5.8048	2.3052	0.5685	5.8066	23.1860	0.7329	0.7436
	Present	6.4055	5.7573	2.2123	0.5445	6.1287	23.9620	0.8006	0.7754
2	Li <i>et al.</i> (2010)	8.0676	7.4420	3.1134	0.7691	6.8823	27.0989	0.6696	0.6810
	Ould Larbi <i>et al.</i> (2015)	8.0676	7.4421	3.1146	0.7691	6.7591	27.0682	0.6704	0.6811
	Present	8.1941	7.3256	2.9753	0.7303	7.1607	27.9772	0.6711	0.5966
5	Li <i>et al.</i> (2010)	9.8280	8.8182	3.7089	0.9133	8.1104	31.8127	0.5896	0.6011
	Ould Larbi <i>et al.</i> (2015)	9.8280	8.8186	3.7128	0.9134	7.9252	31.7667	0.5903	0.6012
	Present	9.9849	8.6951	3.5761	0.8693	8.4172	32.8328	0.5067	0.4002
10	Li <i>et al.</i> (2010)	10.9381	9.6905	3.8860	0.9536	9.7119	38.1382	0.6456	0.6584
	Ould Larbi <i>et al.</i> (2015)	10.9381	9.6907	3.8898	0.9537	9.5285	38.0926	0.6465	0.6585
	Present	11.2077	9.6274	3.7888	0.9162	10.0728	39.3600	0.5473	0.4287

Fig. 1 displays the variation of deflection \bar{w} versus the natural frequency ω for various values of the arbitrary parameter \bar{t} . From this figure, it can be seen that the displacements are strongly influenced in the low frequency zone, that is to say at the level of the first modes of vibration. This influence is clearly noted for cases of $\bar{t} > 1$. While for the case where $\bar{t} = 1$, the displacements are little disturbed. Then, there is a gradual attenuation of the displacements to approach the zero value that is to say the stability of the beam for the high modes.

Fig. 2 presents the variation of the axial stress $\bar{\sigma}_x$ versus the natural frequency ω for different values of the arbitrary parameter \bar{t} .

It can be seen from the figure that, for all cases of the values of the arbitrary parameter \bar{t} , the stresses have a sinusoidal shape whatever the mode of vibration. The increase in the values of this parameter leads to an increase in the stresses.

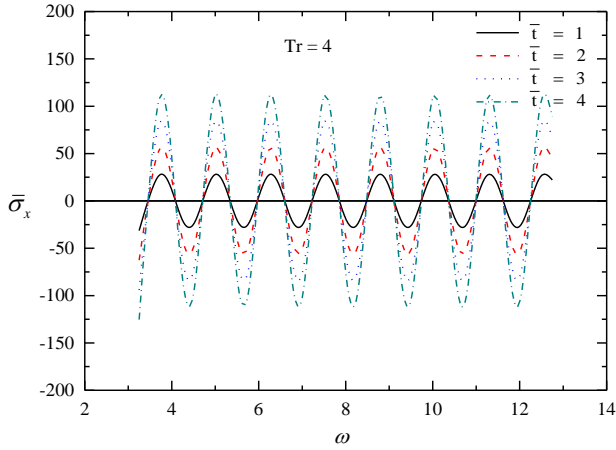


Fig. 2 Variation of the axial stress $\bar{\sigma}_x$ versus the natural frequency ω ($L/h = 5, t = 5, \eta = 1, T^- = 25, k = 1, K_w = 100, K_p = 10$)

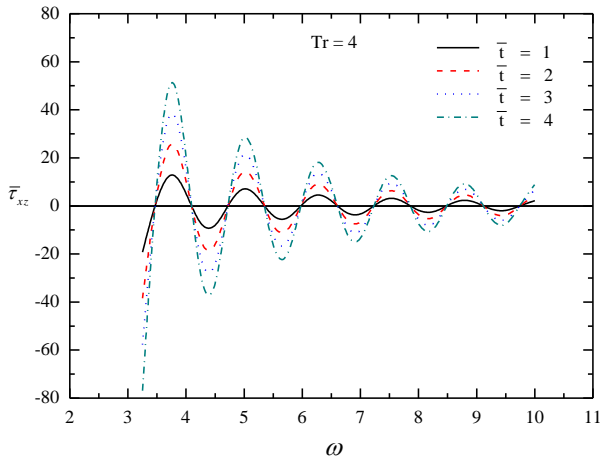


Fig. 3 Variation of the transverse shear stress $\bar{\tau}_{xz}$ versus the natural frequency ω ($L/h = 5, t = 5, \eta = 1, T^- = 25, k = 1, K_w = 100, K_p = 10$)

Fig. 3 depicts the variation of the transverse shear stress $\bar{\tau}_{xz}$ versus the natural frequency. The same observation as Fig. 1 is noted, namely that the tangential stresses are strongly affected during the first modes of vibration. Then, as the natural frequency of the plate rise, the transverse shear stress decrease until canceled (vanished).

Figs. 4-6 show the variation of the deflection \bar{w} , the axial stress $\bar{\sigma}_x$ and the transverse shear stress $\bar{\tau}_{xz}$ with the time t respectively for different values of the temperature exponent η .

From these figures, the following remarks can be made:

- Increasing in the temperature exponent η values leads to a reduction of the deflection and stresses (axial and shear transverse),
- The shape of the displacement and stresses is sinusoidal regardless of the value of the temperature exponent η or time.
- Deflections and stresses reach their maxima at time $t = 0, \frac{\pi}{2}, \pi, \dots$

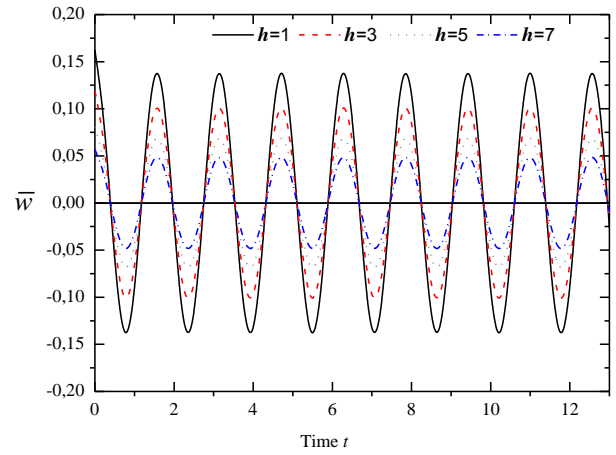


Fig. 4 Effect of time " t " on deflection " \bar{w} " of FG beam for different values of the temperature exponent η with ($L/h = 5, \omega = 4, \bar{t} = 2, T^- = 25, T_r = 4, k = 1, K_w = 100, K_p = 10$)

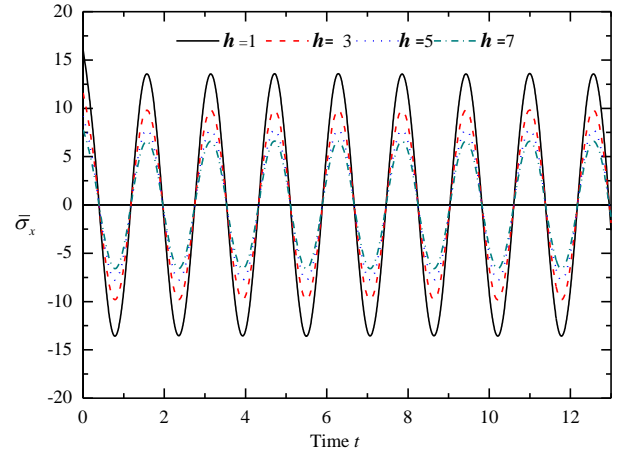


Fig. 5 Effect of time " t " on the longitudinal stress $\bar{\sigma}_x$ of FG beam for different values of the temperature exponent η with ($L/h = 10, \omega = 4, \bar{t} = 2, T^- = 25, T_r = 4, k = 1, K_w = 100, K_p = 10$)

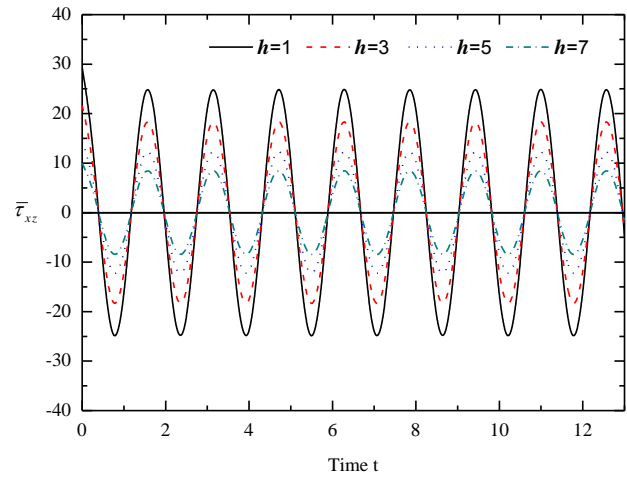


Fig. 6 Effect of time " t " on the transverse shear stress $\bar{\tau}_{xz}$ of FG beam for different values of the temperature exponent η with ($L/h = 5, \omega = 4, \bar{t} = 2, T^- = 25, T_r = 4, k = 1, K_w = 100, K_p = 10$)

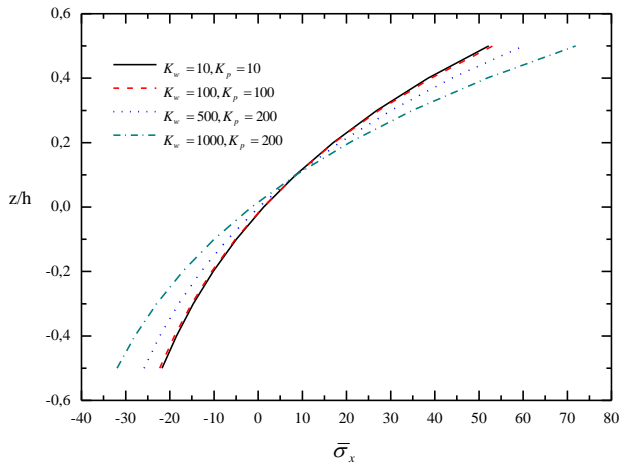


Fig. 7 Variation of the axial stress $\bar{\sigma}_x$ through the thickness of the FG beam resting on elastic foundations for different values of elastic foundations parameters ($L/h = 5, t = 3, \eta = 1, \omega = 4, \bar{t} = 2, T^- = 25, k = 1, T_r = 4$)

The effect of the elastic foundation parameters on the variation of the axial stresses across the thickness is plotted on Fig. 7.

According to this figure, it can be concluded that,

- Increasing in values of elastic foundation parameters leads to a increasing in values of the maximum axial stresses,
- The maximum compressive stresses occur at a point on the top surface and the maximum tensile stresses occur at a point on the bottom surface of the FG beam.

In the same way, the variation of the shear stresses through the thickness of the FG beam resting on elastic foundations for different values of parameter of these foundations is represented in Fig.8. It is observed that the increase in the parameters of the elastic foundation increases the transverse shear stresses.

5. Conclusion

In this paper, the thermodynamic response of FG beam resting on elastic foundation subjected to a harmonic temperature field across its thickness is analyzed by a refined quasi 3D theory. Influences of the temperature and elastic foundation parameters are investigated in great details. From the numerical examples it can be concluded that above mentioned parameters all affect the thermodynamic response of the FG beam especially in the first modes as it is the case of the variation of the displacement and the shear stress as a function of the arbitrary time parameter.

In addition, the present method is very efficient for the thermodynamic response analysis of FG beam resting on elastic foundation. This work can be extended in the future work for other type of materials (Arani and Kolahchi 2016, Daouadji 2017, Sharma *et al.* 2018abc, Ayat *et al.* 2018, Panjehpour *et al.* 2018, Narwariya *et al.* 2018, Mehar and Panda 2018abcd and 2019ab, Malikan 2018 and 2019, Othman and Fekry 2018, Hussain and Naeem 2019,

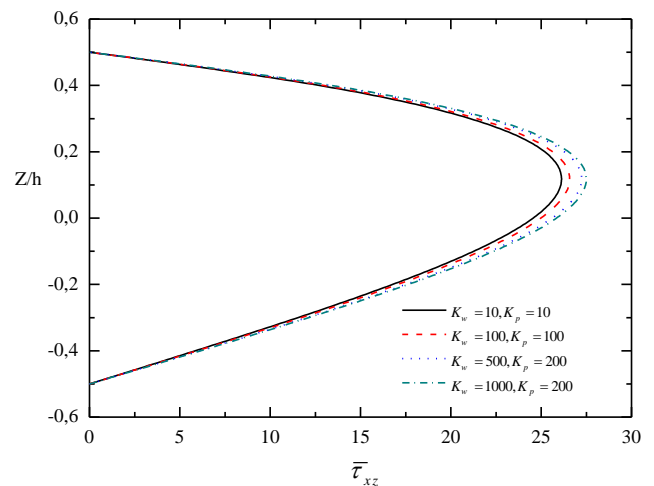


Fig. 8 Variation of the transverse shear stress $\bar{\tau}_{xz}$ through the thickness of the FG beam for different values of elastic foundations parameters ($L/h = 5, t = 3, \eta = 1, \omega = 4, \bar{t} = 2, T^- = 25, k = 1, T_r = 4$)

Belbachir *et al.* 2019 and 2020, Pandey *et al.* 2019, Semmah *et al.* 2019, Hussain *et al.* 2019, Selmi 2019, Bensattallah *et al.* 2019, Abualnour *et al.* 2019, Alimirzaei *et al.* 2019, Karami *et al.* 2019bc, Adda Bedia *et al.* 2019, Timesli 2020, Asghar *et al.* 2020, Taj *et al.* 2020, Matouk *et al.* 2020, Al-Maliki *et al.* 2020, Khorasani *et al.* 2020, Bourada *et al.* 2020, Bousahla *et al.* 2020, Chikr *et al.* 2020, Bisen *et al.* 2020).

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