Robust quasi 3D computational model for mechanical response of FG thick sandwich plate

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Abstract. This paper aims to develop a quasi-3D shear deformation theory for the study of bending, buckling and free vibration responses of functionally graded (FG) sandwich thick plates. For that, in the present theory, both the components of normal deformation and shear strain are included. The displacement field of the proposed model contains undetermined integral terms and involves only four unknown functions with including stretching effect. Using Navier's technique the solution of the problem is derived for simply supported sandwich plate. Numerical results have been reported, and compared with those available in the open literature were excellent agreement was observed. Finally, a detailed parametric study is presented to demonstrate the effect of the different parameters on the flexural responses, free vibration and buckling of a simply supported sandwich plates.

Keywords: Quasi 3D solution; FG sandwich plate; bending; buckling; vibration; stretching effect

1. Introduction

FGMs are heterogeneous materials in which the properties vary from one surface to the other, continuously and gradually. The remarkable benefits offered by these materials compared to conventional materials and the demands for overcoming technical challenges have prompted an increased application of FGM structures (Ebrahimi and Reza Barati (2017)).

FGM plates are widely used in various engineering applications such as mechanical and civil engineering. These structures undergo different types of dynamic loads, bending and buckling. The analysis of these structures against these loads must be carried out to ensure safety and uninterrupted operation.

Sandwich structures are one of FGM plate type which have been the subject of more attention and are nowadays widely used in various types of engineering. This is due to its excellent advantages over the monolithic solid structure. FG Sandwich structures present improved characteristics such as higher stiffness and strength to weight ratio, long fatigue life and thermal resistances, etc. (Al-shujairi and Çağrı (2018)).

So far, many bending, buckling and vibration studies on FG structures such as plates, sandwich, beams and functionally graded-carbon nanotubes (FG-CNT) have been

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Copyright © 2019 Techno-Press, Ltd. http://www.techno-press.com/journals/sem&subpage=7 reported in several literatures (Ahmed 2014, Ait Amar et al. 2014, Akavci 2016, Kolahchi et al. 2016, Madani et al. 2016, Kolahchi et al. 2017a, Hajmohammad et al. 2017, Hosseini and Kolahchi 2018, Bourada et al. 2018 and 2019).

In order to study FG plate the classic plate theory (CPT) which neglected transverse shear stress and transverse normal stress was used. This theory gives good results for thin plates. Several researchers have used this theory in their research investigations such as (Kolahchi and Arani 2016, Bilouei *et al.* 2016, Zamanian *et al.* 2017).

Zenkour (2005a) and Zenkour and Alghamdi (2010) used the CPT for the study of the bending behavior of functionally graded sandwich plates.

Based on the CPT and nonlocal elasticity theory Shahsavari *et al.* (2017) studied the dynamic deflections of viscoelastic orthotropic nanoplates under moving load embedded within visco-Pasternak substrate and hygrothermal environment.

In the case of thick and moderately thick plates, the CPT gives inaccurate results. First order shear deformation (FSDT) proposed by Reissner (1945) and Mindlin (1951) includes the effect of transverse shear deformation. This theory has been widely used in several research publications such as (Kolahchi 2017, Zarei *et al.* 2017, Hajmohammad *et al.* 2018a, Amnieh *et al.* 2018)

Meksi *et al.* (2015) have analyzed the bending and free vibration of FG plate on elastic foundation by developing a new first-order shear deformation theory (NFSDT). Thai H.T. and Choi D.H. (2013) have developed a simple FSDT for the bending and free vibration analysis of functionally

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graded plates. Also, Thai H.-T et al. (2014) developed a new FSDT for FG sandwich plates composed of FG face sheets and an isotropic homogeneous core. This theory has been developed in a way to eliminate the use of the shear correction factor. Karami et al. (2018a) studied the wave propagation in mounted nanoplates made of porous functionally graded materials by using the first-order shear deformation theory and nonlocal elasticity theory. Also, Karami and Janghorban (2019) used the Timoshenko beam model and the nonlocal strain gradient theory to study the vibration of porous nanotubes having thickness and material terms varying along the length. She Gui-Lin et al. (2018, 2019) presented an analytic model based on the nonlocal strain gradient theory for porous FG nanotubes to analyze the wave propagation and nonlinear bending behavior.

However, the FSDT depends on a shear correction factor which is difficult to estimate for composite materials. For this reason and to overcome this shortcoming, various higher order shear deformation theories (HSDT) have been developed.

Neves *et al.* (2012) have developed a hyperbolic theory counting for HSDT for static analysis of functionally graded sandwich plates. Nguyen *et al.* (2014) presented a trigonometric shear deformation theory to study the static, buckling and free vibration of two types of FG sandwich plates.

Karami *et al.* (2018b) presented a solution based on a second-order shear deformation theory in conjunction with nonlocal strain gradient theory for the wave propagation analysis of nanoplate made of temperature-dependent FG porous materials rested on Winkler–Pasternak foundation under in-plane magnetic field. Based on the same theory, Karami *et al.* (2018c) studied the thermal buckling of embedded sandwich piezoelectric nanoplates with functionally graded core.

Based on an n-order shear deformation theory, Xiang *et al.* (2011) presented a solution for free vibration of FG and composite sandwich plates. Also, using this theory and the meshless method, Xiang *et al.* (2013) analyzed the free vibration of FG sandwich plates. Alipour and Shariyat (2014) studied the stress and deformation analysis of functionally graded annular sandwich plates subjected to non-uniform normal and/or shear tractions. The high order shear deformation theory has also attracted the attention of other researchers (Kolahchi and Cheraghbak, 2017, Kolahchi 2017, Kolahchi *et al.* 2017b, Golabchi *et al.* 2018, Hajmohammad *et al.* 2018b, Petrov *et al.* 2018, Fakhar and Kolahchi 2018 and Hosseini and Kolahchi 2018).

These theories are cumbersome and costly in calculus due to involving many unknowns (Gupta and Talha (2017)).

Recently, new refined HSDT theories have been developed by researchers with low variables. Therefore, several works on FG sandwich plates have been developed on the basis of this theory (see references (Abdelaziz *et al.* 2011, Houari *et al.* 2011, Hadji *et al.* 2011, Merdaci *et al.* 2011, Bourada *et al.* 2012, Tounsi *et al.* 2013, Taibi *et al.* 2015, Li *et al.* 2016). Abdelaziz *et al.* (2017) presented a hyperbolic shear deformation theory for bending, buckling and vibration of PFG sandwich plate with various boundary conditions.

Shahsavari *et al.* (2018a) used the higher-order nonlocal strain-gradient model to analyze the vibration of singlelayer graphene sheets (SLGSs) in hygrothermal environment. Thermal buckling behavior of FG porous nanobeam resting on Kerr foundation and integrated with piezoelectric has been investigated by Karami *et al.* (2018d). The nonlocal higher-order shear deformation beam theory was used. Karami *et al.* (2018 e) proposed a new size-dependent higher-order shear deformation theory for the wave dispersion in anisotropic doubly-curved nano shells. She *et al.* (2017) investigated the thermal buckling and post-buckling behavior of FG porous nanotubes using a refined beam theory. Karami *et al.* (2018f) used the refined HSDT to model the buckling behavior of FG nanoplate.

The two theories FSDT and HSDT are based on a key assumption which states that the transverse displacement through the thickness of the plate is constant. This led to neglecting the thickness stretching. However, this hypothesis is inadequate for thick plates (BachirBouiadjra *et al.* 2018)

Thus, needs exist for the development of quasi-3D HSDTs. In quasi 3D solutions, the stretching effect is naturally taken into account since the displacement is expanded as a higher-order variation through the thickness of the plate.

Mantari and Soares (2014) presented a solution including the thickness stretching effect with 5 unknown for the bending analysis of functionally graded single-layer and sandwich plates. Hamidi et al. (2015) investigate the thermomechanical bending response of functionally graded sandwich plates by using a theory with 5-unknowns and stretching effect. Bessaim et al. (2013) studied free vibration of functionally sandwich plates with FG face sheets based on higher-order shear and normal deformation theory with five unknown displacement functions. Hebali et al. (2014) proposed a new quasi-three-dimensional (3D) hyperbolic shear deformation theory for the bending and free vibration analysis of FGM plate. Mahmoudi et al. (2017) presented a refined 3D shear deformation theory for thermo-mechanical analysis of functionally graded sandwich plates resting on a Pasternak foundation.

Shahsavari *et al.* (2018b) presented a quasi-3D hyperbolic theory for the free vibration analysis of FG porous plates resting on different elastic foundations. Shahsavari *et al.* (2018c) used a new polynomial quasi-3D shear deformation theory in conjunction with the Eringen nonlocal differential model (ENDM) to study the size-dependent shear buckling force of FG materials.

Karami *et al.* (2018 g and h) develop a new sizedependent quasi-3D plate theory for the wave dispersion analysis of functionally graded nanoplates while resting on an elastic foundation and under the hygrothermal environment and for mechanical analysis of anisotropic nano-particles Also, Karami *et al.* (2019) present an accurate analysis of nanoshell structures by combining the HSDT with stretching effects and the Bi-Helmholtz nonlocal strain gradient elasticity theory (B-H-NSGT).

Recently, Tounsi and his co-workers (Meksi et al. (2019) Sekkal et al. (2017), Menasria et al. (2017), Bouhadra et al. (2018), Ait Sidhoum et al. (2017,2018), Bourada et al. (2016), Mahmoudi et al. (2018)) proposed a

new displacement field which using undetermined integral terms. This new kinematic reduces the number of unknowns and equations of motion compared with other theories of the same nature.

This paper aims to improve this theory by including the stretching effect to study the bending, buckling, and free vibration of FG sandwich thick plate. The highlight of the proposed theory is that involves only four unknowns which is a reduced number of variables and governing equations than the conventional quasi-3D theories, but its solutions compare well with 3D and quasi-3D solutions.

In addition, it does not require a shear correction factor as in the case of FSDT and its scope is wider than the CPT since the CPT applies only to the case of thin plates. Equations of motion are obtained from Hamilton's principle. Analytical solutions of simply supported FG sandwich plates are presented. To check the validity of the present quasi 3D solution, the obtained results are compared with available data in literature. Parametric study is performed to show the influences many factors on the bending, buckling and free vibration of FGM sandwich plates.

2. Problem formulation

Consider a rectangular plate with length a, width b and uniform thickness h. The plate is assumed to be subjected to a transverse mechanical load at the top surface and a compressive in-plane load on the mid-plane of the plate. Three different types of FG plates are considered:

2.1 Type A: Isotropic FG plates

The plate of Type A is graded from metal at its bottom surface to ceramic at the top one (Fig. 1). The volume fraction of ceramic material Vc is given as follows is given as follows (Bourada *et al.* (2018), Attia *et al.* (2018), Bousahla *et al.* (2016)):

$$V_c = \left(\frac{2z+h}{2h}\right)^p \tag{1}$$

where p is the power-law index, which is positive and $z \in [-h/2, h/2]$.

2.2 Type B: Sandwich plates with FG core

The core of this type is graded from metal to ceramic. The bottom face is made of isotropic metal, whereas the top face is isotropic ceramic. The vertical positions of the bottom and top surfaces and of two interfaces between the layers are denoted by $h_0 = -h/2$, h_1 , h_2 , $h_3 = h/2$.respectively. h_1 , h_2 vary according the thickness ratio of layers. The volume fraction functions of ceramic phase $V_c^{(j)}$ defined by Nguyen *et al.* (2014).

$$\begin{bmatrix} V_c^{(j)}(z) = 0 & \text{for } z \in [h_0, h_1] \end{bmatrix}$$
(2a)

$$\begin{cases} V_{c}^{(j)}(z) = \left(\frac{z - h_{1}}{h_{2} - h_{1}}\right)^{r} & \text{for } z \in [h_{1}, h_{2}] \end{cases}$$
(2b)

$$\left[V_{c}^{(j)}(z) = 1 \right]$$
 for $z \in [h_{2}, h_{3}]$ (2c)



Fig. 1 Geometry of functionally graded plates

The variation of ceramic material through the plate thickness for (1-2-1) sandwich plate of Type B is displayed in fig. 1a.

2.3 Type C: Sandwich plates with FG faces

The faces of this type are graded from metal to ceramic. The core is made of isotropic ceramic. The volume fraction functions of ceramic phase $V_c^{(j)}$ given by (Nguyen *et al.* 2014):

$$V_{c}^{(j)}(z) = \left(\frac{z - h_{0}}{h_{1} - h_{0}}\right)^{p} \qquad \text{for } z \in [h_{0}, h_{1}]$$
(3a)

$$\begin{cases} V_c^{(j)}(z) = 1 & \text{for } z \in [h_1, h_2] \\ \end{cases}$$
(3b)

$$\left| V_c^{(j)}(z) = \left(\frac{z - h_3}{h_2 - h_3} \right) \qquad \text{for } z \in \left[h_2, h_3 \right]$$
(3c)

The variation of ceramic material through the plate thickness for (1-2-1) sandwich plate of Type C is displayed in Fig.1b.

2.4 Kinematics and strains

Based on a new inverse trigonometric shear deformation theory, the following displacement field is assumed (AitSidhoum *et al.* 2018):

$$u(x, y, z, t) = u_0(x, y, t) - z \frac{\partial w_0}{\partial x} + k_1 f(z) \int \theta(x, y, t) dx \quad (4a)$$

$$v(x, y, z, t) = v_0(x, y, t) - z \frac{\partial w_0}{\partial y} + k_2 f(z) \int \theta(x, y, t) dy \quad (4b)$$

$$w(x, y, z, t) = w_0(x, y, t) + g(z) \ \theta(x, y, t)$$
(4c)

Where $u_0; v_0; w_0; \theta$; are four unknown displacements of the mid-plane of the plate.

The coefficient k_1 and k_2 depends on the geometry. It can be seen that the kinematic in Eq. (4) introduces only four unknowns $(u, v_0, w_0 \text{ and } \theta)$ with considering the thickness stretching effect.

In this work, the present quasi - 3D HSDT is obtained by setting (Nguyen et al. 2014):

$$f(z) = h \arctan\left(\frac{rz}{h}\right) - \frac{16rz^3}{3h^2(r^2 + 4)}$$
 (5)

The strain- displacement expressions, based on the formulation, are written under following compact form:

$$\begin{cases} \varepsilon_{x} \\ \varepsilon_{y} \\ \gamma_{xy} \end{cases} = \begin{cases} \varepsilon_{x}^{0} \\ \varepsilon_{y}^{0} \\ \gamma_{xy}^{0} \end{cases} + z \begin{cases} k_{x}^{b} \\ k_{y}^{b} \\ k_{xy}^{b} \end{cases} + f(z) \begin{cases} k_{x}^{s} \\ k_{y}^{s} \\ k_{xy}^{s} \end{cases}$$
(6)

$$\begin{cases} \gamma_{yz} \\ \gamma_{xz} \end{cases} = f'(z) \begin{cases} \gamma_{yz}^{0} \\ \gamma_{yz}^{0} \end{cases} + g(z) \begin{cases} \gamma_{yz}^{1} \\ \gamma_{xz}^{1} \end{cases} \varepsilon_{z} = g'(z) \varepsilon_{z}^{0} \end{cases}$$

where

$$\begin{cases} \varepsilon_{x}^{0} \\ \varepsilon_{y}^{0} \\ \gamma_{xy}^{0} \end{cases} = \begin{cases} \frac{\partial u_{0}}{\partial x} \\ \frac{\partial v_{0}}{\partial x} \\ \frac{\partial u_{0}}{\partial y} + \frac{\partial v_{0}}{\partial x} \end{cases}, \quad \begin{cases} k_{x}^{b} \\ k_{y}^{b} \\ k_{xy}^{b} \end{cases} = \begin{cases} -\frac{\partial^{2} w_{0}}{\partial x^{2}} \\ -\frac{\partial^{2} w_{0}}{\partial y^{2}} \\ -2\frac{\partial^{2} w_{0}}{\partial x \partial y} \end{cases}, \quad (7a)$$
$$\begin{cases} k_{x}^{s} \\ k_{y}^{s} \\ k_{yy}^{s} \end{cases} = \begin{cases} k_{1}\theta \\ k_{2}\theta \\ k_{1}\frac{\partial}{\partial y}\int \theta \, dx + k_{2}\frac{\partial}{\partial x}\int \theta \, dy \end{cases},$$

$$g(z) = \frac{1}{9} \frac{\partial f(z)}{\partial z}, \begin{cases} \gamma_{yz}^{0} \\ \gamma_{xz}^{0} \end{cases} = \begin{cases} k_{2} \int \theta \, dy \\ k_{1} \int \theta \, dx \end{cases}, \begin{cases} \gamma_{yz}^{1} \\ \gamma_{xz}^{1} \end{cases} = \begin{cases} \frac{\partial \theta}{\partial y} \\ \frac{\partial \theta}{\partial x} \end{cases}, \\ \frac{\partial \theta}{\partial x} \end{cases},$$
(7b)
$$\mathcal{E}_{z}^{0} = \theta \text{ and } g'(z) = \frac{dg(z)}{dz}$$

The integrals presented in the above equations shall be resolved by a Navier type method and can be expressed as follows:

$$\frac{\partial}{\partial y} \int \theta \, dx = A' \frac{\partial^2 \theta}{\partial x \partial y}, \quad \frac{\partial}{\partial x} \int \theta \, dy = B' \frac{\partial^2 \theta}{\partial x \partial y},$$

$$\int \theta \, dx = A' \frac{\partial \theta}{\partial x}, \quad \int \theta \, dy = B' \frac{\partial \theta}{\partial y}$$
(8)

Where the coefficients A' and B' are considered according to the type of solution employed, in this case via Navier method .Therefore, A', B', k_1 and k_2 are expressed as follows:

$$A' = -\frac{1}{\alpha^2}, \quad B' = -\frac{1}{\beta^2}, \quad k_1 = -\alpha^2, \quad k_2 = -\beta^2 \quad (9)$$

¬ (

Where α and β are defined in expression 27. The linear constitution relations are given below:

$$\begin{cases} \sigma_{x} \\ \sigma_{y} \\ \sigma_{z} \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{cases} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & 0 & 0 & 0 \\ C_{12} & C_{22} & C_{23} & 0 & 0 & 0 \\ C_{13} & C_{23} & C_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & C_{66} & 0 & 0 \\ 0 & 0 & 0 & 0 & C_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & C_{44} \end{bmatrix} \begin{bmatrix} \varepsilon_{x} \\ \varepsilon_{y} \\ \varepsilon_{z} \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{cases}$$
(10)

where C_{ij} are the three- dimensional elastic constants defined by:

$$C_{11} = C_{22} = C_{33} = \frac{(1-v)E(z)}{(1-2v)(1+v)},$$
 (11a)

$$C_{12} = C_{13} = C_{23} = \frac{v E(z)}{(1-2v)(1+2v)},$$
 (11b)

$$C_{44} = C_{55} = C_{66} = \frac{E(z)}{2(1+\nu)}$$
 (11c)

2.5 Equation of motion

Hamilton's principle is herein employed to determine the equation of motion:

$$0 = \int_{0}^{t} (\delta U + \delta \mathbf{V} - \delta K) dt$$
(12)

Where δU is the variation of strain energy, δV is the variation of the external work done by external load applied to the place, and δK is the variation of kinetic energy. The variation of strain energy of the plate is expressed by:

$$\delta U = \int_{V} \begin{bmatrix} \sigma_{x} \delta \varepsilon_{x} + \sigma_{y} \delta \varepsilon_{y} + \sigma_{z} \delta \varepsilon_{z} + \tau_{xy} \delta \gamma_{xy} \\ + \tau_{yz} \delta \gamma_{yz} + \tau_{xz} \delta \gamma_{xz} \end{bmatrix} dV$$

$$= \int_{\Omega} \begin{bmatrix} N_{x} \delta \varepsilon_{x}^{0} + N_{y} \delta \varepsilon_{y}^{0} + N_{z} \delta \varepsilon_{z}^{0} + N_{xy} \delta \gamma_{xy}^{0} \\ + M_{x}^{b} \delta k_{x}^{b} + M_{y}^{b} \delta k_{y}^{b} + M_{xy}^{b} \delta k_{xy}^{b} + M_{x}^{s} \delta k_{x}^{s} \\ + M_{y}^{b} \delta k_{y}^{s} + M_{xy}^{s} \delta k_{xy}^{s} + Q_{yz}^{s} \delta \gamma_{yz}^{0} + S_{yz}^{s} \delta \gamma_{yz}^{1} \end{bmatrix} dA$$
(13)

Where A is the top surface and the stress resultants N, M, S and Q are defined by

$$(N_{i}, M_{i}^{b}, M_{i}^{s}) = \int_{-h/2}^{h/2} (1, z, f) \sigma_{i} dz, \quad (i = x, y, xy);$$

$$N_{z} = \int_{-h/2}^{h/2} g'(z) \sigma_{z} dz$$

$$(14a)$$

$$(S_{xz}^{s}, S_{yz}^{s}) = \int_{-h/2}^{h/2} g(z)(\tau_{xz}, \tau_{yz}) dz,$$

$$(Q_{xz}^{s}, Q_{yz}^{s}) = \int_{-h/2}^{h/2} f'(z)(\tau_{xz}, \tau_{yz}) dz,$$
(14b)

The variation of work done by in – plane loads is given by:

$$\delta V = -\int_{A} \overline{N} \, \delta w \, dA - \int_{A} q \, \delta w \, dA \tag{15}$$

with

$$\overline{N} = \left[N_x^0 \frac{\partial^2 w}{\partial x^2} + 2 N_{xy}^0 \frac{\partial^2 w}{\partial x \partial y} + N_y^0 \frac{\partial^2 w}{\partial y^2} \right]$$
(16)

Where (N_x^0, N_y^0, N_{xy}^0) are in – plane applied loads.

The variation of kinetic energy of the plate can be written as

$$\begin{split} \delta K &= \int_{V} \left[\dot{u} \,\delta \,\dot{u} + \dot{v} \,\delta \,\dot{v} + \dot{w} \,\delta \,\dot{w} \right] \rho(z) \, dV \\ &= \int_{A} \left\{ I_{0} \left[\dot{u}_{0} \delta \dot{u}_{0} + \dot{v}_{0} \delta \dot{v}_{0} + \dot{w}_{0} \delta \dot{w}_{0} \right] - I_{1} \left(\dot{u}_{0} \frac{\partial \delta \dot{w}_{0}}{\partial x} + \frac{\partial \dot{w}_{0}}{\partial x} \,\delta \,\dot{u}_{0} + \dot{v}_{0} \frac{\partial \delta \dot{w}_{0}}{\partial y} + \frac{\partial \dot{w}_{0}}{\partial y} \,\delta \,\dot{v}_{0} \right) \right. \\ &+ J_{1} \left(\left(k_{1} \, A^{\prime} \right) \left(\dot{u}_{0} \frac{\partial \delta \dot{\theta}}{\partial x} + \frac{\partial \dot{\theta}}{\partial x} \,\delta \,\dot{u}_{0} \right) + \left(k_{2} \, B^{\prime} \right) \left(\dot{v}_{0} \frac{\partial \delta \dot{\theta}}{\partial y} + \frac{\partial \dot{\theta}}{\partial y} \,\delta \,\dot{v}_{0} \right) \right) \\ &+ J_{1}^{st} (\dot{w}_{0} \delta \dot{\theta} + \dot{\theta} \delta \dot{w}_{0}) + I_{2} \left(\frac{\partial \dot{w}_{0}}{\partial x} \frac{\partial \delta \,\dot{w}_{0}}{\partial x} + \frac{\partial \dot{w}_{0}}{\partial y} \frac{\partial \delta \,\dot{w}_{0}}{\partial y} \right) \\ &+ K_{2} \left(\left(k_{1} \, A^{\prime} \right)^{2} \left(\frac{\partial \dot{\theta}}{\partial x} \frac{\partial \delta \,\dot{\theta}}{\partial x} + \frac{\partial \dot{\theta}}{\partial x} \frac{\partial \delta \,\dot{w}_{0}}{\partial x} \right) + \left(k_{2} \, B^{\prime} \right)^{2} \left(\frac{\partial \dot{w}_{0}}{\partial y} \frac{\partial \delta \,\dot{\theta}}{\partial y} + \frac{\partial \dot{\theta}}{\partial y} \frac{\partial \delta \,\dot{w}_{0}}{\partial y} \right) \\ &- J_{2} \left(\left(k_{1} \, A^{\prime} \right) \left(\frac{\partial \dot{w}_{0}}{\partial x} \frac{\partial \delta \,\dot{\theta}}{\partial x} + \frac{\partial \dot{\theta}}{\partial x} \frac{\partial \delta \,\dot{w}_{0}}{\partial x} \right) + \left(k_{2} \, B^{\prime} \right) \left(\frac{\partial \dot{w}_{0}}{\partial y} \frac{\partial \delta \,\dot{\theta}}{\partial y} + \frac{\partial \dot{\theta}}{\partial y} \frac{\partial \delta \,\dot{w}_{0}}{\partial y} \right) \right] dA \end{split}$$

Where dot-superscript convention indicates the differentiation with respect to the time variable t; $\rho(z)$ is the mass density and (I_i, J_i, K_i) are mass inertias of metal and ceramic materials respectively expressed by:

$$(I_0, I_1, I_2) = \int_{-h/2}^{h/2} (1, z, z^2) \rho(z) dz$$
(18a)

$$(J_1, J_1^{st}, J_2, K_2, K_2^{st}) = \int_{-h/2}^{h/2} (f, g, zf, f^2, g^2) \rho(z) dz$$
(18b)

By substituting equations (13), (15) and (17) into equation (12), the following equation of motion can be obtained:

$$\begin{split} \delta u_{0} : \frac{\partial N_{x}}{\partial x} + \frac{\partial N_{xy}}{\partial y} &= I_{0}\ddot{u}_{0} - I_{1}\frac{\partial \ddot{w}_{0}}{\partial x} + J_{1}k_{1}A'\frac{\partial \ddot{\theta}}{\partial x} \\ \delta v_{0} : \frac{\partial N_{xy}}{\partial x} + \frac{\partial N_{y}}{\partial y} &= I_{0}\ddot{v}_{0} - I_{1}\frac{\partial \ddot{w}_{0}}{\partial y} + J_{1}k_{2}B'\frac{\partial \ddot{\theta}}{\partial y} \\ \delta w_{0} : \frac{\partial^{2}M_{x}^{b}}{\partial x^{2}} + 2\frac{\partial^{2}M_{xy}^{b}}{\partial x\partial y} + \frac{\partial^{2}M_{y}^{b}}{\partial y^{2}} + N_{x}^{0}\frac{\partial^{2}w}{\partial x^{2}} \\ &+ 2N_{xy}^{0}\frac{\partial^{2}w}{\partial x\partial y} + N_{y}^{0}\frac{\partial^{2}w}{\partial y^{2}} = I_{0}\ddot{w}_{0} + I_{1}\left(\frac{\partial \ddot{u}_{0}}{\partial x} + \frac{\partial \ddot{v}_{0}}{\partial y}\right) \\ &+ J_{2}\left(k_{1}A'\frac{\partial^{2}\ddot{\theta}}{\partial x^{2}} + k_{2}B'\frac{\partial^{2}\ddot{\theta}}{\partial y^{2}}\right) - I_{2}\nabla^{2}\ddot{w}_{0} + J_{1}^{ts}\ddot{\theta} \\ \delta \theta : -k_{1}A'\frac{\partial^{2}M_{x}}{\partial x^{2}} - k_{2}B'\frac{\partial^{2}M_{y}}{\partial y^{2}} - (k_{1}A'+k_{2}B')\frac{\partial^{2}M_{xy}}{\partial x\partial y} \\ &+ k_{1}A'\frac{\partial Q_{xz}}{\partial x} + k_{2}B'\frac{\partial Q_{yz}}{\partial y} - N_{z} + \frac{\partial S_{xz}}{\partial x} + \frac{\partial S_{yz}}{\partial y} \\ &+ g(0)\left[N_{x}^{0}\frac{\partial^{2}w}{\partial x^{2}} + 2N_{xy}^{0}\frac{\partial^{2}w}{\partial x\partial y} + N_{y}^{0}\frac{\partial^{2}w}{\partial y^{2}}\right] = \\ -J_{1}\left(k_{1}A'\frac{\partial \ddot{u}_{0}}{\partial x} + k_{2}B'\frac{\partial \ddot{v}_{0}}{\partial y}\right) + J_{2}\left(k_{1}A'\frac{\partial^{2}\ddot{w}_{0}}{\partial x^{2}} + k_{2}B'\frac{\partial^{2}\ddot{w}_{0}}{\partial y^{2}}\right) \\ -K_{2}\left((k_{1}A')^{2}\frac{\partial^{2}\ddot{\theta}}{\partial x^{2}} + (k_{2}B')^{2}\frac{\partial^{2}\ddot{\theta}}{\partial y^{2}}\right) + J_{1}^{st}\ddot{w}_{0} + K_{2}^{st}\ddot{\theta} \end{split}$$

The effective material properties at the *j*-th layer of FG plates according to the power-law form are expressed by:

$$P^{(j)}(z) = P_m + (P_c - P_m) V_c^{(j)}(z)$$
(20)

Where P_m and P_c are the Young's modulus (*E*), Poisson's ratio (*V*), of metal and ceramic materials, respectively. For elastic and isotropic FG plates,

Substituting equation (7) into equation (10) and the subsequent results into equation (14), the stress resultants are obtained in terms of strains as following compact from:

$$\begin{cases}
N\\M^{b}\\M^{s}$$

 $N_{z} = L(\varepsilon_{x}^{0} + \varepsilon_{y}^{0}) + L^{a}(k_{x}^{b} + k_{y}^{b}) + R(k_{x}^{s} + k_{y}^{s}) + R^{a}\varepsilon_{z}^{0}$ (21b)

In which

$$N = \left\{ N_x, N_y, N_{xy} \right\}^{l},$$

$$M^{b} = \left\{ M_x^{b}, M_y^{b}, M_{xy}^{b} \right\}^{l},$$

$$M^{s} = \left\{ M_x^{s}, M_y^{s}, M_{xy}^{s} \right\}^{l}$$

$$\varepsilon = \left\{ \varepsilon_x^{0}, \varepsilon_y^{0}, \gamma_{xy}^{0} \right\}^{l},$$

$$k^{b} = \left\{ k_x^{b}, k_y^{b}, k_{xy}^{b} \right\}^{l},$$

$$k^{s} = \left\{ k_x^{s}, k_y^{s}, k_{xy}^{s} \right\}^{l}$$
(22a)
$$k^{s} = \left\{ e_x^{0}, e_y^{0}, e_y^{0} \right\}^{l},$$
(22b)

$$A = \begin{bmatrix} A_{11} & A_{12} & 0 \\ A_{12} & A_{22} & 0 \\ 0 & 0 & A_{66} \end{bmatrix}, B = \begin{bmatrix} B_{11} & B_{12} & 0 \\ B_{12} & B_{22} & 0 \\ 0 & 0 & B_{66} \end{bmatrix}, D = \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{12} & D_{22} & 0 \\ 0 & 0 & D_{66} \end{bmatrix}$$
(22c)

$$B^{s} = \begin{bmatrix} B_{11}^{s} & B_{12}^{s} & 0 \\ B_{12}^{s} & B_{22}^{s} & 0 \\ 0 & 0 & B_{66}^{s} \end{bmatrix}, D^{s} = \begin{bmatrix} D_{11}^{s} & D_{12}^{s} & 0 \\ D_{12}^{s} & D_{22}^{s} & 0 \\ 0 & 0 & D_{66}^{s} \end{bmatrix}, H^{s} = \begin{bmatrix} H_{11}^{s} & H_{12}^{s} & 0 \\ H_{12}^{s} & H_{22}^{s} & 0 \\ 0 & 0 & H_{66}^{s} \end{bmatrix}$$
(22d)

$$Q = \{Q_{xz}^{s}, Q_{yz}^{s}\}^{t}, S = \{S_{xz}^{s}, S_{yz}^{s}\}^{t},$$

$$\gamma^{0} = \{\gamma_{xz}^{0}, \gamma_{yz}^{0}\}^{t}, \gamma^{1} = \{\gamma_{xz}^{1}, \gamma_{yz}^{1}\}^{t}$$
(22e)

$$F^{s} = \begin{bmatrix} F_{55}^{s} & 0\\ 0 & F_{44}^{s} \end{bmatrix}, X^{s} = \begin{bmatrix} X_{55}^{s} & 0\\ 0 & X_{44}^{s} \end{bmatrix}, A^{s} = \begin{bmatrix} A_{55}^{s} & 0\\ 0 & A_{44}^{s} \end{bmatrix}$$
(22f)

$$\begin{cases} L \\ L^{a} \\ R \\ R^{a} \end{cases} = \int_{z} \lambda(z) \begin{cases} 1 \\ z \\ f(z) \\ g'(z) \frac{1-\nu}{\nu} \end{cases} g'(z) dz$$
(22g)

and stiffness components are given as:

$$\begin{cases} A_{11} & B_{11} & D_{11} & B_{11}^{s} & D_{11}^{s} & H_{11}^{s} \\ A_{12} & B_{12} & D_{12} & B_{12}^{s} & D_{12}^{s} & H_{12}^{s} \\ A_{66} & B_{66} & D_{66} & B_{66}^{s} & D_{66}^{s} & H_{66}^{s} \\ \end{cases} = \int_{z} \lambda(z)(1, z, z^{2}, f(z), zf(z), f^{2}(z)) \begin{cases} \frac{1-\nu}{\nu} \\ 1 \\ \frac{1-2\nu}{2\nu} \end{cases} dz$$

$$(23a)$$

$$(A_{22}, B_{22}, D_{22}, B_{22}^{s}, D_{22}^{s}, H_{22}^{s}) = (A_{11}, B_{11}, D_{11}, B_{11}^{s}, D_{11}^{s}, H_{11}^{s})$$
(23b)

$$(F_{44}^{s}, X_{44}^{s}, A_{44}^{s}) = \int_{-h/2}^{h/2} \frac{E(z)}{2(1+\nu)} ([f'(z)]^{2}, f'(z)g(z), g^{2}(z)) dz$$
(23c)

$$(F_{55}^s, X_{55}^s, A_{55}^s) = (F_{44}^s, X_{44}^s, A_{44}^s)$$
(23d)

By substituting equation (21) into equation (19), the equation of motion can be expressed in terms of displacements (u_0, v_0, w_0, θ) and the appropriate equations take the form:

$$A_{11}d_{11}u_{0} + A_{66} d_{22}u_{0} + (A_{12} + A_{66})d_{12}v_{0} -B_{11}d_{111}w_{0} - (B_{12} + 2B_{66})d_{122}w_{0} + (B_{66}^{s}(k_{1}A' + k_{2}B') + B_{12}^{s}k_{2}B')d_{122}\theta + B_{11}^{s}k_{1}A'd_{111}\theta + Ld_{1}\theta = I_{0}\ddot{u}_{0} - I_{1}d_{1}\ddot{w}_{0} + J_{1}k_{1}A'd_{1}\ddot{\theta}$$
(24a)

$$A_{22} d_{22} v_0 + A_{66} d_{11} v_0 + (A_{12} + A_{66}) d_{12} u_0$$

$$- B_{22} d_{222} w_0 - (B_{12} + 2B_{66}) d_{112} w_0$$

$$+ (B_{66}^s (k_1 A' + k_2 B') + B_{12}^s k_1 A') d_{112} \theta$$

$$+ B_{22}^s k_2 B' d_{222} \theta + L d_2 \theta = I_0 \ddot{v}_0 - I_1 d_2 \ddot{w}_0$$

$$+ J_1 k_2 B' d_2 \ddot{\theta}$$
(24b)

$$B_{11} d_{111} u_0 + (B_{12} + 2B_{66}) d_{122} u_0 + (B_{12} + 2B_{66}) d_{112} v_0 + B_{22} d_{222} v_0 - D_{11} d_{1111} w_0 - 2 (D_{12} + 2D_{66}) d_{1122} w_0 - D_{22} d_{2222} w_0 + D_{11}^s k_1 A' d_{1111} \theta + ((D_{12}^s + 2D_{66}^s) (k_1 A' + k_2 B')) d_{1122} \theta + D_{22}^s k_2 B' d_{2222} \theta + L^a (d_{11} \theta + d_{22} \theta) + N_x^0 d_{11} w + 2N_{xy}^0 d_{12} w + N_y^0 d_{22} w = I_0 \ddot{w}_0 + I_1 (d_1 \ddot{u}_0 + d_2 \ddot{v}_0) - I_2 (d_{11} \ddot{w}_0 + d_{22} \ddot{w}_0) + J_2 (k_1 A' d_{11} \ddot{\theta} + k_2 B' d_{22} \ddot{\theta}) + J_1^{ts} \ddot{\theta}$$
(24c)

$$-k_{1}A'B_{11}^{s}d_{111}u_{0} - \left(B_{12}^{s}k_{2}B' + B_{66}^{s}\left(k_{1}A' + k_{2}B'\right)\right)d_{122}u_{0} \\ - \left(B_{12}^{s}k_{1}A' + B_{66}^{s}\left(k_{1}A' + k_{2}B'\right)\right)d_{112}v_{0} - B_{22}^{s}k_{2}B'd_{222}v_{0} \\ + D_{11}^{s}k_{1}A'd_{1111}w_{0}\left(\left(D_{12}^{s} + 2D_{66}^{s}\right)\left(k_{1}A' + k_{2}B'\right)\right)d_{1122}w_{0} \\ + D_{22}^{s}k_{2}B'd_{2222}w_{0} - H_{11}^{s}\left(k_{1}A'\right)^{2}d_{1111}\theta - H_{22}^{s}\left(k_{2}B'\right)^{2}d_{2222}\theta \\ - \left(2H_{12}^{s}k_{1}k_{2}A'B' + \left(k_{1}A' + k_{2}B'\right)^{2}H_{66}^{s}\right)d_{1122}\theta \\ + \left(\left(k_{1}A'\right)^{2}F_{55}^{s} + 2k_{1}A'X_{55}^{s} + A_{55}^{s}\right)d_{11}\theta + \left(24d\right) \\ \left(\left(k_{2}B'\right)^{2}F_{44}^{s} + 2k_{2}B'X_{44}^{s} + A_{44}^{s}\right)d_{22}\theta \\ - 2R(k_{1}A'd_{11}\theta + k_{2}B'd_{11}\theta) - L(d_{1}u_{0} + d_{2}v_{0}) \\ + L^{a}(d_{11}w_{0} + d_{22}w_{0}) - R^{a}\theta + g(0)\left(N_{x}^{0}d_{11}w + 2N_{xy}^{0}d_{12}w + N_{y}^{0}d_{22}w) = -J_{1}\left(k_{1}A'd_{1}\ddot{u}_{0} + k_{2}B'd_{2}\ddot{v}_{0}\right) \\ + J_{2}(k_{1}A'd_{11}\ddot{w}_{0} + k_{2}B'd_{22}\ddot{w}_{0}) \\ - K_{2}\left(\left(k_{1}A'\right)^{2}d_{11}\ddot{\theta} + \left(k_{2}B'\right)^{2}d_{22}\ddot{\theta}\right) + J_{1}^{st}\ddot{w}_{0} + K_{2}^{st}\ddot{\theta}$$

where d_{ij} , d_{ijl} and d_{ijlm} are the following differential operators:

$$d_{ij} = \frac{\partial^2}{\partial x_i \partial x_j}, \quad d_{ijl} = \frac{\partial^3}{\partial x_i \partial x_j \partial x_l}, \quad d_{ijlm} = \frac{\partial^4}{\partial x_i \partial x_j \partial x_l \partial x_m},$$

$$d_i = \frac{\partial}{\partial x_i}, \quad (i, j, l, m = 1, 2).$$
(25)

2.6 Close form solutions

The Navier solution method is utilized to deduce the analytical solutions for which the displacement variables are written as product of arbitrary parameters and known trigonometric functions to respect the equation of motion and boundary condition.

$$\begin{cases}
 u_{0} \\
 v_{0} \\
 w_{0} \\
 \theta
 \end{bmatrix} = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \begin{cases}
 U_{mn} e^{i\omega t} \cos(\alpha x) \sin(\beta y) \\
 V_{mn} e^{i\omega t} \sin(\alpha x) \cos(\beta y) \\
 W_{mn} e^{i\omega t} \sin(\alpha x) \sin(\beta y) \\
 X_{mn} e^{i\omega t} \sin(\alpha x) \sin(\beta y)
 \end{bmatrix}$$
(26)

Where ω is the frequency of free vibration of the plate, $\sqrt{i} = -1$ the imaginary unit with

$$\alpha = m\pi / a \text{ and } \beta = n\pi / b \tag{27}$$

The transverse load q is also expanded in the double-Fourier sine series as:

$$q(x, y) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} q_{mn} \sin(\alpha x) \sin(\beta y)$$
(28)

Where $q_{mn} = q_0$ forsinusoidally distributed load. Assuming that the plate is subjected to in-plane compressive loads of form: $N_x^0 = -N_0$, $N_y^0 = -\gamma N_0$, $N_{xy}^0 = 0$, $\gamma = N_y^0 / N_x^0$ (where γ are non – dimensional load parameter).

Substituting equations (26) and (28) into equation (24), the following equation is obtained:

$$\begin{pmatrix} S_{11} & S_{12} & S_{13} & S_{15} \\ S_{12} & S_{22} & S_{23} & S_{24} \\ S_{13} & S_{23} & S_{33} + \eta & S_{34} + g(0)\eta \\ S_{14} & S_{24} & S_{34} + g(0)\eta & S_{44} + [g(0)]^2 \eta \end{bmatrix} \begin{pmatrix} U_{mn} \\ V_{mn} \\ 0 \\ m_{22} & m_{23} & m_{24} \\ m_{13} & m_{23} & m_{33} & m_{34} \\ m_{14} & m_{24} & m_{34} & m_{44} \end{bmatrix}$$

$$(29)$$

where

$$S_{11} = (\alpha^{2}A_{11} + \beta^{2}A_{66}), S_{12} = \alpha\beta(A_{12} + A_{66})$$

$$S_{13} = -\alpha^{3}B_{11} - \alpha\beta^{2}(B_{12} + 2B_{66})$$

$$S_{14} = \alpha((k_{2}B'B_{12}^{s} + (k_{1}A' + k_{2}B')B_{66}^{s})\beta^{2} + k_{1}A'B_{11}^{s}\alpha^{2} - L), S_{22} = (\alpha^{2}A_{66} + \beta^{2}A_{22})$$

$$S_{23} = -\alpha^{2}\beta(B_{12} + 2B_{66}) - \beta^{3}B_{22}$$

$$S_{24} = \beta((k_{1}A'B_{12}^{s} + (k_{1}A' + k_{2}B')B_{66}^{s})\alpha^{2} + k_{2}B'B_{22}^{s}\beta^{2} - L)$$

$$S_{33} = (\alpha^{4}D_{11} + \beta^{4}D_{22} + 2\alpha^{2}\beta^{2}(D_{12} + 2D_{66}))$$
(30)

$$S_{34} = -(\alpha^{4}k_{1}A'D_{11}^{s} + 2\alpha^{2}\beta^{2}k_{2}B'D_{12}^{s} + \beta^{4}k_{2}B'D_{22}^{s}$$

$$-2\alpha^{2}\beta^{2}(k_{1}A' + k_{2}B')D_{66}^{s}) + L^{a}(\alpha^{2} + \beta^{2})$$

$$S_{44} = (k_{1}A'\alpha^{4}H_{11}^{s} + \beta^{4}(k_{2}B')^{2}H_{22}^{s}$$

$$+(2k_{1}k_{2}A'B'H_{12}^{s} + (k_{1}A' + k_{2}B')^{2}H_{66}^{s})\alpha^{2}\beta^{2}$$

$$+\alpha^{2}((k_{1}A')^{2}F_{55}^{s} + 2k_{1}A'X_{55}^{s} + A_{55}^{s}) + \beta^{2}((k_{2}B')^{2}F_{44}^{s})$$

$$+2k_{2}B'X_{44}^{s} + A_{44}^{s}) - 2R(k_{1}A'\alpha^{2} + k_{2}B'\beta^{2}) + R^{a}$$

$$\eta = -N_{0}(\alpha^{2} + \gamma\beta^{2})$$

and

$$m_{11} = I_0, \quad m_{13} = -\alpha I_1, \quad m_{14} = J_1 k_1 A' \alpha ,$$

$$m_{22} = I_0, \quad m_{23} = -\beta I_1, \quad m_{24} = k_2 B' \beta J_1,$$

$$m_{33} = I_0 + I_2 (\alpha^2 + \beta^2), \qquad (30)$$

$$m_{34} = -J_2 (k_1 A' \alpha^2 + k_2 B' \beta^2) + J_1^s,$$

$$m_{44} = K_2 ((k_1 A')^2 \alpha^2 + (k_2 B')^2 \beta^2) + K_2^{st}$$

Table 1 Material properties of metal and ceramic

Material	Young's Material modulus (<i>GPa</i>)		Poisson's ratio
Aluminium (Al*)	70	2702	
Aluminium (Al)	70	2707	0.0
$Zirconia(ZrO_2)$	151	3000	0.3
Alumina (Al_2O_3)	380	3800	

Eq. (29) is a general form for bending, buckling and free vibration analysis of isotropic and FG sandwich plates under in-plane and transverse loads. In order to solve bending problem, the in plane compressive load N_0 and mass matrix M are set to zeros. The critical buckling loads (N_{cr}) can be obtained from the stability problem $|K_{ij} = 0|$ while the free vibration problem is achieved by omitting both in-plane and transverse loads.

3. Numerical results and discussion

In this section, various numerical examples using the present quasi 3D solution are presented for bending, buckling and free vibration of simply supported FG sandwich plate. The results of the proposed computational solution will be first compared with existing data available in literature to check their accuracy. For this, the FGM sandwich plates are combined from ceramic and metal AI/ZrO_2 and AI/Al_2O_3 (see table 1 for their material properties). For convenience, the following dimensionless forms are used:

$$\begin{split} \bar{u}(z) &= \frac{100h^{3}E_{c}}{a^{4}q_{0}}u\left(0,\frac{b}{2},z\right), \quad \overline{w} = \frac{10h^{3}E_{c}}{a^{4}q_{0}}w\left(\frac{a}{2},\frac{b}{2}\right), \\ \hat{w} &= \frac{10h}{a}\frac{E_{0}}{q_{0}}w\left(\frac{a}{2},\frac{b}{2}\right), \quad \overline{\sigma}_{xx}(z) = \frac{h}{a}\frac{\sigma}{q_{0}}\sigma_{xx}\left(\frac{a}{2},\frac{b}{2},z\right), \\ \hat{\sigma}_{xx}(z) &= \frac{10h^{2}}{a^{2}q_{0}}\sigma_{xx}\left(\frac{a}{2},\frac{b}{2},z\right), \quad \overline{\sigma}_{xy}(z) = \frac{h}{a}\frac{\sigma}{q_{0}}\sigma_{xy}(0,0,z), \\ \overline{\sigma}_{xz}(z) &= \frac{h}{a}\frac{\sigma}{q_{0}}\sigma_{xz}\left(0,\frac{b}{2},z\right), \quad \overline{N}_{cr} = \frac{N_{cr}a^{2}}{E_{mh}^{3}}, \\ \hat{N}_{cr} &= \frac{N_{cr}a^{2}}{100}\frac{10}{E_{0}}h^{3}, \quad \overline{\omega} = \frac{\omega ab}{\pi^{2}h}\sqrt{\frac{12(1-V_{c}^{2})\rho_{c}}{E_{c}}} \\ \hat{\omega} &= \frac{\omega a^{2}}{h}\sqrt{\frac{\rho_{0}}{E_{0}}}, \quad \rho_{0} = 1kg/m^{3}, \quad E_{0} = 1\ GPa, \end{split}$$

3.1 Bending analysis

In order to prove the validity of the present improved higher shear deformation theory, comparisons are made

р	Theory	$\overline{u}(-h/4)$	w	$\overline{\sigma_{xx}}(h/3)$	$\overline{\sigma_{xy}}(-h/3)$	$\overline{\sigma_{_{xz}}}(h/6)$
	present	0.6269	0.6110	1.5859	0.6013	0.3536
	HSDT (Nguyen et al. 2014)	0.6413	0.5890	1.4897	0.6111	0.2611
	Quasi-3D (Carrera et al. 2008)	0.6436	0.5875	1.5062	0.6081	0.2510
1	Quasi-3D (Wu and Chui 2011)	0.6436	0.5876	1.5061	0.6112	0.2511
	SSDT (Zenkour 2006)	0.6626	0.5889	1.4894	0.6110	0.2622
	HSDT (Mantari et al. 2012)	0.6398	0.5880	1.4888	0.6109	0.2566
	TSDT (Thai and Kim 2013)	0.6414	0.5890	1.4898	0.6111	0.2608
	present	0.8792	0.7874	1.4760	0.5362	0.3353
	HSDT (Nguyen et al. 2014)	0.8982	0.7573	1.3959	0.5442	0.2742
	Quasi-3D (Carrera et al. 2008)	0.9012	0.7570	1.4147	0.5421	0.2496
2	Quasi-3D (Wu and Chui 2011)	0.9013	0.7571	1.4133	0.5436	0.2495
	SSDT (Zenkour 2006)	0.9281	0.7573	1.3954	0.5441	0.2763
	HSDT (Mantari et al. 2012)	0.8957	0.7564	1.3940	0.5438	0.2741
	TSDT (Thai and Kim 2013)	0.8984	0.7573	1.3960	0.5442	0.2737
	present	1.0409	0.8402	1.2285	0.5657	0.2677
	HSDT (Nguyen et al. 2014)	1.0500	0.8816	1.1792	0.5669	0.2546
	Quasi-3D (Carrera et al. 2008)	1.0541	0.8823	1.1985	0.5666	0.2362
4	Quasi-3D (Wu and Chui 2011)	1.0541	0.8823	1.1841	0.5671	0.2362
	SSDT (Zenkour 2006)	1.0941	0.8819	1.1783	0.5667	0.2580
	HSDT (Mantari et al. 2012)	1.0457	0.8814	1.1755	0.5662	0.2623
	TSDT (Thai and Kim 2013)	1.0502	0.8815	1.1794	0.5669	0.2537
	present	1.0853	0.9411	0.9842	0.5945	0.2066
	HSDT (Nguyen et al. 2014)	1.0759	0.9746	0.9473	0.5857	0.2094
0	Quasi-3D (Carrera et al. 2008)	1.0830	0.9738	0.9687	0.5879	0.2262
8	Quasi-3D (Wu and Chui 2011)	1.0830	0.9739	0.9622	0.5883	0.2261
	SSDT (Zenkour 2006)	1.1340	0.9750	0.9466	0.5856	0.2121
	HSDT (Mantari et al. 2012)	1.0709	0.9737	0.9431	0.5850	0.2140
	TSDT (Thai and Kim 2013)	1.0763	0.9746	0.9477	0.5858	0.2088

Table 2 Comparison of the nondimensional stress and displacements of Al/Al_2O_3 square plates (a/h = 10, Type A)

between the results obtained from this theory and those of obtained by Nguyen *et al.* (2014), Carrera *et al.* (2008), Wu and Chui (2011), Zenkour (2006), Mantari *et al.* (2012) and Thai and Kim (2013), Zenkour (2006), Neves *et al.* (2013) and Bessaim *et al.* (2013).

As a first example, a square plate type A (Al/Al_2O_3) subjected to a sinusoidal load is studied. In table 2, displacement results, axials and transvers stress are compared with the HSDT of Nguyen *et al.* (2014), the quasi-three-dimensional (3D) solutions of Carrera *et al.* (2008) and Wu and Chui (2011), the sinusoidal shear deformation theory (SSDT) of Zenkour (2006), the HSDT of Mantari *et al.* (2012) and the third shear deformation theory (TSDT) of Thai and Kim (2013). It is clear that the results of the present quasi 3D solution are in very close agreement with the different shear deformation theories.

The second example is performed for the bending responses of a Al/Al_2O_3 square sandwich plate Type B. The results of the displacement and stresses are given for fifth values of the power index "p" as shown in Table 3. According to the results presented in this table it can be

seen that the results of the present improved model are in very great agreement with those of the different solutions.

In the third example, a sandwich plate type C is examined. Table 4 shows the effects of power law index "p" on the dimensionless displacements and stresses for different configuration of the sandwich plate. The present results are compared with the results of the HSDT of Nguyen et al. (2014), the TSDT and the SSDT of Zenkour et al. (2005a), and the quasi 3D solutions of Zenkour et al. (2013), Neves et al. (2013) and Bessaim et al. (2013) respectively. It is clear from this table that whatever the case of the sandwich (2008) and Wu and Chui (2011), the sinusoidal shear deformation theory (SSDT) of Zenkour (2006), the HSDT of Mantari et al. (2012) and the third shear deformation theory (TSDT) of Thai and Kim (2013). It is clear that the results of the present quasi 3D solution are in very close agreement with the different shear deformation theories.

The second example is performed for the bending responses of a Al/Al_2O_3 square sandwich plate Type B. The results of the displacement and stresses are given for fifth

р	Theory	$\overline{u}(-h/4)$	 W	$\overline{\sigma_{w}}(h/3)$	$\overline{\sigma_{n}}(-h/3)$	$\overline{\sigma}(h/6)$
	Present	0 3033	0 3653	1 4906	0.9518	0 3050
0	HSDT (Nguyen <i>et al.</i> 2014)	0.3247	0.3247	1.4761	1.0130	0.2161
	Quasi-3D (Neves <i>et al.</i> 2013)	-	0.3711	-	-	0.2227
	Present	0.5002	0.5031	1.5677	0.6343	0.3402
0.5	HSDT (Nguyen et al. 2014)	0.5542	0.5245	1.5750	0.6965	0.2509
	Quasi-3D (Neves et al. 2013)	-	0.5238	-	-	0.2581
	Present	0.6532	0.6022	1.5451	0.4896	0.3429
	HSDT (Nguyen et al. 2014)	0.7337	0.6345	1.5691	0.5447	0.2733
1	FSDT (Brischetto 2009)	-	0.6337	-	-	0.2458
1	Quasi-3D (Carrera et al. 2011)	-	0.6324	-	-	0.2594
	Quasi-3D (Neves et al. 2012)	-	0.6305	-	-	0.2788
	Quasi-3D (Neves et al. 2013)	-	0.6305	-	-	0.2789
	Present	0.9317	0.7811	1.1858	0.5013	0.2405
	HSDT (Nguyen et al. 2014)	1.0550	0.8331	1.2539	0.5614	0.2697
4	FSDT (Brischetto 2009)	-	0.8191	-	-	0.1877
4	Quasi-3D (Carrera et al. 2011)	-	0.8307	-	-	0.2398
	Quasi-3D (Neves et al. 2012)	-	0.8202	-	-	0.2778
	Quasi-3D (Neves et al. 2013)	-	0.8199	-	-	0.2747
	Present	0.9618	0.8287	0.8679	0.5193	0.1524
	HSDT (Nguyen et al. 2014)	1.0798	0.8807	0.9258	0.5758	0.1982
10	FSDT (Brischetto 2009)	-	0.8556	-	-	0.1234
	Quasi-3D (Carrera et al. 201)	-	0.8740	-	-	0.1944
	Quasi-3D (Neves et al. 2012)	-	0.8650	-	-	0.2059
	Quasi-3D (Neves et al. 2013)	-	0.8645	-	-	0.2034

Table 3 Comparison of the nondimensional stress and displacements of Al/Al_2O_3 square sandwich plates (a/h = 10, Type B)

values of the power index "p" as shown in Table 3. According to the results presented in this table it can be seen that the results of the present improved model are in very great agreement with those of the different solutions. plate studied and whatever the value of the material index "p" the results of this model are in very good agreement with those of the literature.

Another comparison of the results of this method is presented in the following. Tables 5 presents the axial stress $\hat{\sigma}_{xx}$ of Al/ ZrO₂ sandwich plates type C for p = 0, 1, 2, 5and 10 and different sandwich configuration. As it can see from these tables, there is a good agreement between all solutions.

After these comparisons, it can be concluded that the present quasi 3D solution with only four unknowns is not only accurate but also efficient in predicting the bending responses of FG sandwich plates.

In figure 2, we present respectively the variations of transverse displacement and in-plane and out of plane stresses through the plate thickness of the FG plate type A with a/h=10 and different values of p.

From these figures, we can see that:

- The transverse displacement increases as the power law index p increases.

- For the axial non dimensional stress, we can see that this stress is tensile at the top surface and compressive at the bottom surface. - The in-plane tangential stress is tensile at the bottom surface and compressive at the top surface of the FG plates. The distribution of the transverse shear stress through the thickness of the plate is not parabolic except the case of p=0 (isotropic plate).

Figures 3 and 4 plot respectively the variation of stresses through the thickness direction for different values of p of Al/Al_2O_3 and Al/ZrO_2 square sandwich plates subjected to sinusoidal load with a/h=10 and for type B and Type C. As it can be seen from the case of sandwich plate type C (figure 4), the maximum stresses are situated at layer's interfaces except the case of sandwich plate 1-2-1 where the maximum is located at the mid plane.

3.2 Free vibration analysis

Other examples to check the accuracy of the present quasi 3D computational solution in predicting the natural frequency of FG plate and FG sandwich plate are reported in Tables 6, 7 and 8.

Table 6 gives a comparison of the non dimensional frequencies ϖ of a square FG plate type A between the present solution and those of Nguyen *et al.* (2014) and the 3D solution of Uymaz and Aydogdu (2007). It is observed an excellent agreement.

Table 7 compares the nondimensional fundamental frequency $(\hat{\omega})$ by the present theory for a square sandwich



Fig. 2 Nondimensional displacements and stresses through the thickness direction for different values of p of Al/Al_2O_3 square plates subjected to sinusoidal load (a/h = 10, Type A)



Fig. 3 Nondimensional stresses through the thickness direction for different values of p of Al/Al_2O_3 square sandw ich plates subjected to sinusoidal load with (a/h = 10, Type B)

plates (Al/Al_2O_3 , type B) with those given by Nguyen *et al.* (2014) and the two HSDT given by Natarajan and Manickam (2012). The results are given for three value of the ratio a / h and three configuration of the sandwich plate. From the results shown in the table, there is a slight

difference. This can be explained by the fact that HSDTs neglect the effect of stretching thing that is taken into consideration by the present 3D solution.

The nondimensional fundamental frequency $\hat{\omega}$ for a rectangular FG sandwich plates Al/Al_2O_3 type C predicted

Table 4 Nondimensional center deflections (\hat{W}) of Al/Zr_2O_2 square sandwich plates (a/h = 10, Type C)

p	Theory	1-0-1	2-1-2	2-1-1	1-1-1	2-2-1	1-2-1
	Present	0.19344	0.19344	0.19344	0.19344	0.□9344	0.19344
	HSDT (Nguyen et al. 2014)	0.19597	0.19597	0.19597	0.19597	0.19597	0.19597
	TSDT (Zenkour 2005a)	0.19606	0.19606	_	0.19606	0.19606	0.19606
0	SSDT (Zenkour 2005a)	0.19605	0.19605	_	0.19605	0.19605	0.19605
	Quasi-3D (Zenkour 2013)	0.19487	0.19487	_	0.19487	0.19487	0.19487
	Quasi-3D (Neves et al. 2013)	_	0.19490	0.19490	0.19490	0.19490	0.19490
	Quasi-3D (Bessaim et al. 2013)	_	0.19486	0.19486	0.19486	0.19486	0.19486
	Present	0.31463	0.29750	0.28854	0.28377	0.27331	0.26394
	HSDT (Nguyen et al. 2014)	0.32348	0.30622	0.29666	0.29191	0.28077	0.27086
	TSDT (Zenkour 2005a)	0.32358	0.30632	_	0.29199	0.28085	0.27094
1	SSDT (Zenkour 2005a)	0.32349	0.30624	_	0.29194	0.28082	0.27093
	Quasi-3D (Zenkour 2013)	0.32001	0.30275	_	0.28867	0.27760	0.26815
	Quasi-3D (Neves et al. 2013)	_	0.30700	0.29750	0.29290	0.28200	0.27220
	Quasi-3D (Bessaim et al. 2013)	_	0.30430	0.29448	0.29007	0.27874	0.26915
	Present	0.36232	0.34055	0.32699	0.32174	0.30610	0.29330
	HSDT (Nguyen et al. 2014)	0.37322	0.35221	0.33769	0.33279	0.31608	0.30255
	TSDT (Zenkour 2005a)	0.37335	0.35231	_	0.33289	0.31617	0.30263
2	SSDT (Zenkour 2005a)	0.37319	0.35218	_	0.33280	0.31611	0.30260
	Quasi-3D (Zenkour 2013)	0.36891	0.34737	_	0.32816	0.31152	0.29874
	Quasi-3D (Neves et al. 2013)	_	0.35190	0.33760	0.33290	0.31640	0.30320
	Quasi-3D (Bessaim et al. 2013)	_	0.35001	0.33495	0.33068	0.31356	0.30060
	Present	0.39895	0.37827	0.36072	0.35769	0.33724	0.32296
	HSDT (Nguyen et al. 2014)	0.40911	0.39170	0.37295	0.37134	0.34950	0.33472
	TSDT (Zenkour 2005a)	0.40927	0.39183	-	0.37145	0.34960	0.33480
5	SSDT (Zenkour 2005a)	0.40905	0.39160	_	0.37128	0.34950	0.33474
	Quasi-3D (Zenkour 2013)	0.40532	0.38612	_	0.36546	0.34361	0.32966
	Quasi-3D (Neves et al. 2013)	-	0.39050	0.37220	0.37050	0.34900	0.33470
	Quasi-3D (Bessaim et al. 2013)	_	0.38934	0.36981	0.36902	0.34649	0.33255
	Present	0.40915	0.39069	0.37218	0.37110	0.34911	0.33538
	HSDT (Nguyen et al. 2014)	0.41754	0.40393	0.3843	0.38540	0.36202	0.34815
	TSDT (Zenkour 2005a)	0.41772	0.40407	_	0.38551	0.36215	0.34824
10	SSDT (Zenkour 2005a)	SSDT (Zenkour 2005a) 0.41750 0.40376		_	0.38490	0.34916	0.34119
10	Quasi-3D (Zenkour 2013)	0.41448	0.39856	_	0.37924	0.35577	0.34259
	Quasi-3D (Neves et al. 2013)	-	0.40260	0.38350	0.38430	0.36120	0.34800
	Quasi-3D (Bessaim et al. 2013)		0.40153	0.38111	0.38303	0.35885	0.34591



Fig. 4 Nondimensional stresses through the thickness direction for different values of p of Al/ZrO_2 square sandwich plates subjected to sinusoidal load with (a/h=10, Type C)

р	Theory	1-0-1	2-1-2	2-1-1	1-1-1	2-2-1	1-2-1
	present	2.05448	2.05448	2.05448	2.05448	2.05448	2.05448
	HSDT (Nguyen et al. 2014)	1.99482	1.99482	1.99482	1.99482	1.99482	1.99482
	TSDT (Zenkour 2005a)	2.04985	2.04985	_	2.04985	2.04985	2.04985
0	SSDT (Zenkour 2005a)	2.05452	2.05452	-	2.05452	2.05452	2.05452
	Quasi-3D (Zenkour 2013)	2.00773	2.00773	-	2.00773	2.00773	2.00773
	Quasi-3D (Neves et al. 2013)	-	2.00660	2.00640	2.00660	2.00650	2.00640
	Quasi-3D (Bessaim et al. 2013)	_	1.99524	1.99524	1.99524	1.99524	1.99524
	present	1.60902	1.52551	1.41084	1.45341	1.33931	1.34511
	HSDT (Nguyen et al. 2014)	1.54441	1.46297	1.35703	1.39406	1.28852	1.29174
	TSDT (Zenkour 2005a)	1.57923	1.49587	-	1.42617	1.32062	1.32309
1	SSDT (Zenkour 2005a)	1.58204	1.49859	-	1.42892	1.32342	1.32590
	Quasi-3D (Zenkour 2013)	1.57004	1.48833	-	1.41781	1.30907	1.31204
	Quasi-3D (Neves et al. 2013)	_	1.48130	1.37680	1.41370	1.30920	1.31330
	Quasi-3D (Bessaim et al. 2013)	_	1.46131	1.35053	1.39243	1.28274	1.29030
	present	1.85971	1.76303	1.59352	1.66669	1.49720	1.51163
	HSDT (Nguyen et al. 2014)	1.78383	1.68682	1.52988	1.59393	1.43693	1.44707
	TSDT (Zenkour 2005a)	1.82167	1.72144	-	1.62748	1.47095	1.47988
2	SSDT (Zenkour 2005a)	1.82450	1.72412	-	1.63025	1.47387	1.48283
	Quasi-3D (Zenkour 2013)	1.81509	1.72030	-	1.62591	1.46372	1.47421
	Quasi-3D (Neves et al. 2013)	_	1.69940	1.54560	1.60880	1.45430	1.46590
	Quasi-3D (Bessaim et al. 2013)	_	1.68472	1.52101	1.59170	1.42887	1.44497
	present	2.02673	1.96180	1.74878	1.86542	1.64438	1.68005
	HSDT (Nguyen et al. 2014)	1.95031	1.87709	1.67895	1.78159	1.57620	1.60459
	TSDT (Zenkour 2005a)	1.99272	1.91302	-	1.81580	1.61181	1.63814
5	SSDT (Zenkour 2005a)	1.99567	1.91547	-	1.81838	1.61477	1.64106
	Quasi-3D (Zenkour 2013)	1.97912	1.91504	-	1.82018	1.60953	1.63906
	Quasi-3D (Neves et al. 2013)	_	1.88380	1.69090	1.79060	1.58930	1.61950
	Quasi-3D (Bessaim et al. 2013)	_	1.87516	1.66856	1.77919	1.56627	1.60203
	present	2.05479	2.01933	1.79924	1.93603	1.69875	1.74982
	HSDT (Nguyen et al. 2014)	1.98382	1.93431	1.72890	1.84933	1.62840	1.67019
	TSDT (Zenkour 2005a)	2.03036	1.97126	-	1.88376	1.66660	1.70417
10	SSDT (Zenkour 2005a)	2.03360	1.97313	-	1.88147	1.61979	1.64851
	Quasi-3D (Zenkour 2013)	2.00692	1.97075	-	1.89162	2.18558	1.67350
	Quasi-3D (Neves et al. 2013)	_	1.93970	1.74050	1.85590	1.63950	1.68320
	Quasi-3D (Bessaim et al. 2013)	_	1.93266	1.71835	1.84705	1.61792	1.66754

Table 5 Nondimensional axial stress $\hat{\sigma}_{rr}(h/2)$ of Al/Zr_2O_2 square sandwich plates(a/h = 10, Type C)



Fig. 5 Effect of the power-law index p on the nondimensional fundamental frequency ($\hat{\omega}$) of Al/Al_2O_3 square sandwich plates (a/h = 10, Type C)

by Nguyen *et al.* (2014) using the HSDT, Zenkour (2005b) using the TSDT and SSDT, Bessaim *et al.* (2013) using a quasi 3D solution, Li *et al.* (2008) using a 3D modeland the present theory are compared in Table 8. An excellent agreement is observed.

Figure 5 plots the variation of the non-dimensional fundamental natural frequency of simply supported FG sandwich plate type C as a function the power-law index.

As can be seen, the increase in the power index p reduces the frequency. On the other hand, the lowest frequencies and the largest correspond respectively to sandwich plates type 1-0-1 and 1-2-1. On the other hand, the lowest frequencies and the largest correspond respectively to sandwich plates type 1-0-1 and 1-2-1 this is due to the fact that these plates contain the lowest and the largest volume fraction of the ceramic. The latter plays a very important role in making the plate flexible or rigid.

- /1-	These	power - law index								
a/n	Incory	0	0.1	0.2	0.5	1	2	5	10	
	present	1.2671	1.2383	1.2134	1.1570	1.0998	1.0425	0.9840	0.9580	
2	HSDT (Nguyen et al. 2014)	1.2454	1.2162	1.1913	1.1356	1.0784	1.0234	0.9685	0.9435	
	3D (Uymaz and Aydogdu 2007)	1.2589	1.2296	1.2049	1.1484	1.0913	1.0344	0.9777	0.9507	
	present	1.7830	1.7369	1.6982	1.6149	1.5392	1.4762	1.4175	1.3786	
5	HSDT (Nguyen et al. 2014)	1.7683	1.7208	1.6818	1.5974	1.5212	1.4601	1.4058	1.3690	
	3D (Uymaz and Aydogdu 2007)	1.7748	1.7262	1.6881	1.6031	1.4764	1.4628	1.4106	1.3711	
	present	1.9388	1.8861	1.8424	1.7503	1.6708	1.6108	1.5575	1.5139	
10	HSDT (Nguyen et al. 2014)	1.9317	1.8773	1.8332	1.7393	1.6583	1.5986	1.5492	1.5083	
	3D (Uymaz and Aydogdu 2007)	1.9339	1.8788	1.8357	1.7406	1.6583	1.5968	1.5491	1.5066	
	present	1.9853	1.9303	1.8849	1.7902	1.7100	1.6520	1.6012	1.5561	
20	HSDT (Nguyen et al. 2014)	1.9821	1.9254	1.8797	1.7827	1.7003	1.6415	1.5943	1.5521	
	3D (Uymaz and Aydogdu 2007)	1.9570	1.9261	1.8788	1.7827	1.6999	1.6401	1.5937	1.5491	
	present	1.9989	1.9431	1.8972	1.8017	1.7215	1.6642	1.6143	1.5687	
50	HSDT (Nguyen et al. 2014)	1.9971	1.9397	1.8935	1.7956	1.7129	1.6543	1.6078	1.5652	
	3D (Uymaz and Aydogdu 2007)	1.9974	1.9390	1.8920	1.7944	1.7117	1.6522	1.6062	1.5620	
	present	2.0009	1.9450	1.8990	1.8033	1.7231	1.6659	1.6162	1.5705	
100	HSDT (Nguyen et al. 2014)	1.9993	1.9418	1.8955	1.7975	1.7147	1.6562	1.6098	1.5671	
	3D (Uymaz and Aydogdu 2007)	1.9974	1.9418	1.8920	1.7972	1.7117	1.6552	1.6062	1.5652	

Table 6 Comparison of the nondimensional fundamental frequency ϖ of Al^*/ZrO_2 square plates (Type A)

Table 7 Comparison of the nondimensional fundamental frequency ($\hat{\omega}$) of Al/Al_2O_3 square sandwich plates (Type B)

	Theorem		1-1	1-1			1-2-1			2-2-1	
a/n	Тпеогу	0	0.5	1	5	0.5	1	5	0.5	1	5
	present	1.2985	1.2412	1.2121	1.1635	1.2818	1.2326	1.1545	1.2022	1.1791	1.1383
5	HSDT (Nguyen et al. 2014)	1.1147	1.1414	1.1561	1.1996	1.1574	1.1827	1.2569	1.1916	1.2268	1.3160
5	HSDT (Natarajan and Manickam 2012)	1.1021	1.1449	1.1639	1.2113	1.1597	1.1884	1.2644	1.1965	1.2350	1.3249
	HSDT(Natarajan and Manickam 2012)	1.0893	1.1511	1.1701	1.2162	1.1663	1.1952	1.2712	1.2031	1.2421	1.3312
	present	1.3744	1.3217	1.2964	1.2625	1.3665	1.3209	1.2650	1.2895	1.2757	1.2638
10	HSDT (Nguyen et al. 2014)	1.2172	1.2359	1.2478	1.2883	1.2567	1.2763	1.3466	1.2827	1.3187	1.4130
10	HSDT (Natarajan and Manickam 2012)	1.2138	1.2373	1.2506	1.2921	1.2578	1.2785	1.3492	1.2846	1.3216	1.4161
	HSDT (Natarajan and Manickam 2012)	1.2087	1.2392	1.2524	1.2935	1.2598	1.2806	1.3513	1.2865	1.3238	1.4180
	present	1.3934	1.3482	1.3269	1.3030	1.3937	1.3529	1.3114	1.3221	1.3144	1.3174
100	HSDT (Nguyen et al. 2014)	1.2617	1.2752	1.2853	1.3238	1.2984	1.3147	1.3824	1.3198	1.3558	1.4518
100	HSDT (Natarajan and Manickam 2012)	1.2617	1.2751	1.2854	1.3239	1.2981	1.3148	1.3825	1.3198	1.3559	1.4519
	HSDT (Natarajan and Manickam 2012)	1.2616	1.2751	1.2854	1.3239	1.2981	1.3148	1.3825	1.3198	1.3559	1.4519

3.3 Buckling analysis

First, for the verification purpose, the results computed by the present quasi 3D theory are compared with those obtained by different higher shear deformation theory as reported in tables 9 and 10.

In table 9, we present a comparison of the critical buckling load of FG plate "type A" obtained from the present solution and those of Nguyen *et al.* (2014) and Thai and Choi (2012). The results are presented for different power law index "p" and different ratio of a/h and a/b. Also, two cases are studied plate subjected to uniaxial

compression $\gamma = 0$ and biaxial compression $\gamma = 1$. It can be seen from this table that there is an excellent agreement between the results.

Another comparison is presented in Table 10. Critical buckling loads are obtained for sandwich plate "type B" and for biaxial compressive loads. The comparison with the different theories and for the different scheme of sandwich plates reveals that the results of the present theory are in very great concordance.

Table 11 gives the critical buckling loads of Al/Al_2O_3 square sandwich plate type B under biaxial compressions. Three scheme of sandwich plate are presented 1-1-1, 1-2-1 and 2-2-1.

р	Theory	1-0-1	2-1-2	2-1-1	1-1-1	2-2-1	1-2-1
	present	1.83452	1.83452	1.83452	1.83452	1.83452	1.83452
	HSDT (Nguyen et al. 2014)	1.82489	1.82489	1.82489	1.82489	1.82489	1.82489
0	TSDT (Zenkour 2005b)	1.82445	1.82445	1.82445	1.82445	1.82445	1.82445
0	SSDT (Zenkour 2005b)	1.82452	1.82452	1.82452	1.82452	1.82452	1.82452
	Quasi-3D (Bessaim et al. 2013)	1.82682	1.82682	-	1.82682	1.82682	1.82682
	3D (Li et al. 2008)	1.82682	1.82682	_	1.82682	1.82682	1.82682
	present	1.46638	1.50645	1.52819	1.54086	1.56800	1.59425
	HSDT (Nguyen et al. 2014)	1.44348	1.48355	1.50597	1.51885	1.54680	1.57437
0.5	TSDT (Zenkour 2005b)	1.44424	1.48408	1.51253	1.51922	1.55199	1.57451
0.5	SSDT (Zenkour 2005b)	1.44436	1.48418	1.51258	1.51927	1.55202	1.57450
	Quasi-3D (Bessaim et al. 2013)	1.44621	1.48611	_	1.52130	1.55016	1.57670
	3D (Li et al. 2008)	1.44614	1.48608	_	1.52131	1.54926	1.57668
	present	1.27115	1.32941	1.36216	1.38183	1.42324	1.46496
1	HSDT (Nguyen et al. 2014)	1.24332	1.30024	1.33352	1.35345	1.39579	1.43948
	TSDT (Zenkour 2005b)	1.24320	1.30011	1.34888	1.35333	1.40789	1.43934
1	SSDT (Zenkour 2005b)	1.24335	1.30023	1.34894	1.35339	1.40792	1.43931
	Quasi-3D (Bessaim et al. 2013)	1.24495	1.30195	_	1.35527	1.39987	1.44143
	3D (Li et al. 2008)	1.24470	1.30181	_	1.35523	1.39763	1.44137
	present	0.97167	1.01844	1.06756	1.08337	1.14760	1.21055
	HSDT (Nguyen et al. 2014)	0.94611	0.98193	1.03067	1.04473	1.10905	1.17403
5	TSDT (Zenkour 2005b)	0.94598	0.98184	1.07432	1.04466	1.14731	1.17397
5	SSDT (Zenkour 2005b)	0.94630	0.98207	1.07445	1.04481	1.14741	1.17399
	Quasi-3D (Bessaim et al. 2013)	0.94716	0.98311	_	1.04613	1.11723	1.17579
	3D (Li et al. 2008)	0.94476	0.98103	_	1.04532	1.10983	1.17567
	present	0.94693	0.97831	1.02814	1.03476	1.10038	1.16140
	HSDT (Nguyen et al. 2014)	0.92854	0.94305	0.99219	0.99558	1.06114	1.12320
10	TSDT (Zenkour 2005b)	0.92839	0.94297	1.03862	0.99551	1.10533	1.12314
10	SSDT (Zenkour 2005b)	0.92875	0.94332	1.04558	0.99519	1.04154	1.13460
	Quasi-3D (Bessaim et al. 2013)	0.92952	0.94410	_	0.99684	1.07015	1.12486
	3D (Li <i>et al.</i> 2008)	0.92727	0.94078	_	0.99684	1.06104	1.12466

Table 8 Nondimensional fundamental frequency ($\hat{\omega}$) of Al/Al_2O_3 square sandwich plates(a/h = 10, Type C)

Table 9 Comparison of the critical buckling load (\overline{N}_{cr}) of Al/Al_2O_3 plates (Type A)

	/ I -	/b a/h				power - la	w index p		
γ	a/b	a/n	1 neorie	0	0.5	1	2	5	10
			present	6.7482	4.5091	3.5203	2.7412	2.2031	1.9419
		5	HSDT (Nguyen et al. 2014)	6.7204	4.4221	3.4164	2.6450	2.1479	1.9210
			TSDT (Thai and Choi 2012)	6.7203	4.4235	3.4164	2.6451	2.1484	1.9213
			present	7.4335	4.9084	3.8315	3.0165	2.5043	2.2342
	0.5	10	HSDT (Nguyen et al. 2014)	7.4053	4.8190	3.7111	2.8896	2.4163	2.1897
			TSDT (Thai and Choi 2012)	7.4053	4.8206	3.7111	2.8897	2.4165	2.1896
			present	7.6104	5.0050	3.9106	3.0930	2.5928	2.3214
		20	HSDT (Nguyen et al. 2014)	7.5993	4.9298	3.7930	2.9581	2.4944	2.2692
0			TSDT (Thai and Choi 2012)	7.5993	4.9315	3.7930	2.9582	2.4944	2.2690
0			present	16.0518	10.8051	8.4448	6.5376	5.1456	4.5007
		5	HSDT (Nguyen et al. 2014)	16.0216	10.6215	8.2247	6.3430	5.0513	4.4800
			TSDT (Thai and Choi 2012)	16.0211	10.6254	8.2245	6.3432	5.0531	4.4807
			present	18.6639	12.3579	9.6441	7.5712	6.2404	5.5526
	1	10	HSDT (Nguyen et al. 2014)	18.5786	12.1181	9.3391	7.2630	6.0346	5.4530
			TSDT (Thai and Choi 2012)	18.5785	12.1229	9.3391	7.2631	6.0353	5.4528
			present	19.3936	12.7656	9.9715	7.8783	6.5912	5.8969
		20	HSDT (Nguyen et al. 2014)	19.3528	12.5616	9.6675	7.5371	6.3446	5.7674
-			TSDT (Thai and Choi 2012)	19.3528	12.5668	9.6675	7.5371	6.3448	5.7668

_									
			present	5.3986	3.6072	2.8162	2.1930	1.7624	1.5535
		5	HSDT (Nguyen et al. 2014)	5.3763	3.5377	2.7331	2.1160	1.7183	1.5368
			TSDT (Thai and Choi 2012)	5.3762	3.5388	2.7331	2.1161	1.7187	1.5370
			present	5.9468	3.9267	3.0652	2.4132	2.0034	1.7874
	0.5	10	HSDT (Nguyen et al. 2014)	5.9243	3.8552	2.9689	2.3117	1.9330	1.7517
			TSDT (Thai and Choi 2012)	5.9243	3.8565	2.9689	2.3117	1.9332	1.7517
			present	6.0883	4.0040	3.1285	2.4744	2.0743	1.8571
		20	HSDT (Nguyen et al. 2014)	6.0794	3.9438	3.0344	2.3665	1.9955	1.8153
1 _			TSDT (Thai and Choi 2012)	6.0794	3.9452	3.0344	2.3665	1.9955	1.8152
1			present	8.0259	5.4025	4.2224	3.2688	2.5728	2.2503
		5	HSDT (Nguyen et al. 2014)	8.0108	5.3108	4.1124	3.1715	2.5256	2.2400
			TSDT (Thai and Choi 2012)	8.0105	5.3127	4.1122	3.1716	2.5265	2.2403
			present	9.3319	6.1789	4.8220	3.7856	3.1202	2.7763
	1	10	HSDT (Nguyen et al. 2014)	9.2893	6.0590	4.6696	3.6315	3.0173	2.7265
			TSDT (Thai and Choi 2012)	9.2893	6.0615	4.6696	3.6315	3.0177	2.7264
			present	9.6968	6.3828	4.9857	3.9391	3.2956	2.9484
		20	HSDT (Nguyen et al. 2014)	9.6764	6.2808	4.8337	3.7686	3.1723	2.8837
			TSDT (Thai and Choi 2012)	9.6764	6.2834	4.8337	3.7686	3.1724	2.8834



Fig. 6 Effect of the power-law index p on the critical buckling load (\hat{N}_{cr}) of Al/Al_2O_3 square sandwich plates (a/h = 10, Type C).

From this table, it is found that, for the same sandwich plate scheme, the increase in the power index and aspect ratio a / h increased the critical value of the buckling load.

Figure 6 plots the variation of the critical buckling load versus the power law index and for different scheme of sandwich plate. It has seen a rapid decrease of the critical buckling loads until a value of p = 5, Once exceeding this value, the critical buckling loads tends to keep a more or less constant shape and this whatever the sandwich plate used.

4. Conclusions

Bending, buckling and vibration analysis of functionally graded sandwich plate is carried out in the present study by

Table 11Nondimensional critical buckling loads (\overline{N}_{cr}) of Al/Al_2O_3 square sandwich plates subjected to biaxial compressive loads ($\gamma = 1$, Type B)

	sahama			р		
a/n	scheme	0	0.5	1	5	10
	1-1-1	3.0420	2.6861	2.5170	2.2326	2.1832
5	1-2-1	3.4979	2.8840	2.5985	2.1504	2.077
	2-2-1	2.7574	2.4480	2.3000	2.0337	1.9788
	1-1-1	3.3232	2.9675	2.8044	2.5630	2.5324
10	1-2-1	3.8448	3.1993	2.9100	2.5229	2.4835
	2-2-1	3.0163	2.7437	2.6255	2.4579	2.4294
	1-1-1	3.3801	3.0522	2.9026	2.6945	2.6740
100	1-2-1	3.9095	3.2918	3.0175	2.6763	2.6530
	2-2-1	3.0814	2.8494	2.7525	2.6367	2.6208

an improved quasi 3D theory. Three different sandwich plates were studied. The results generated in the present work for three cases analyzed namely buckling, bending and free vibration were compared with the results of various theories from the literature. This comparison revealed that the present theory is very precise. A detailed parametric study was presented to highlight the various factors influencing the behavior of sandwich plates.

Based on the Numerical results section of this research, the following considerations are valuable:

The transverse displacement increases as the power law index p increases.

The axial non dimensional stress is tensile at the top surface and compressive at the bottom surface.

The in-plane tangential stress is tensile at the bottom surface and compressive at the top surface of the FG plates.

The distribution of the transverse shear stress through

р	Theory	1-0-1	2-1-2	2-1-1	1-1-1	2-2-1	1-2-1
	present	6.53239	6.53239	6.53239	6.53239	6.53239	6.53239
	HSDT (Nguyen et al. 2014)	6.50566	6.50566	6.50566	6.50566	6.50566	6.50566
0	TSDT (Zenkour 2005b)	6.50248	6.50248	6.50248	6.50248	6.50248	6.50248
0	SSDT (Zenkour 2005b)	6.50303	6.50303	6.50303	6.50303	6.50303	6.50303
	HSDT (Neves et al. 2012)	6.50266	6.50266	6.50266	6.50266	6.50266	6.50266
	Quasi-3D (Neves et al. 2012)	6.47652	6.47652	6.47652	6.47652	6.47652	6.47652
	present	3.77542	4.06891	4.20890	4.31559	4.49937	4.69824
	HSDT (Nguyen et al. 2014)	3.67832	3.96760	4.10999	4.21622	4.40304	4.60760
0.5	TSDT (Zenkour 2005b)	3.68219	3.97042	4.11235	4.21823	4.40499	4.60841
0.5	SSDT (Zenkour 2005b)	3.68284	3.97097	4.11269	4.21856	4.40519	4.60835
	HSDT (Neves et al. 2012)	3.59354	3.87157	4.00853	4.11071	4.29073	4.48676
	Quasi-3D (Neves et al. 2012)	3.58096	3.85809	3.99480	4.09641	4.27592	4.47110
	present	2.68780	3.03833	3.21593	3.35339	3.59508	3.86818
1	HSDT (Nguyen et al. 2014)	2.58410	2.92060	3.09759	3.23299	3.47544	3.75403
	TSDT (Zenkour 2005b)	2.58357	2.92003	3.09697	3.23237	3.47472	3.75328
	SSDT (Zenkour 2005b)	2.58423	2.92060	3.09731	3.23270	3.47490	3.75314
	HSDT (Neves et al. 2012)	2.53913	2.86503	3.03679	3.16779	3.40280	3.67204
	Quasi-3D (Neves et al. 2012)	2.53062	2.85563	3.02733	3.15750	3.39207	3.66013
	present	1.39547	1.63023	1.81826	1.91731	2.19255	2.50656
	HSDT (Nguyen et al. 2014)	1.32948	1.52155	1.70203	1.79002	2.05633	2.36760
5	TSDT (Zenkour 2005b)	1.32910	1.52129	1.70176	1.78978	2.05605	2.36734
3	SSDT (Zenkour 2005b)	1.33003	1.52203	1.70224	1.79032	2.05644	2.36744
	HSDT (Neves et al. 2012)	1.32331	1.50935	1.68594	1.77072	2.03078	2.33036
	Quasi-3D (Neves et al. 2012)	1.31829	1.50409	1.68128	1.76507	2.02534	2.32354
	present	1.28686	1.47198	1.65311	1.71918	1.98553	2.27897
	HSDT (Nguyen et al. 2014)	1.24406	1.37341	1.54622	1.59758	1.85403	2.14020
10	TSDT (Zenkour 2005b)	1.24363	1.37316	1.54595	1.59736	1.85376	2.13995
10	SSDT (Zenkour 2005b)	1.24475	1.37422	1.56721	1.59728	1.57287	2.19087
	HSDT (Neves et al. 2012)	1.24090	1.36547	1.53468	1.58421	1.83573	2.10897
	Quasi-3D (Neves et al. 2012)	1.23599	1.36044	1.53036	1.57893	1.83083	2.10275

Table 10 Nondimensional critical buckling loads (\bar{N}_{cr}) of Al/Al_2O_3 square sandwich plates subjected to biaxial compressive loads ($\gamma = 1, a/h = 10$, Type C)

the thickness of the plate is not parabolic except the case of isotropic plate.

The increase in the power index p reduces the frequency.

The increase in the power index and aspect ratio a / h increased the critical value of the buckling load.

The kinematics used in this research can be extended to study other cases of structures such as nano-plates, beams and nano-beams with or without elastic foundation (Bouadi *et al.* (2018), Bellifa *et al.* (2017)).

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