

Chattering-free sliding mode control with a fuzzy model for structural applications

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Abstract. This paper proposes a chattering-free sliding mode control (CFSMC) method for seismically excited structures. The method is based on a fuzzy logic (FL) model applied to smooth the control force and eliminate chattering, where the switching part of the control law is replaced by an FL output. The CFSMC is robust and keeps the advantages of the conventional sliding mode control (SMC), whilst removing the chattering and avoiding the time-consuming process of generating fuzzy rule basis. The proposed method is tested on an 8-story shear frame equipped with an active tendon system. Results indicate that the new method not only can effectively enhance the seismic performance of the structural system compared to the SMC, but also ensure system stability and high accuracy with less computational cost. The CFSMC also requires less amount of energy from the active tendon system to produce the desired structural dynamic response.

Keywords: sliding mode control; chattering; fuzzy logic; structural control

1. Introduction

Vibration mitigation and structural control have drawn the attention of many researchers over the last decades as an effective method for dissipating vibration energy. The necessity of reducing building vibrations has motivated researchers into developing various control schemes such as active, semi-active, and passive methods, with the first two being proposed more recently. These systems are characterised by adaptive mechanisms in which control forces are generated by employing external power (Yeganeh Fallah and Taghikhany 2014, Askari *et al.* 2016, Marian and Giaralis 2017, Younespour and Ghaffarzadeh 2016).

The active structural control process requires measuring the structural response, determining the force from the measurements, and applying a designed load to obtain the controlled or desired structural response. Adaption to structural changes and environment relies on the algorithm used as a processor in the active control mechanisms, which can strongly impact the performance of the control system. Fisco and Adeli (2011a) carried out a review study on active and semi-active control of structures performed from 1997. In a companion paper, the authors also reviewed variously improved and new control strategies developed for civil structures (Fisco and Adeli 2011b). The key element to achieve a proper control requires selecting an effective control algorithm for obtaining the control force that needs

to be applied to the structural system.

The sliding mode control (SMC) method, as a nonlinear algorithm, was introduced to active control of civil structures by Yang *et al.* (1995) and Adhikari and Yamaguchi (1997), and is based on high-frequency switching (Solea and Nunes 2007). The variable structure of the SMC makes it capable of switching between different control laws. Since the SMC is insensitive against changes and external excitation, it has become a competitive choice among other control methods. Several applications can be highlighted (Yu *et al.* 2016, Yeganeh Fallah and Taghikhany 2015, Wu and Yang 2004, Lee and Chen 2011, Baradaranian *et al.* 2012, Yang *et al.* 2015).

Even though the SMC has many advantages, the chattering phenomenon associated with the switches in the control force can negatively impact the actuators during the dynamic mitigation and is often pointed out as the major drawback for practical implementation. Various alternatives were proposed to improve the control performance of conventional SMC, for example, based on the boundary layer method (Adhikari and Yamaguchi 1997), higher order SMC (Ozer *et al.* 2017), gain adaption (Wang and Adeli 2012), and neural networks (Yakut and Alli 2011, Li *et al.* 2000).

Having into account the current state of knowledge, a different approach is proposed in this paper to achieve a chattering-free SMC. The method is based on a fuzzy logic model to estimate and replace the discontinuity of the SMC law, i.e., the source of the chattering, by a smoother approximation. Fuzzy logic control (FLC) as a smart control technique has been used for active control in structures (Guclu and Yazici 2008, Yu *et al.* 2016, Ghaffarzadeh and Aghabalaei 2017, Gu *et al.* 2019). Human

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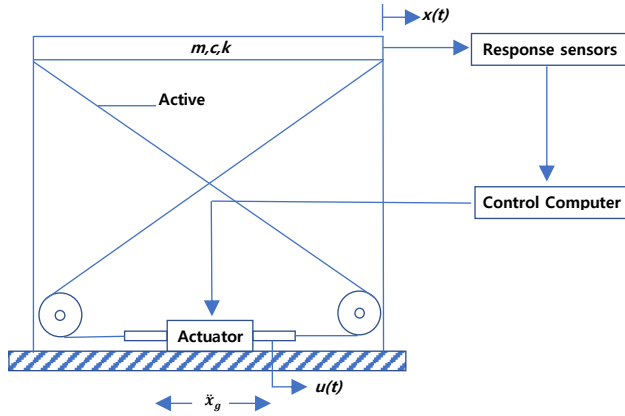


Fig. 1 Active tendon system

knowledge base and less mathematical effort made it a convenient control technique. The method uses an approximation reasoning and applies linguistic statements to the relationship between system variables. In this paper, the CFSMC is applied to a control system based on active tendons. Such system uses pre-stressed cables or diagonal bracings located between floors of a structure or at the ends of cables in cable-stayed bridges that can be activated axially by servo-controlled hydraulic actuators to quickly adjust the stress state. The method proposed in the following sections is validated using a numerical example under earthquake excitations where uncontrolled and controlled responses are analyzed.

2. Control system model

The motion equation for a controlled structural system with n -degrees of freedom can be written as

$$M\ddot{x}(t) + C\dot{x}(t) + Kx(t) = Bu(t) + MR\ddot{x}_g(t), \quad (1)$$

where M , C , and K are $(n \times n)$ mass, damping and stiffness matrices, respectively, $\ddot{x}(t)$, $\dot{x}(t)$ and $x(t)$ are the $(n \times 1)$ acceleration, velocity and displacement vectors, respectively, B is a $(n \times r)$ location matrix of r controllers, and R is a $(n \times 1)$ vector denoting the influence of the earthquake excitation \ddot{x}_g with terms equal to -1.

The state space form of Eq. (1) can be expressed as follows

$$\dot{z}(t) = Az(t) + B_1u(t) + B_2\ddot{x}_g(t), \quad (2)$$

where

$$A = \begin{bmatrix} 0 & I \\ -M^{-1}K & -M^{-1}C \end{bmatrix}, \quad B_1 = \begin{bmatrix} 0 \\ M^{-1}B \end{bmatrix} \\ B_2 = \begin{bmatrix} 0 \\ M^{-1}R \end{bmatrix}, \quad z(t) = \begin{Bmatrix} x(t) \\ \dot{x}(t) \end{Bmatrix}$$

and A is a $(2n \times 2n)$ plant matrix of the system, B_1 is a $(2n \times r)$ control location matrix, B_2 is an excitation influence vector of size $(2n \times 1)$, $z(t)$ is a $(2n \times 1)$ state vector related to the floor displacements and velocities, and $u(t)$ refers to the control law making Eq. (2) solvable.

In this paper, an active tendon configuration is proposed to apply the control force on the structure. Since such system is based on diagonal elements, which already exist in many structures after stiffening and strengthening, it becomes an attractive practical solution. Fig. 1 shows the control mechanism.

As it is shown in Fig. 1, tendons are installed between two stories. The hydraulic actuator is comprised of an actuator, a servo valve, and a fluid pumping system attached to the lower floor. One end of the tendon is connected to the upper floor and the other end to the piston. The relative movement due to inter-story drift caused by structural vibration alters the tension state of the tendons, which generates a dynamic force to mitigate the response.

3. Sliding mode control

The basic strategy of the SMC is based on enforcing the system to move towards a steady state regime by defining a suitable control force. The steady state is known as the sliding switching surface. In the SMC, the structure of the controller is purposely changed by a switching feedback law to drive the trajectories of the controlled system onto the specified sliding surface, known as reaching phase, and enforce them to remain on the surface sliding towards the equilibrium point. Such condition is known as sliding mode (Slotine and Li 1991).

The sliding surface is herein set as a linear function of system states

$$\sigma(z) = Sz, \quad (3)$$

where S is the sliding surface coefficient matrix $(r \times 2n)$. A suitable choice of S together with constraint conditions in Eq. (4) leads the trajectories to reach the sliding surface and slide over it

$$\dot{\sigma}(z) = 0 \quad \text{and} \quad \sigma(z) = 0. \quad (4)$$

The linear quadratic regulator (LQR) method is used to determine S and design the sliding surface (Yang *et al.* 1995), where the integral of the quadratic function of the state vector is minimised to derive the sliding surface coefficient matrix.

$$J = \int_0^\infty Z(t)^T Q Z(t) dt. \quad (5)$$

In Eq. (5), Q denotes a $(2n \times 2n)$ positive definite diagonal weighting matrix. Using transformation matrix, D , the state equation and the sliding surface can be written in terms of a transformed state vector Y ,

$$Y = DZ, \quad Z = D^{-1}Y \\ D = \begin{bmatrix} I_{2n-r} & -B_1B_2^{-1} \\ 0 & I_r \end{bmatrix}, \quad B_1 = \begin{bmatrix} B_{11} \\ B_{12} \end{bmatrix}, \quad (6)$$

where I_{2n-r} and I_r are $(2n-r) \times (2n-r)$ and $(r \times r)$ identity matrices, respectively. $B_{11} = (2n-r) \times r$ and $B_{12} = r \times r$ sub-matrices are obtained from the partition of B_1 in Eq. (2). Hence,

$$\dot{Y} = \bar{A}Y + \bar{B}U, \quad \sigma = \bar{S}Y = 0, \quad (7)$$

in which

$$\bar{A} = DAD^{-1}, \quad \bar{S} = SD^{-1}, \quad \bar{B} = \begin{bmatrix} 0 \\ B_{12} \end{bmatrix}. \quad (8)$$

The performance index J defined earlier then becomes

$$J = \int_0^\infty [Y_1', Y_2']^T T \begin{bmatrix} Y_1 \\ Y_2 \end{bmatrix} dt, \quad (9)$$

where Y_1 and Y_2 are $(2n-r)$ and r vectors, respectively, and

$$T = [(D^{-1})' Q D^{-1}], \quad T = \begin{bmatrix} T_{11} & T_{12} \\ T_{21} & T_{22} \end{bmatrix}. \quad (10)$$

T_{11} and T_{22} are $(2n-r) \times (2n-r)$ and $(r \times r)$ matrices, respectively, and by minimizing Eq. (9), S can be obtained from Eq. (8) as $S = \bar{S}D$.

To calculate the control law, Eq. (2) is replaced into $\dot{\sigma}(z) = 0$ as follows

$$\dot{\sigma}(z) = S\dot{z} = S(Az + B_1u + B_2\ddot{x}_g) = 0, \quad (11)$$

$$u_{eq} = -(SB_1)^{-1}(SAz + SB_2\ddot{x}_g). \quad (12)$$

Since the earthquake excitation is not known beforehand, the control law in Eq. (12) cannot be directly used, and the disturbance $(B_2\ddot{x}_g)$ has to be neglected. To account for the earthquake excitation and compensate the uncertainties in the disturbances, a discontinuous control law can be obtained via the known system parameters and under appropriate conditions (Slotine and Li 1991). To guarantee the existence and reachability of the sliding mode, the control law is implemented with the following inequality

$$\sigma^T(z)\dot{\sigma}(z) < -\eta|\sigma|, \quad (13)$$

where η is a positive constant value. Substituting Eq. (2) into Eq. (13), we get

$$\sigma^T(z)S(Az + B_1u + B_2\ddot{x}_g) < -\eta|\sigma|. \quad (14)$$

Considering $u(t)$ as

$$u(t) = -(SB_1)^{-1}SAz - (\eta + \gamma) \operatorname{sgn}(\sigma^T SB_1)^T \\ = u_{eq} - (\eta + \gamma) \operatorname{sgn}(\sigma^T SB_1)^T, \quad (15)$$

where γ is the bound on excitation vector, and 'sgn' stands for the sign function, Eq. (14) can be written as

$$\begin{aligned} \sigma^T \dot{\sigma} &= \sigma^T (SAz - SB_1[(SB_1)^{-1}SAz \\ &\quad - (\eta + \gamma) \operatorname{sgn}(\sigma^T SB_1)^T] + SB_2\ddot{x}_g) \\ &= \sigma^T (-SB_1(\eta + \gamma) \operatorname{sgn}(\sigma^T SB_1)^T + SB_2\ddot{x}_g) \\ &= -\eta|\sigma^T SB_1| - \gamma|\sigma^T SB_1| + \sigma^T SB_2\ddot{x}_g \\ &= -\eta|\sigma^T SB_1| - \gamma|\sigma^T SB_1|(1 - \frac{\sigma^T SB_2\ddot{x}_g}{\gamma|\sigma^T SB_1|}) \\ &< -\eta|\sigma^T SB_1|. \end{aligned} \quad (16)$$

Therefore, considering $u(t)$ given by Eq. (15) and satisfying Eq. (13) guarantees the existence and reachability of a sliding mode. For $K = \eta + \gamma$, the control law can finally be rewritten as

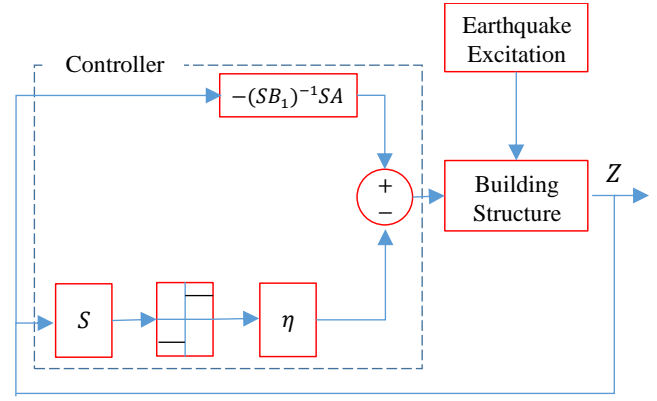


Fig. 2 Block diagram of SMC

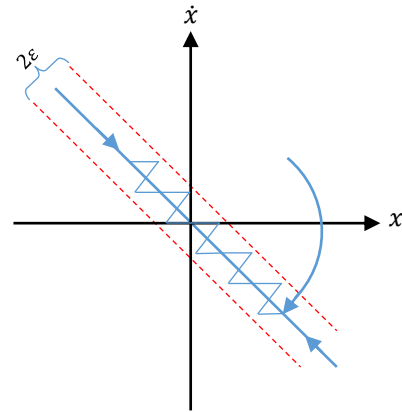


Fig. 3 Sliding surface with chattering and boundary layer

$$u = u_{eq} - K \operatorname{sgn}(\sigma^T SB_1)^T. \quad (17)$$

Fig. 2 illustrates the block diagram of the SMC. However, the direct implementation of Eq. (15) causes the chattering phenomenon due to the discontinuous part of the equation ($\operatorname{sgn}(\sigma^T SB_1)$) which changes the sign of the control force within short time periods generating high-frequency switches.

Chattering can be reduced by introducing a continuous approximation of the discontinuous sliding mode controller within a thin boundary layer neighbouring the sliding surface to smooth switches. One possible mathematical form of such solution is based on the replacement of the sign function with a term derived from the fuzzy inference mechanism as discussed in the next section.

4. Chattering-free sliding mode control

Among various techniques available to reduce chattering, the boundary layer method can approximate the sign function in Eq. (15) by using a saturation function. Accordingly, a thin boundary layer is defined in the neighbourhood of the sliding surface where chattering occurs. Fig. 3 indicates the schematic view of the chattering phenomenon and the boundary layer neighbouring the sliding surface.

The saturation function is written as follows

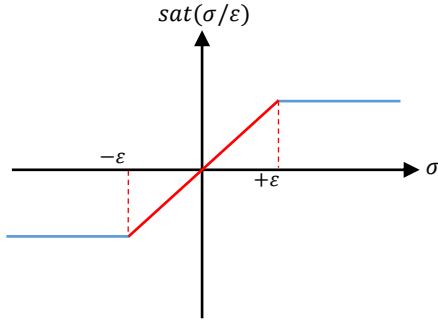


Fig. 4 Linear approximation of the sign function

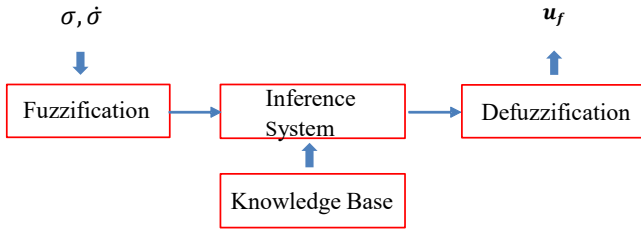


Fig. 5 Structure of a fuzzy logic system

$$\text{sat}(\sigma/\varepsilon) = \begin{cases} \sigma/\varepsilon & \text{if } |\sigma/\varepsilon| \leq 1 \\ \text{sgn}(\sigma/\varepsilon) & \text{otherwise} \end{cases} \quad (18)$$

where ε is a positive constant and 2ε is the thickness of the boundary layer. This method smooths the control signal by estimating and replacing the sign function with the saturation function illustrated in Fig. 4.

Since the method creates the loss of accuracy in the control signal, a different approach is proposed in this paper based on a fuzzy inference system to estimate the discontinuous part of Eq. (15) and smooth the control signal. Fig. 5 shows a typical fuzzy logic system.

The step of fuzzification converts crisp inputs into fuzzy sets and allocates a degree of membership to every fuzzy input value between 0 and 1. Each fuzzy set can make use of different types of membership functions such as triangular, trapezoidal, and Gaussian. The knowledge base unit consists of IF-THEN rules, each comprising antecedent and consequent propositions. A fuzzy rule based on SMC can be written as

$$\text{IF } \underbrace{\sigma \text{ is } A_1 \text{ and } \dot{\sigma} \text{ is } A_2}_{(1)} \text{ THEN } \underbrace{u_f \text{ is } B}_{(2)},$$

where σ is a switching variable, $\dot{\sigma}$ stands for its derivative, u_f is the fuzzy output, and A_i and B are the fuzzy input and output sets, and (1) and (2) represent the statements. The inference system performs fuzzy operations to map the fuzzy inputs to outputs. The defuzzification step maps the fuzzy output in a crisp value for the control law.

To apply the SMC strategy, the fuzzy rules can be obtained based on the trajectories in the phase plane. Specifically, the control force is calculated to bring back the trajectory to a proper state leading to the desired control action. The fuzzy rules can be explained with respect to the various positions and directions of trajectories and without any trial and error as in conventional rule bases.

Table 1 Knowledge base of fuzzy SMC

| $\dot{\sigma}/\sigma$ | PL | PM | PS | Z | NS | NM | NL |
|-----------------------|----|----|----|----|----|----|----|
| PL | NL | NL | NM | NS | NS | Z | Z |
| PM | NL | NM | NM | NS | Z | Z | PS |
| PS | NM | NM | NS | NS | Z | PS | PS |
| Z | NM | NS | NS | Z | PS | PS | PM |
| NS | NS | NS | Z | PS | PS | PM | PM |
| NM | NS | Z | Z | PS | PM | PM | PL |
| NL | Z | Z | PS | PS | PM | PL | PL |

Table 1 shows the fuzzy rule basis, where P, N, L, M, S, Z means Positive, Negative, Large, Medium, Small, Zero, respectively. Symbols represent linguistic values of σ , $\dot{\sigma}$, and u_f . For example, for a position in the trajectory far from the sliding surface and in the positive region ($\sigma = PL$) while moving from it ($\dot{\sigma} = PL$), a considerable control force is needed to restore the trajectory towards the sliding surface ($u_f = NL$).

The proper choice of membership functions can lead to the most suitable approximation of sign functions. In this study, Gaussian and singleton type membership functions are used for input and output fuzzy members, respectively. Moreover, by using singleton fuzzification, product inference, and center-average defuzzification, the fuzzy output can also be obtained as (Hsiao *et al.* 2005)

$$u_f = \frac{\sum_{j=1}^m w_j c_j}{\sum_{j=1}^m w_j} = v^T \psi, \quad (19)$$

where

$$w_j = \prod_{i=1}^n \mu_{F_i^j}(x_i), \quad v = [c_1, \dots, c_m]^T, \quad (20)$$

$$\psi = \frac{[w_1 \quad \dots \quad w_m]^T}{\sum_{j=1}^m w_j}. \quad (21)$$

In Eqs. (19)-(21), m and n are the total number of fuzzy rules and input variables, respectively, c_j represents the center of the membership function in the consequent part of the j -th rule, $\mu_{F_i^j}(x_i)$ denotes the membership value of the linguistic variable x_i to the fuzzy set F_i in the j -th rule, w_j represents the firing strength of the j -th rule, and ψ is the firing strength vector.

Based on the fuzzy control rules for $\sigma \neq 0$, the fuzzy control output (u_f) enforces the system trajectories to return to the sliding surface, which is in fact identical to the SMC inequality law, i.e., $\sigma(z)\dot{\sigma}(z) < 0$. Using the fuzzy model and replacing the sign function with u_f then fulfills the reachability and existence of a sliding mode.

The new control method can handle different control actions based on the different states of σ and $\dot{\sigma}$, which implies a nonlinear mapping from σ and $\dot{\sigma}$ to u_f . Hence, the chattering-free SMC (CFSMC) law can be written as shown in Eq. (22), and the nonlinear approximation of the sign function within the boundary layer in the

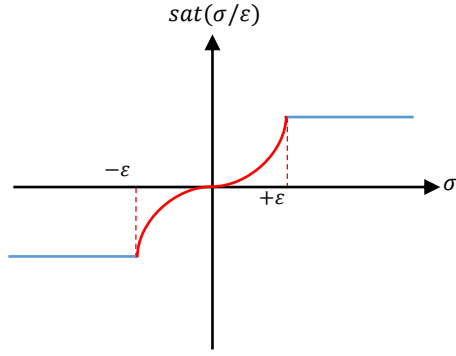


Fig. 6 Fuzzy approximation of the sign function

Table 2 Mass, stiffness, and damping values of the building

| Story | Mass (ton) | Stiffness (kN/m) | Damping (kN.s/m) |
|-------|------------|--------------------|------------------|
| 1 | 345.6 | 3.4×10^5 | 490 |
| 2 | 345.6 | 3.2×10^5 | 467 |
| 3 | 345.6 | 2.85×10^5 | 410 |
| 4 | 345.6 | 2.69×10^5 | 386 |
| 5 | 345.6 | 2.43×10^5 | 349 |
| 6 | 345.6 | 2.07×10^5 | 298 |
| 7 | 345.6 | 1.69×10^5 | 243 |
| 8 | 345.6 | 1.37×10^5 | 196 |

Table 3 Properties of selected ground motions

| Earthquake | El Centro | Northridge |
|--------------------------|------------------------------------|---------------------------------|
| Station | Imperial Valley, Station No.117 | Alhambra, CA, Fermont School |
| Magnitude | 6.9 | 6.6 |
| Depth (km) | 8.8 | 18 |
| PGA (cm/s ²) | 341.69 | 99.08 |
| PGV (cm/s) | 33.45 | 10.89 |
| PGD (cm) | 10.86 | 2.47 |

neighbourhood of the sliding surface takes the shape illustrated in Fig. 6.

$$u_{CFSMC} = u_{eq} - (\eta + \gamma) \operatorname{sgn}(\sigma^T S B_1)^T = u_{eq} - K(u_f). \quad (22)$$

5. Numerical study

A numerical example based on an eight-story shear building equipped with active tendons in the first and eighth stories is used in this section to illustrate the application of the CFSSMC and its effectiveness in avoiding chattering whilst reducing the dynamic responses. The method is also compared against conventional SMC.

The dynamic properties of the structure selected for analysis are indicated in Table 2 (Yang *et al.* 1995). The earthquake records of El Centro (1940) and Northridge (1994) are used as dynamic excitation, as detailed in Table

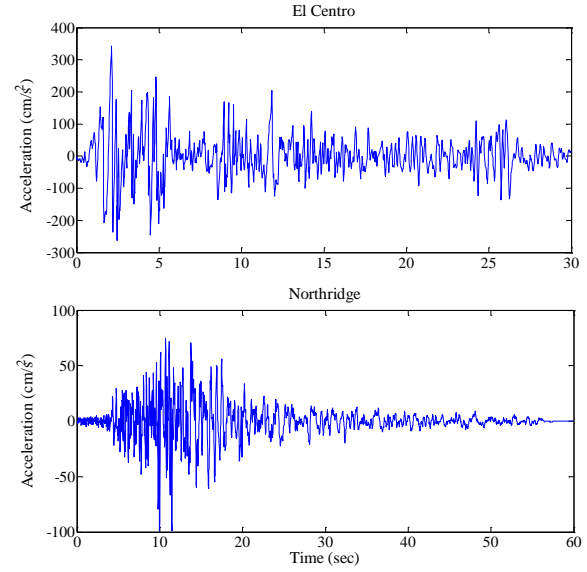


Fig. 7 Time histories of the selected ground motions

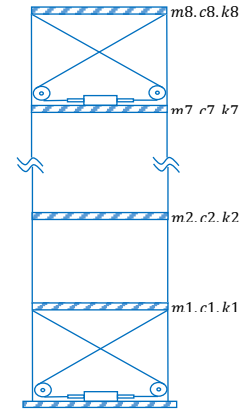


Fig. 8 Structural model of the active tendon system

3. The acceleration records of the two earthquakes are also depicted in Fig. 7.

Fig. 8 illustrates the configuration of the building, where due to the significant values of the shear force and displacement in the first and eighth stories, respectively those floors are equipped with the active tendon systems. The standard response time of the actuator is considered between 6-16 milliseconds, in which case the active tendon system can be assumed to produce the desired control force instantly.

With the SMC, the sliding surface is determined with the LQR method using a diagonal weighting matrix Q where $Q_{ii} = 10^6$ for $i=1, 2, \dots, 8$, and $Q_{ii} = 1$ for $i=9, 10, \dots, 16$. For the configuration of the active tendon system shown in Fig. 8, i.e., with a 45° inclination angle, the sliding surface equation for the controller in the first floor becomes

$$\begin{aligned} \sigma_1 = & 709.206(z_1) - 278.298(z_2) - 498.556(z_3) \\ & + 31.819(z_4) - 20.578(z_5) - 17.553(z_6) \\ & - 9.142(z_7) - 4.444(z_8) + 90.214(z_9) \\ & + 89.211(z_{10}) + 52.61(z_{11}) + 37.889(z_{12}) \\ & + 28.719(z_{13}) + 19.434(z_{14}) + 12.165(z_{15}) \\ & + 6.082(z_{16}). \end{aligned}$$

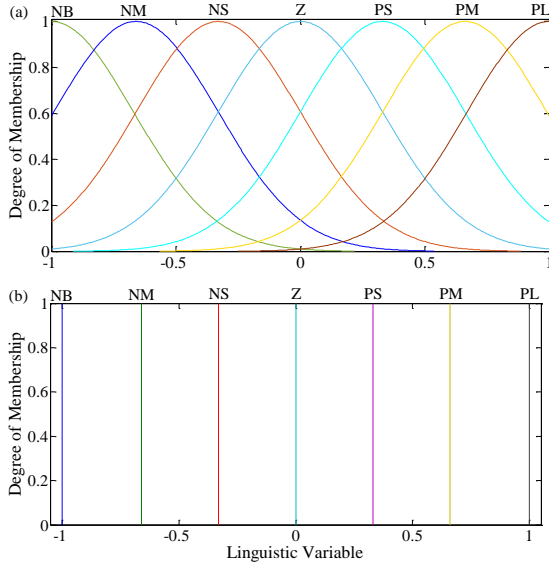


Fig. 9 Membership functions: (a) input variables ($\sigma, \dot{\sigma}$), (b) output variable (u_f)

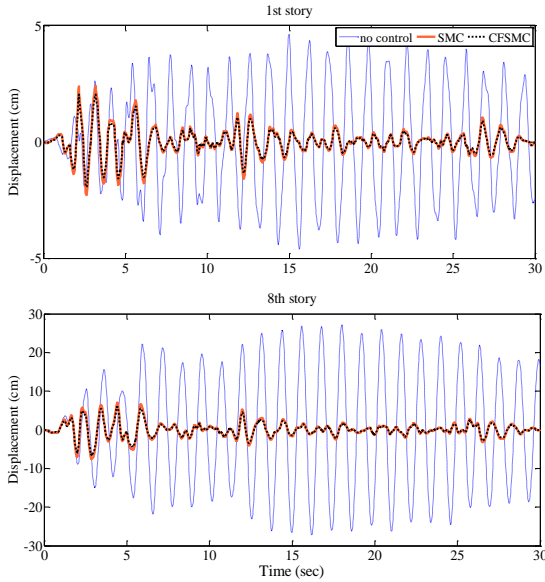


Fig. 10 Displacement responses during El Centro earthquake

For the controller installed on the eighth floor, the corresponding sliding surface equation is given by

$$\begin{aligned} \sigma_8 = & 4.444(z_1) + 4.286(z_2) + 22.001(z_3) \\ & + 46.535(z_4) + 29.971(z_5) - 4.206(z_6) \\ & - 160.228(z_7) + 709.206(z_8) + 6.745(z_9) \\ & + 6.745(z_{10}) + 7.132(z_{11}) + 8.497(z_{12}) \\ & + 10.567(z_{13}) + 10.629(z_{14}) + 29.409(z_{15}) \\ & + 15.498(z_{16}). \end{aligned}$$

The FLC model is also designed using two input variables (σ and $\dot{\sigma}$) and one output variable (u_f) each with seven membership functions. The functions chosen for both input and output variables are Gaussian-shaped and singleton functions, respectively, as shown in Fig 9. Therefore, the fuzzy model is constructed with 49 rules.

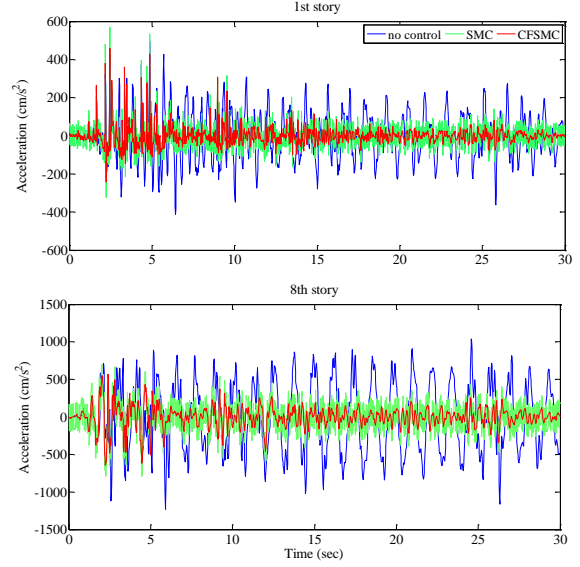


Fig. 11 Acceleration responses during El Centro earthquake

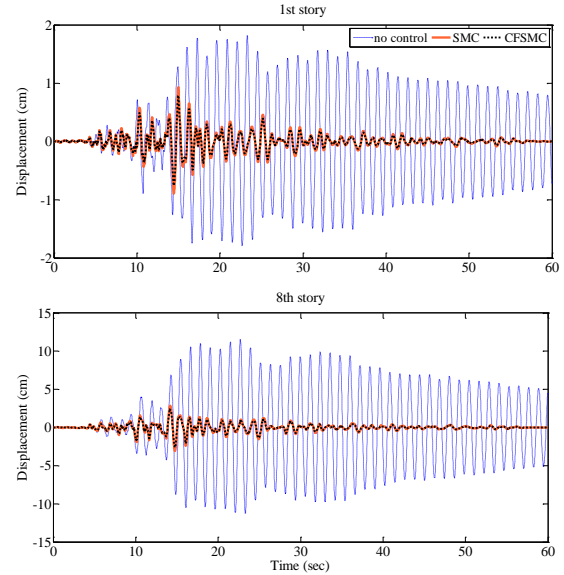


Fig. 12 Displacement responses during Northridge earthquake

The values of v are obtained according to the fuzzy control rules set in Table 1. Finally, it should be mentioned that K is considered as 200 for both SMC and CFSMC laws.

Fig. 10 shows the uncontrolled and controlled displacements with the SMC and CFSMC for the first and the eighth stories during the El Centro excitation. From Fig. 10 it can be concluded that both methods can decrease the displacements considerably.

The acceleration responses are depicted in Fig. 11, where the high-frequency switches obtained with the SMC method are evident. During the Northridge earthquake, both control methods demonstrated a good performance (Fig. 12). However, the high-frequency switches prevent the SMC to reduce the acceleration responses satisfactorily (Fig. 13).

To better illustrate the chattering phenomenon in the

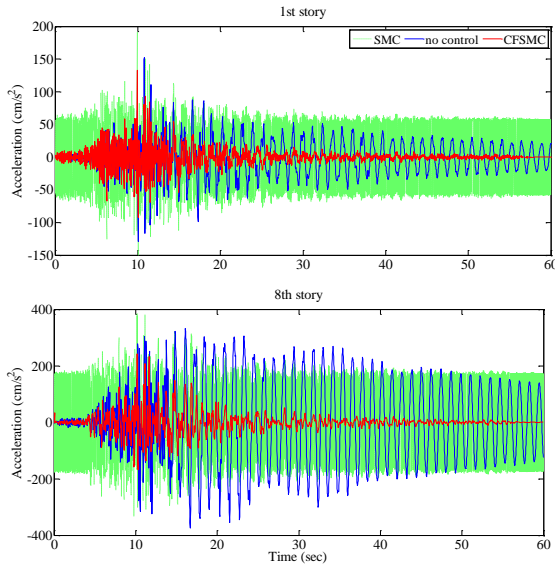


Fig. 13 Acceleration responses during Northridge earthquake

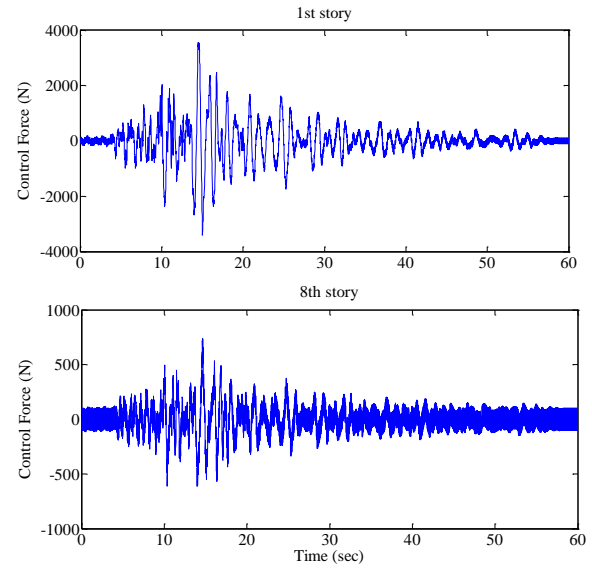


Fig. 15 Control force with SMC during Northridge earthquake

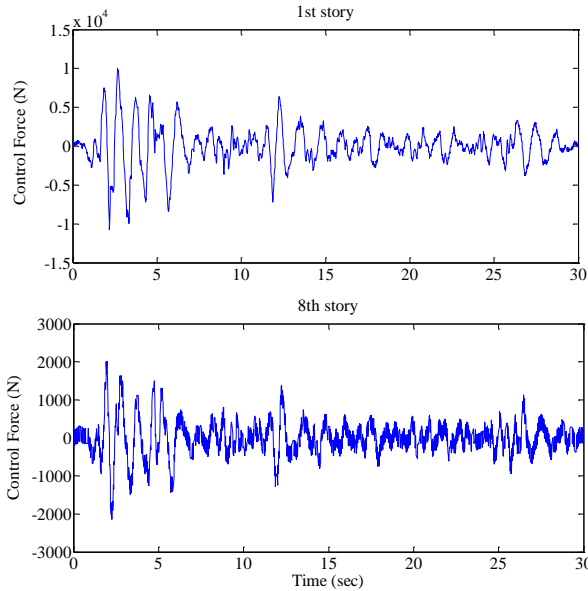


Fig. 14 Control force with SMC during El Centro earthquake

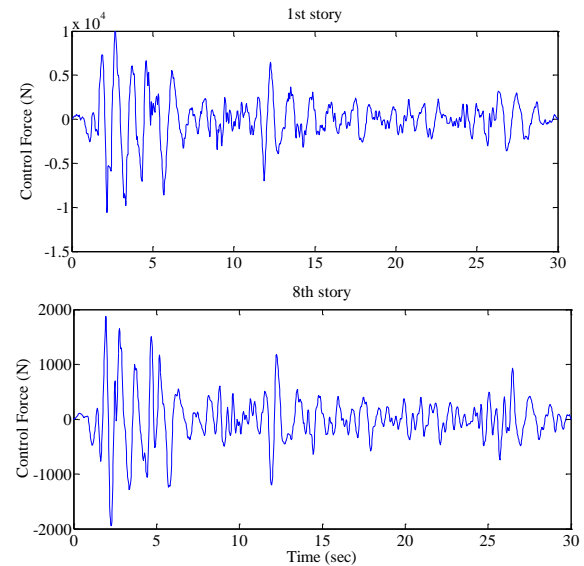


Fig. 16 Control force with CFSMC during El Centro earthquake

conventional SMC, the time histories for the control forces are represented in Figs. 14 and 15 for both floors. Considerable switches are present in the time histories of the control forces with the SMC, which can lead to reduced control accuracy and high wear of moving mechanical parts, thus preventing the actuators from generating the desired control force in a non-simulated situation.

Fig. 16 shows the forces for first and eighth stories during the El Centro excitation with the CFSMC, whereas Fig. 17 shows the same output for the Northridge excitation. Comparison with Figs. 14 and 15 allows concluding that chattering is effectively eliminated with the CFSMC due to the replacement of the sign function with the fuzzy output without losing accuracy. The maximum response quantities registered during both earthquakes-see Tables 4 and 5-are also significantly smaller with the CFSMC.

The performance of the control system given by the root mean square (RMS) of uncontrolled and controlled responses for both SMC and CFSMC methods is represented in Figs. 18 and 19. Even though the displacement responses in both approaches are identical, the chattering negatively impacts the RMS values obtained with the SMC, which is evident in Fig. 19.

Finally, an indication about the energy consumption of the control method can be derived from the RMS for the control forces shown in Fig. 20. The CFSMC requires smaller forces to achieve suitable dynamic performance in comparison to the SMC. The proposed method not only reduces the dynamic responses with less amount of energy, but also removes chattering in the actuator, which could cause a control system malfunction in practical applications.

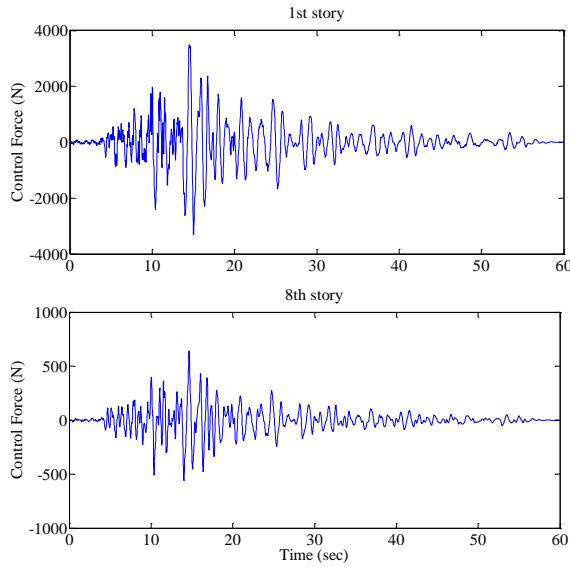


Fig. 17 Control force with CFSSMC during Northridge earthquake

Table 4 Maximum response quantities during El Centro earthquake

| | Story | No control | SMC | CFSSMC |
|---------------------------------|-------|------------|--------|--------|
| x (cm) | 1 | 4.63 | 2.41 | 2.41 |
| | 8 | 27.16 | 7.48 | 7.48 |
| \ddot{x} (cm/s ²) | 1 | 496 | 568 | 459 |
| | 8 | 1,230 | 792 | 643 |
| U (N) | 1 | - | 10,790 | 10,587 |
| | 8 | - | 2,141 | 1,940 |

Table 5 Maximum response quantities during Northridge earthquake

| | Story | No control | SMC | CFSSMC |
|---------------------------------|-------|------------|-------|--------|
| x (cm) | 1 | 1.81 | 0.93 | 0.93 |
| | 8 | 11.56 | 3.1 | 3.1 |
| \ddot{x} (cm/s ²) | 1 | 152 | 191 | 133 |
| | 8 | 375 | 385 | 241 |
| U (N) | 1 | - | 3,660 | 3,492 |
| | 8 | - | 839 | 641 |

6. Conclusions

A chattering-free sliding mode control (CFSSMC) methodology is presented in this paper to improve the performance of the conventional SMC. The proposed approach takes advantage of a fuzzy model for designing a chattering-free SMC effectively avoiding excessive switches. Moreover, using the concept of the sliding mode for constructing the fuzzy rules basis, a trial-and-error process is avoided. To validate the proposed method, the CFSSMC was employed to reduce the seismic responses of an eight-story building equipped with an active tendon system. Results demonstrate the performance of the proposed method against the SMC to eliminate chattering

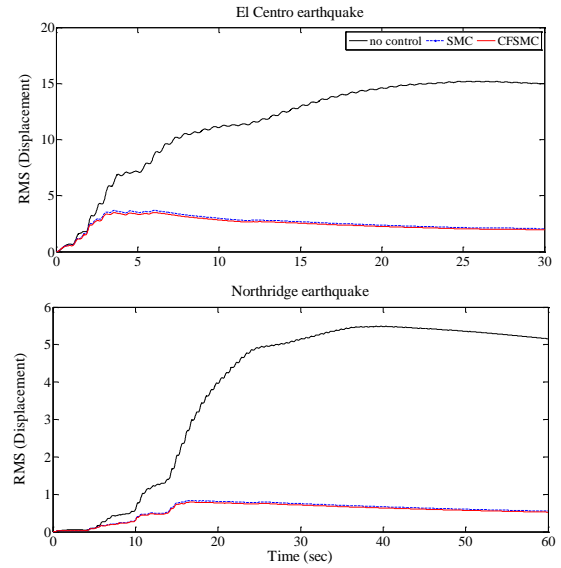


Fig. 18 RMS of displacements

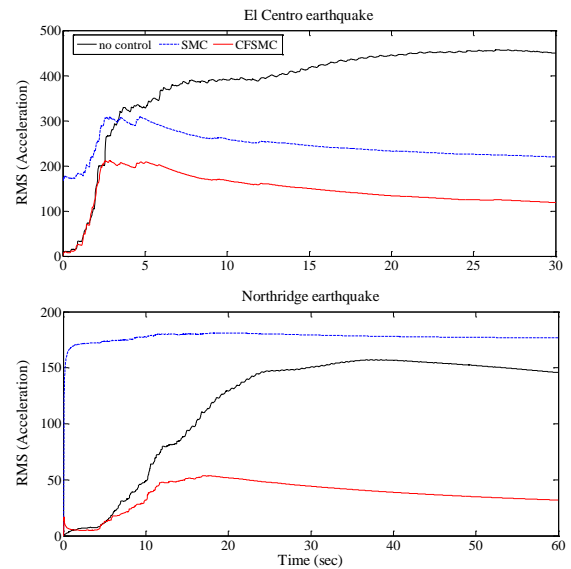


Fig. 19 RMS of accelerations

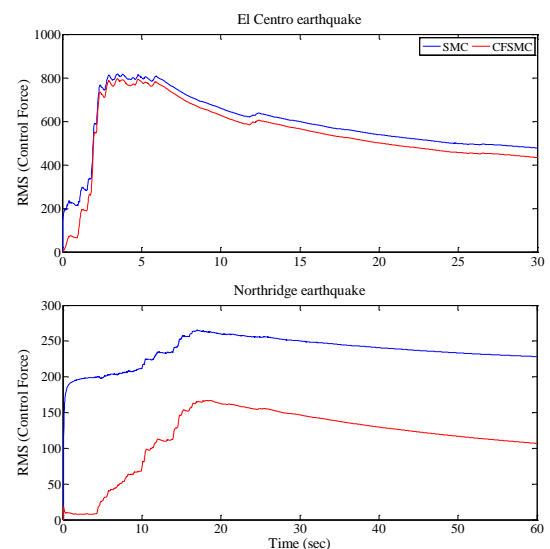


Fig. 20 RMS of control forces

with high accuracy, whilst reducing the dynamic responses. It was demonstrated that the CFSMC is an effective strategy for enhancing the performance of the conventional method in seismic isolation of structures.

While this study focussed on the dynamic response of structures due to seismic excitation, some important issues will require further studies to fully assess the proposed control strategy, such as stability analysis, nonlinearity, and uncertainty in the structural properties. The proposed method could also be extended to time-delay problems and structures with material deterioration under strong excitations.

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