Free vibration of functionally graded thin elliptic plates with various edge supports

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Abstract. In this article, free vibration of functionally graded (FG) elliptic plates subjected to various classical boundary conditions has been investigated. Literature review reveals no study has been performed based on functionally graded elliptic plates till date. The mechanical kinematic relations are considered based on classical plate theory. Rayleigh-Ritz technique is used to obtain the generalized eigenvalue problem. The material properties of the FG plate are assumed to vary along thickness direction of the constituents according to power-law form. Trial functions denoting the displacement components are expressed in simple algebraic polynomial forms which can handle any edge support. The objective is to study the effect of geometric configurations and gradation of constituent volume fractions on the natural frequencies. New results for frequency parameters are incorporated after performing a test of convergence. A comparison study is carried out with existing literature for validation in special cases. Three-dimensional mode shapes for circular and elliptic FG plates are also presented with various boundary conditions at the edges.

Keywords: vibration; elliptic functionally graded plate; Rayleigh-Ritz method; classical plate theory; mode shapes

1. Introduction

The functionally graded materials (FGMs) are advanced composite materials which are designed to achieve functional behavior with the variation of mechanical properties continuously from one surface to the other. Use of FG structural members can be observed in various engineering applications and in manufacturing industries viz. aerospace, biomedical, nuclear, automobile, space-plane project and steel industries. The concept of FGMs was first introduced in 1984 by a group of material scientists in Japan (Loy *et al.* 1999) while preparing a space–plane project as an exceptional material to withstand a very intense temperature deviation through comparatively less thickness. The primary constituents for these materials are metal with ceramic or a combination of materials. The study of dynamic characteristics of FGMs has also been gained considerable attention in research sectors during the past decades.

As present work is based on flexural vibration of circular and elliptic plates, one may follow various sources that are available viz. (Wang *et al.* 2000, Chakraverty 2009, Leissa 1969, Rao

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2004). Dynamic characteristics of isotropic circular and elliptic plates are quite well presented by different researchers throughout the globe using various computational techniques. Natural frequencies of simply supported elliptic plates are evaluated by Leissa (1967) by means of Rayleigh-Ritz technique. Mazumdar (1971) has computed fundamental frequencies of elliptic plates, for both clamped and simply supported edge supports, by the method of constant deflection lines. Leissa and Narita (1980) have analyzed natural frequencies of simply supported circular plates using Classical plate theory with ordinary and modified Bessel functions of the first kind. Galerkin method and Bolotin's method is used by Chen and Hwang (1988) to study dynamic stability of isotropic Mindlin circular plates subjected to periodic radial loads. The spline-finite-strip method with subparametric mapping concept is employed by Cheung et al. (1988) in static and free vibration analysis of arbitrary shaped plates. Transverse vibration of circular and elliptic plates are studied by Singh and Chakraverty (1991, 1992b, c) with three respective boundary conditions viz. completely free, simply supported and clamped. Axisymmetric vibration of circular and its analogous elliptic plates are examined by Rajalingham and Bhat (1993) using characteristic orthogonal polynomials. Rajalingham et al. (1994) have analyzed vibration of clamped elliptic plates using exact circular plate modes as shape functions in Rayleigh-Ritz method. Circular and elliptic plates with variable thickness is being investigated in Singh and Chakraverty (1994) with all three boundary conditions. Natural frequencies for free vibration of nonhomogeneous circular and elliptic plates using two dimensional orthogonal polynomials is studied in (Chakraverty and Petyt 1997). Liew et al. (1997) used differential quadrature method and linear shear deformation Mindlin theory to analyze axisymmetric free vibration characteristics of moderately thick circular plates. Three-dimensional vibration of circular and annular plates are analyzed in (Liu and Lee 2000) using finite element method and in Zhao et al. (2003) applying Chebyshev-Ritz method. Free vibration of solid circular plates is studied in (Wu and Liu 2001) and (Wu et al. 2002) applying generalized differential quadrature rule. Chakraverty et al. (2007) have provided vibration behavior of plates by using the effects of non-homogeneity.

While considering functionally graded circular and elliptic plates, one may find very few literature available for such analysis. Reddy et al. (1999) used first-order shear deformation Mindlin plate theory to study axisymmetric bending and stretching of functionally graded solid and annular circular plates. Buckling analysis is presented in (Najafizadeh and Eslami 2002) for radially loaded functionally graded solid circular plate subject to either clamped or simply supported edge conditions. Ma and Wang (2003) investigated axisymmetric large deflection bending and thermal post-buckling behavior of functionally graded circular plate under mechanical, thermal and combined thermo-mechanical loadings based on classical nonlinear von Karman plate theory. An inverse problem of a functionally graded elliptic plate is developed by Hsieh and Lee (2006) with large deflection and distributed boundary under uniform load. Prakash and Ganpathi (2006) have found free vibration characteristics and thermoelastic stability of functionally graded circular plates using finite element procedure. Semi-analytical solution methods are used in (Nie and Zhong 2007) to analyze three-dimensional free and forced vibration of functionally graded circular plates and in (Allahverdizadeh et al. 2008) to examine nonlinear free and forced vibration of thin circular functionally graded plate respectively. Saidi et al. (2009) have investigated axisymmetric bending and buckling of perfect functionally graded solid circular plates based on unconstrained third-order shear deformation plate theory. Benachour et al. (2011) have used a four variable refined plate theory for free vibration analysis of FG plates and a new hyperbolic theory is presented by El Meiche et al. (2011) for the buckling and free vibration

analysis of thick FG sandwich plates. Zhang (2013) has presented nonlinear bending analysis for functionally graded elliptical plates resting on two-parameter elastic foundations based on Reddy's higher-order shear deformation plate theory. A unified nonlocal shear deformation theory is proposed by Tounsi et al. (2013) to study bending, buckling and free vibration of nanobeams. Liu et al. (2013) have studied the free vibration problems of uniform Euler-Bernoulli beam using a modified differential transform method. Thermoelastic bending analysis of functionally graded sandwich plates has been developed respectively by Tounsi et al. (2013) using a refined trigonometric shear deformation theory and by Houari et al. (2013) using a new higher order shear and normal deformation theory. On the other hand, Bouderba et al. (2013) have implemented a refined trigonometric shear deformation theory in finding the thermomechanical bending response of FGM thick plates resting on Winkler-Pasternak elastic foundations. Nonlinear behavior of FG plates under thermal loads has been investigated by Bachir Bouiadjra et al. (2013) using an efficient sinusoidal shear deformation theory. Bessaim et al. (2013) have developed a new higher-order shear and normal deformation theory for the bending and free vibration analysis of sandwich plates with functionally graded isotropic face sheets. Free vibration of a sandwich curved beam with FG core is investigated by Fard (2014) based on 2D refined higher order beam theory. The bending response of FG plate resting on elastic foundation and subjected to hygro-thermo-mechanical loading has been studied by Zidi et al. (2014) by the use of four varibale refined plate theory. Belabed et al. (2014) have proposed an efficient and simple higher order shear and normal deformation theory to obtain analytical results for the bending and free vibration of simply supported FG plates and Hebali et al. (2014) have given a new quasi-3D hyperbolic shear deformation theory for the static and free vibration analysis of functionally graded plates. To the best of present authors' knowledge, no work is yet done on free vibration characteristics of functionally graded circular and elliptic plates.

Whence the objective of present work is to evaluate natural frequencies and mode shapes of functionally graded circular and elliptic plates within the framework of classical plate theory. Material properties are assumed to vary along thickness direction of FG constituents in simple power-law exponent form. Rayleigh-Ritz method is used in mathematical formulation to obtain the generalized eigenfrequency equation. Trial functions denoting the displacement components are expressed in simple algebraic polynomial forms which can handle any edge support. A test of convergence of present results is performed with a comparison study with existing literature for validation. New results for frequencies and three-dimensional mode shapes are also presented under various boundary conditions at the edges of elliptic FG plates.

2. Elliptic FG plate

Let us consider a functionally graded elliptic plate with semi-major axis a, semi-minor axis b and thickness h as shown in Fig. 1. Circular FG plate is a special case of elliptic FG plate while equating semi-major and semi-minor axes that is a=b.

Material properties of FG plate are assumed to vary along thickness direction according to power-law form as

$$\wp(z) = (\wp_c - \wp_m) \left(\frac{z}{h} + \frac{1}{2}\right)^k + \wp_m \tag{1}$$

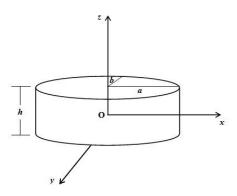


Fig. 1 A typical functionally graded elliptic plate element with Cartesian coordinates

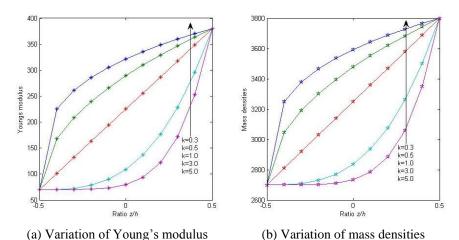


Fig. 2 Power-law variation of (a) Young's modulus and (b) mass densities of FG plate

where \wp_c and \wp_m denote the values of the material properties of the ceramic and metal constituents of the FG plate respectively. *k* (power-law exponent) is a non-negative variable parameter. According to this distribution, the bottom surface (z=-h/2) of FG plate is pure metal, whereas the top surface (z=h/2) is pure ceramic and for different values of *k* one can obtain different volume fractions of material plate. For our present formulations, Young's modulus (*E*) and mass densities (ρ) are taken into consideration as in Eq. (1) while other properties will remain constant through the thickness of the plate. It may be noted in the above Eq. (1) that $\wp = \wp_m$ at z=-h/2 and $\wp = \wp_c$ at z=h/2. Fig. 2 indicates the power-law variation of material properties of constituents of elliptic (or circular) FG plate.

3. Mathematical modelling

In this section, classical plate theory, mechanical kinematic relations and Rayleigh-Ritz method are used to obtain the generalized eigenvalue problem for free vibration of elliptic (or circular) functionally graded plates. An overview of these basic components are described as follows.

3.1 Classical plate theory

Classical or Kirchoff's plate theory (CPT) for the free vibration of functionally graded plate is based on the displacement field (Wang *et al.* 2000)

$$u_{x}(x, y, z) = -z \frac{\partial w}{\partial x}$$

$$u_{y}(x, y, z) = -z \frac{\partial w}{\partial y}$$

$$u_{z}(x, y, z) = w(x, y)$$
(2)

where u_x , u_y and u_z are the displacement components along x, y and z coordinate directions respectively and w is the transverse deflection of a point on the mid-plane (x-y plane). Transverse shear deformation is neglected in case of Kirchoff assumption that is deformation is due to bending and in-plane stretching.

3.2 Mechanical kinematic relations

In mechanics, the non-zero linear strains associated with the displacement field can be expressed as

$$\begin{cases} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{xy} \end{cases} = \begin{cases} \frac{\partial u_x}{\partial x} \\ \frac{\partial u_y}{\partial y} \\ \frac{\partial u_x}{\partial y} + \frac{\partial u_y}{\partial x} \end{cases} = \begin{cases} -z \frac{\partial^2 w}{\partial x^2} \\ -z \frac{\partial^2 w}{\partial y^2} \\ -2z \frac{\partial^2 w}{\partial x \partial y} \end{cases}$$
(3)

where ε_{xx} and ε_{yy} are the normal strains in x- and y- directions respectively and γ_{xy} is the shear strain. By assuming the material constituents of FG plate to obey the generalized Hooke's law, the constitutive or stress-strain relationships can be expressed in matrix form as

$$\begin{cases} \sigma_{xx} \\ \sigma_{yy} \\ \tau_{xy} \end{cases} = \begin{pmatrix} Q_{11} & Q_{12} & 0 \\ Q_{21} & Q_{22} & 0 \\ 0 & 0 & Q_{66} \end{pmatrix} \begin{pmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{xy} \end{pmatrix}$$
(4)

where σ_{xx} , σ_{yy} are the normal stresses and τ_{xy} is the shear stress and the reduced stiffness components, Q_{ij} are given by

$$Q_{11} = Q_{22} = \frac{E(z)}{1 - v^2}, \ Q_{12} = Q_{21} = \frac{vE(z)}{1 - v^2}, \ Q_{66} = \frac{E(z)}{2(1 + v)}$$

Here, E and v are Young's modulus and Poisson's ratio of the material constituents respectively. The strain energy U and kinetic energy T of the plate at any instant in cartesian

co-ordinates may be written as

$$U = \frac{1}{2} \int_{\Omega} \left[\int_{-h/2}^{h/2} \left(\sigma_{xx} \varepsilon_{xx} + \sigma_{yy} \varepsilon_{yy} + \tau_{xy} \gamma_{xy} \right) dz \right] dxdy$$
(5)

$$T = \frac{1}{2} \int_{\Omega} \left[\int_{-h/2}^{h/2} \rho(z) \left(\frac{\partial u_z}{\partial t} \right)^2 dz \right] dx dy$$
(6)

where Ω denotes the midplane (domain) of the elliptic FG plate.

Using Eqs. (2), (3) and (4) in Eqs. (5) and (6) lead to

$$U = \frac{1}{2} \int_{\Omega} \left[D_{11} \left\{ \left(\frac{\partial^2 w}{\partial x^2} \right)^2 + \left(\frac{\partial^2 w}{\partial y^2} \right)^2 \right\} + 2D_{12} \frac{\partial^2 w}{\partial x^2} \frac{\partial^2 w}{\partial y^2} + 4D_{66} \left(\frac{\partial^2 w}{\partial x \partial y} \right)^2 \right] dxdy$$
(7)
$$T = \frac{1}{2} \int_{\Omega} I_0 \left(\frac{\partial w}{\partial t} \right)^2 dxdy$$
(8)

where the stiffness coefficients in Eq. (7) are

$$(D_{11}, D_{12}, D_{66}) = \int_{-h/2}^{h/2} (Q_{11}, Q_{12}, Q_{66}) z^2 dz$$

and inertial coefficient, I_0 in Eq. (8) is

$$I_0 = \int_{-h/2}^{h/2} \rho(z) \,\mathrm{d}z.$$

The displacement component can be assumed harmonic type as $w(x,y,t)=W(x,y)\cos\omega t$ with W(x,y) is the maximum deflection and ω is the natural frequency of free vibration. Using the above harmonic motion, Eqs. (7) and (8) may be transformed into maximum strain (U_{max}) and kinetic energies (T_{max}) respectively as follows

$$U_{max} = \frac{1}{2} \int_{\Omega} \left[D_{11} \left\{ \left(\frac{\partial^2 W}{\partial x^2} \right)^2 + \left(\frac{\partial^2 W}{\partial y^2} \right)^2 \right\} + 2D_{12} \frac{\partial^2 W}{\partial x^2} \frac{\partial^2 W}{\partial y^2} + 4D_{66} \left(\frac{\partial^2 W}{\partial x \partial y} \right)^2 \right] dxdy \tag{9}$$

$$T_{max} = \frac{\omega^2}{2} \int_{\Omega} I_0 W^2 \,\mathrm{d}x \mathrm{d}y \tag{10}$$

3.3 Application of Rayleigh-Ritz method

One may express the transverse displacement (W(x,y)) as the sum of simple algebraic polynomials involving both x and y.

$$W(x, y) = \sum_{i=1}^{n} c_i \varphi_i(x, y)$$

	0	i l'i j				0				
i	1	2	3	4	5	6	7	8	9	10
ψ_i	1	x	у	x^2	xy	y^2	x^{3}	x^2y	xy^2	y^3

Table 1 Ten algebraic polynomials obtained from Pascal's triangle

where c_i are unknown constants to be determined and φ_i are the admissible functions, which satisfy the essential boundary conditions and can be represented as

$$\varphi_i(x, y) = f \psi_i(x, y), \quad i = 0, 1, 2, \dots, n$$

Here, *n* is the number of polynomials involved in the admissible functions. The function $f = \left(1 - \frac{x^2}{a^2} - \frac{y^2}{b^2}\right)^p$ with the exponent *p* controls various boundary conditions (BCs). The

parameter p=0,1 or 2 according as the elliptic (or circular) FG plate is free (F), simply supported (S) or clamped (C). Table 1 involves the components of ψ_i generated from Pascal's triangle.

Assuming constant Poisson's ratio (v), the Rayleigh quotient can be obtained by equating U_{max} and T_{max} as

$$\omega^{2} = \frac{\int_{\Omega} D_{11} \left[\left\{ \left(\frac{\partial^{2} W}{\partial x^{2}} \right)^{2} + \left(\frac{\partial^{2} W}{\partial y^{2}} \right)^{2} \right\} + 2\nu \frac{\partial^{2} W}{\partial x^{2}} \frac{\partial^{2} W}{\partial y^{2}} + 2(1-\nu) \left(\frac{\partial^{2} W}{\partial x \partial y} \right)^{2} \right] dxdy}{\int_{\Omega} I_{0} W^{2} dxdy}$$
(11)

The stiffness coefficient (D_{11}) and inertial coefficient (I_0) in Eq. (11) may be expressed as

$$\begin{split} D_{11} &= \int_{-h/2}^{h/2} Q_{11} z^2 \, \mathrm{d}z \\ &= \frac{1}{1 - v^2} \int_{-h/2}^{h/2} E(z) z^2 \, \mathrm{d}z \\ &= \frac{1}{1 - v^2} \int_{-h/2}^{h/2} \Biggl\{ (E_c - E_m) \Biggl(\frac{z}{h} + \frac{1}{2} \Biggr)^k + E_m \Biggr\} z^2 \, \mathrm{d}z \\ &= \frac{1}{1 - v^2} \Biggl[\int_{-h/2}^{h/2} \Biggl\{ (E_c - E_m) \Biggl(\frac{z}{h} + \frac{1}{2} \Biggr)^k \Biggr\} z^2 \, \mathrm{d}z + \int_{-h/2}^{h/2} E_m z^2 \, \mathrm{d}z \Biggr] \\ &= \frac{(E_c - E_m) h^3}{1 - v^2} \Biggl\{ \frac{1}{k + 3} - \frac{1}{k + 2} + \frac{1}{4(k + 1)} \Biggr\} + \frac{E_m h^3}{12(1 - v^2)}, \\ I_0 &= \int_{-h/2}^{h/2} \rho(z) \, \mathrm{d}z \\ &= \int_{-h/2}^{h/2} \Biggl\{ (\rho_c - \rho_m) \Biggl(\frac{z}{h} + \frac{1}{2} \Biggr)^k \Biggr\} \, \mathrm{d}z + \int_{-h/2}^{h/2} \rho_m \, \mathrm{d}z \\ &= \frac{(\rho_c - \rho_m) h}{k + 1} + \rho_m h. \end{split}$$

Accordingly, taking partial derivative of ω^2 with respect to unknown constants as follows

$$\frac{\partial \omega^2}{\partial c_i} = 0, \quad i = 1, 2, 3, \cdots, n \tag{12}$$

Further manipulation of Eq. (12) yields the generalized eigenvalue problem of the form

$$\left(\left[K \right]_{n \times n} - \lambda^2 \left[M \right]_{n \times n} \right) \left\{ \Delta \right\} = 0$$
(13)

where $[K]_{n \times n}$ and $[M]_{n \times n}$ are symmetric stiffness and inertia matrices respectively and $\{\Delta\}$ is the column vector of unknown generalized coefficients. Solutions of the eigenvalue problem, Eq. (13) gives the vibration characteristics viz. frequency parameters and mode shapes, for free vibration of elliptic (or circular) FG plate. As such, frequency parameters and mode shapes are incorporated in the next sections based on CPT taking various BCs. Test of convergence and validation of present results with existing literature are also performed.

4. Convergence study

In this part, convergence studies for frequency parameters of isotropic elliptic (or circular) plates (assuming k=0 in case of FG plates) are reported with respect to number of polynomials involved in the displacement component. The material properties of FG constituents are considered in Table 2.

Non-dimensional frequency parameters of elliptic FG plate may be written as

$$\lambda = \omega a^2 \sqrt{\frac{\rho_c h}{D_c}}$$
(14)

where D_c (flexural rigidity) = $\frac{E_c h^3}{12(1-v^2)}$ of the FG plate due to the deformation effect.

In Tables 3 to 4, convergence of first six frequency parameters of isotropic circular and elliptic plates are incorporated. Isotropic circular plate is considered in Table 3 and elliptic plate in Table 4 with a/b=2 taking clamped (C) and simply supported (S) edge supports. Rather than taking frequency parameters for combination of symmetric and antisymmetric modes separately, present study computes frequencies for all the modes at a time. This means that earlier authors used deflection function of odd-odd, even-odd, odd-even and even-even polynomials of x and y separately. Present authors have taken all the powers of x and y for the ease in computation in a single run. So the number of approximations may be seen less in previous works. It is interesting to note here that increase in number of polynomials in displacement component plays a crucial role

Properties	Unit	Aluminium (Al)	Alumina (Al ₂ O ₃)
E	GPa	70	380
ρ	kg/m ³	2700	3800
v	-	0.3	0.3

Table 2 Material properties of the FGM constituents

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BCs	Sources	λ_1	λ_2	λ_3	λ_4	λ_5	λ_6
	10×10	10.217	21.275	36.661	43.058	54.650	69.202
	13×13	10.216	21.275	35.609	41.210	54.650	69.202
С	16×16	10.216	21.266	34.941	39.921	52.479	64.682
	19×19	10.216	21.263	34.941	39.921	51.914	62.439
	20×20	10.216	21.261	34.941	39.921	51.209	61.407
	10×10	4.941	13.987	35.665	46.706	59.195	88.161
	13×13	4.938	13.987	30.391	39.456	59.195	88.161
S	16×16	4.935	13.941	25.986	30.503	46.102	59.195
	19×19	4.935	13.915	25.986	30.503	42.362	45.976
	20×20	4.935	13.899	25.986	30.503	40.915	42.362

Table 3 Convergence of frequency parameters for isotropic circular plate

Table 4 Convergence of frequency parameters for isotropic elliptic plate with a/b=2

BCs	Sources	λ_1	λ_2	λ_3	λ_4	λ_5	λ_6
	10×10	27.395	39.594	61.455	70.023	88.595	95.687
	13×13	27.378	39.594	56.329	70.023	88.595	89.812
С	16×16	27.378	39.503	56.328	70.023	78.029	88.665
	19×19	27.378	39.500	56.328	69.884	78.025	88.665
	20×20	27.378	39.499	56.328	69.884	78.020	88.665
	10×10	13.258	23.910	46.747	55.136	91.258	93.936
	13×13	13.218	23.910	39.388	46.747	76.705	93.936
S	16×16	13.214	23.696	39.366	46.747	62.309	64.636
	19×19	13.214	23.653	39.366	46.341	60.605	64.636
	20×20	13.214	23.645	39.366	46.341	60.497	64.636

in the conv ergence of frequency parameters irrespective of geometric configuration and edge support of the plate.

5. Verification of results

After satisfactory test of convergence of frequency parameters, we have performed a comparison study for frequency parameters of elliptic (or circular) plates with the existing literature. As there are not much articles related to free vibration of elliptic FG plates, it is worth taking isotropic plates to compare the results. First five frequency parameters of isotropic circular (a/b=1.0) and elliptic plate with different boundary conditions are compared in Table 5 assuming Poisson's ratio as v=0.3. It may be concluded that frequencies related to present study are in excellent agreement with the existing literature.

6. New results and discussion

In view of the above verification, new results for frequency parameters of elliptic FG plates

a/b	BCs	Sources	λ_1	λ_2	λ_3	λ_4	λ_5
	С	Present	10.2158	21.261	34.878	39.773	51.209
	С	Exact	10.216	21.260	34.878	39.773	-
	С	Leissa (1969)	10.2158	21.26	34.88	39.771	51.04
	С	Mazumdar (1971)	10.2151	-	-	-	-
	С	Cheung et al. (1988)	10.2062	21.27	34.94	40.21	52.05
	С	Singh and Chakraverty (1992c, 1994)	10.216	21.260	34.878	39.773	-
	С	Rajalingham et al. (1994)	10.2158	21.2604	34.8770	39.7711	51.0300
	С	Chakrverty and Petyt (1997)	10.216	21.260	34.878	39.773	51.030
	С	Wu and Liu (2001), Wu <i>et al.</i> (2002)	10.216	21.260	34.877	39.771	51.030
	С	Prakash and Ganpathi (2006)	10.213	21.259	34.849	-	50.974
	С	Chakraverty et al. (2007)	10.2158	21.2604	34.8770	39.7712	-
	S	Present	4.9351	13.899	25.619	29.737	40.915
	S	Exact	4.935	13.898	25.613	29.720	-
	S	Leissa (1969)	4.9351	13.8982	25.6173	29.7200	39.9573
1.0	S	Leissa and Narita (1980)	4.93515	13.8982	25.6133	29.7200	39.9573
1.0	S	Cheung et al. (1988)	4.927	13.88	25.54	29.84	40.30
	S	Singh and Chakraverty (1992b,1994)	4.9351	13.898	25.613	29.720	-
	S	Chakraverty and Petyt (1997) Wu and Liu (2001),	4.9351	13.898	25.613	29.720	39.957
	S	Wu et al. (2002)	4.935	13.898	25.613	29.720	39.957
	S	Prakash and Ganpathi (2006)	4.935	13.898	25.613	-	39.957
	S	Chakraverty et al. (2007)	4.9351	13.8982	25.6133	29.7201	-
	F	Present	5.3583	9.0035	12.5645	21.2331	22.1935
	F	Exact	5.3583	9.0031	12.439	20.475	-
	F	Leissa (1969)	5.253	9.084	12.23	20.52	21.6
	F	Singh and Chakraverty (1991,1994)	5.3583	9.0031	12.439	20.475	-
	F	Chakraverty and Petyt (1997)	5.3583	9.0031	12.439	20.475	21.835
	F	Wu and Liu (2001), Wu <i>et al.</i> (2002)	5.358	9.003	12.439	20.475	21.835
	F	Chakraverty et al. (2007)	5.3583	9.0031	12.4390	20.4746	-
	С	Present	27.377	39.499	55.985	69.863	78.020
	С	Leissa (1969)	27.378	-	-	-	-
	С	Mazumdar (1971)	27.471	-	-	-	-
	С	Singh and Chakraverty (1992c)	27.377	39.497	55.985	69.858	-
	С	Singh and Chakraverty (1994)	27.377	39.497	55.985	69.858	77.037
	С	Chakraverty <i>et al.</i> (2007)	27.3774	39.4974	55.9758	69.8580	-
2.0	S	Present	13.213	23.645	38.354	46.165	60.497
	Ŝ	Singh and Chakraverty (1992b)	13.213	23.641	38.354	46.151	57.625
	Ŝ	Singh and Chakraverty (1994)	13.213	23.641	38.354	46.151	-
	Š	Chakraverty <i>et al.</i> (2007)	13.2135	23.6410	38.3259	46.1504	-
	F	Present	6.6706	10.548	17.213	22.353	32.696
	F	Singh and Chakraverty (1994)	6.6706	10.548	16.923	22.019	-
	F	Chakraverty <i>et al.</i> (2007)	6.6705	10.5476	16.9212	22.019	

Table 5 Comparison of first five frequency parameters of isotropic elliptic plate

a/b	k	λ_1	λ_2	λ_3	λ_4	λ_5	λ_6
	0.0	10.216	21.261	34.878	39.773	51.209	61.407
	0.1	9.851	20.501	33.631	38.352	49.379	59.213
1.0	0.5	8.973	18.673	30.633	34.933	44.978	53.935
1.0	1.0	8.500	17.689	29.020	33.093	42.609	51.094
	2.0	8.125	16.909	27.741	31.634	40.730	48.841
	5.0	7.576	15.767	25.865	29.496	37.976	45.539
	0.0	17.129	28.472	41.487	44.392	57.038	65.369
	0.1	16.517	27.454	40.005	42.806	54.999	63.032
15	0.5	15.045	25.007	36.439	38.990	50.097	57.414
1.5	1.0	14.253	23.690	34.519	36.936	47.458	54.389
	2.0	13.624	22.646	32.998	35.308	45.366	51.992
	5.0	12.703	21.115	30.767	32.921	42.299	48.477
	0.0	27.377	39.499	55.985	69.863	78.020	88.074
	0.1	26.399	38.087	53.984	67.366	75.232	84.926
2.0	0.5	24.046	34.692	49.172	61.361	68.526	77.356
2.0	1.0	22.779	32.865	46.582	58.129	64.917	73.282
	2.0	21.775	31.416	44.529	55.566	62.055	70.051
	5.0	20.303	29.292	41.518	51.809	57.859	65.315
	0.0	56.801	71.626	90.350	116.81	147.34	150.18
	0.1	54.771	69.066	87.121	112.64	142.08	144.81
2.0	0.5	49.888	62.909	79.356	102.59	129.41	131.90
3.0	1.0	47.261	59.596	75.176	97.195	122.59	124.95
	2.0	45.177	56.969	71.862	92.909	117.19	119.44
	5.0	42.122	53.117	67.003	86.628	109.27	111.37

Table 6 Effect of aspect ratios (a/b) on frequency parameters of clamped elliptic FG plate

may be evaluated. Effect of aspect ratios (a/b) on first six frequency parameters are discussed in Tables 6 to 8 with various boundary conditions and gradation of properties in FG constituents. Clamped edge support is considered in Table 6 with different power-law exponents (k) to evaluate the first seven frequency parameters of elliptic FG plates. In a similar fashion, frequencies are computed for simply supported and completely free edge supports in Tables 7 and 8 respectively. In case of clamped and simply supported edge supports, it can be observed that frequencies are increasing with increase in aspect ratios and act in a reverse order with increase in power-law indices, whereas frequencies follow peculiar behavior in case of completely free FG elliptic plates that is frequencies are decreasing with increase in k and are showing fluctuations with increase in a/b at higher modes.

In Tables 9 to 10, effect of variation of Poisson's ratio (v) on natural frequencies of elliptic (or circular) FG plates supported by various edge conditions with fixed aspect ratio (a/b) and power-law index (k) are summarized. For both isotropic (k=0) and FG plates, it can be easily seen that frequencies remain constant for different values of v in case of clamped plates. Assuming simply supported, frequencies are increasing with increase in v, whereas frequencies are showing fluctuating order while considering free edge condition, keeping both a/b and k fixed. One may conclude that effects of edge supports and Poisson's ratio on free vibration response of FG elliptic plates is also quite similar to that of isotropic elliptic plates.

$ \begin{array}{c c c c c c c c c c c c c c c c c c c $		-				• • • •		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	a/b	k	λ_1	λ_2	λ_3	λ_4	λ_5	λ_6
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		0.0	4.935	13.899	25.619	29.736	40.915	51.072
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		0.1	4.759	13.402	24.703	28.674	39.453	49.247
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1.0	0.5	4.335	12.207	22.501	26.118	35.936	44.857
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1.0	1.0	4.106	11.564	21.316	24.742	34.043	42.495
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		2.0	3.925	11.055	20.376	23.651	32.542	40.621
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		5.0	3.659	10.307	18.999	22.052	30.342	37.875
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		0.0	8.282	17.835	27.452	31.805	41.409	52.246
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		0.1	7.986	17.198	26.471	30.668	39.929	50.379
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	15	0.5	7.274	15.665	24.112	27.935	36.369	45.889
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	1.5	1.0	6.891	14.839	22.842	26.463	34.454	43.472
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		2.0	6.587	14.186	21.835	25.297	32.935	41.555
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		5.0	6.142	13.227	20.358	23.586	30.708	38.746
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		0.0	13.213	23.645	38.354	46.165	60.497	62.848
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		0.1	12.741	22.799	36.984	44.515	58.335	60.602
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	2.0	0.5	11.606	20.767	33.687	40.547	53.135	55.199
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	2.0	1.0	10.994	19.673	31.913	38.412	50.337	52.292
0.0 27.081 40.146 57.050 84.282 98.673 116.06 0.1 26.114 38.711 55.011 81.269 95.146 111.91 3.0 0.5 23.786 35.260 50.108 74.026 86.665 101.94 1.0 22.533 33.403 47.469 70.127 82.101 96.569 2.0 21.539 31.931 45.376 67.035 78.481 92.312		2.0	10.509	18.806	30.506	36.718	48.118	49.987
0.126.11438.71155.01181.26995.146111.910.523.78635.26050.10874.02686.665101.941.022.53333.40347.46970.12782.10196.5692.021.53931.93145.37667.03578.48192.312		5.0	9.799	17.535	28.443	34.236	44.864	46.607
3.00.523.78635.26050.10874.02686.665101.941.022.53333.40347.46970.12782.10196.5692.021.53931.93145.37667.03578.48192.312		0.0	27.081	40.146	57.050	84.282	98.673	116.06
3.0 1.0 22.533 33.403 47.469 70.127 82.101 96.569 2.0 21.539 31.931 45.376 67.035 78.481 92.312		0.1	26.114	38.711	55.011	81.269	95.146	111.91
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2.0	0.5	23.786	35.260	50.108	74.026	86.665	101.94
	5.0	1.0	22.533	33.403	47.469	70.127	82.101	96.569
5.0 20.083 29.772 42.308 62.503 73.175 86.070		2.0	21.539	31.931	45.376	67.035	78.481	92.312
		5.0	20.083	29.772	42.308	62.503	73.175	86.070

Table 7 Effect of aspect ratios (a/b) on frequency parameters of simply supported elliptic FG plate

Table 8 Effect of aspect ratios (a/b) on frequency parameters of completely free elliptic FG plate

	-						
a/b	k	λ_1	λ_2	λ_3	λ_4	λ_5	λ_6
	0.0	5.358	9.003	12.564	21.233	22.193	37.564
	0.1	5.167	8.682	12.115	20.474	21.400	36.221
1.0	0.5	4.706	7.908	11.035	18.649	19.493	32.993
1.0	1.0	4.458	7.491	10.454	17.667	18.466	31.255
	2.0	4.262	7.161	9.993	16.888	17.652	29.877
	5.0	3.974	6.677	9.318	15.746	16.459	27.857
	0.0	6.477	7.986	16.309	16.510	17.767	29.971
	0.1	6.245	7.701	15.726	15.920	17.132	28.899
1.5	0.5	5.689	7.014	14.324	14.501	15.605	26.323
1.5	1.0	5.389	6.645	13.569	13.737	14.783	24.937
	2.0	5.151	6.352	12.971	13.132	14.131	23.838
	5.0	4.803	5.922	12.094	12.244	13.176	22.226
	0.0	6.671	10.548	17.212	22.353	27.773	32.696
2.0	0.1	6.432	10.171	16.596	21.554	26.780	31.527
2.0	0.5	5.859	9.264	15.117	19.633	24.393	28.717
	1.0	5.550	8.776	14.321	18.599	23.108	27.205

Tuble o conti	maea						
2.0	2.0	5.306	8.389	13.689	17.779	22.089	26.005
2.0	5.0	4.947	7.822	12.764	16.577	20.596	24.247
	0.0	6.757	15.615	17.618	31.415	33.961	51.246
	0.1	6.516	15.057	16.988	30.292	32.748	49.414
3.0	0.5	5.935	13.714	15.474	27.592	29.829	45.009
5.0	1.0	5.622	12.992	14.659	26.139	28.258	42.639
	2.0	5.374	12.419	14.013	24.986	27.012	40.759
	5.0	5.011	11.579	13.065	23.297	25.185	38.003

Table 8 Continued

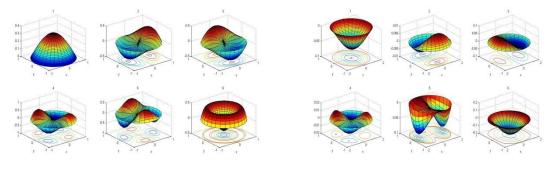
Table 9 Effect of Poisson's ratio (v) on frequency parameters of elliptic FG plate with various BCs for k=0

a/b	BC	v	λ_1	λ_2	λ_3	λ_4	λ_5	λ_6
		0.00	10.216	21.260	34.878	39.773	51.209	61.407
	С	0.25	10.216	21.260	34.878	39.773	51.209	61.407
	C	0.33	10.216	21.260	34.878	39.773	51.209	61.407
		0.50	10.216	21.260	34.878	39.773	51.209	61.407
		0.00	4.4436	13.502	25.249	29.379	40.519	50.644
1.0	S	0.25	4.8601	13.835	25.559	29.678	40.851	51.002
1.0	3	0.33	4.9790	13.936	25.654	29.771	40.953	51.114
		0.50	5.2127	14.141	25.849	29.963	41.166	51.347
	F	0.00	6.1531	8.2441	14.148	20.758	24.659	37.779
		0.25	5.5112	8.8902	12.881	21.158	22.701	37.599
	Г	0.33	5.2620	9.0692	12.363	21.277	21.867	37.543
		0.50	4.6404	9.4141	11.021	19.648	21.519	34.641
		0.00	27.377	39.499	55.985	69.862	78.019	88.074
	C	0.25	27.377	39.499	55.985	69.862	78.019	88.074
	С	0.33	27.377	39.499	55.985	69.862	78.019	88.074
		0.50	27.377	39.499	55.985	69.862	78.019	88.074
		0.00	12.646	22.829	37.367	45.803	59.139	62.416
2.0	S	0.25	13.125	23.519	38.201	46.106	60.283	62.777
2.0	3	0.33	13.265	23.718	38.444	46.201	60.624	62.889
		0.50	13.546	24.115	38.930	46.399	61.314	63.126
		0.00	7.060	12.269	18.235	25.329	27.675	34.809
	Б	0.25	6.778	10.870	17.473	22.938	27.775	33.196
	F	0.33	6.597	10.346	17.034	21.981	27.768	32.359
		0.50	6.037	9.0676	15.683	19.540	27.710	29.828

Three-dimensional mode shapes of FG circular and elliptic plates are plotted in Figs. 3 to 5 taking various BCs with k=1. Fig. 3 is meant for clamped FG plates. In a similar fashion, Fig. 4 is for simply supported and Fig. 5 is for completely free FG plates respectively. Looking into the deflected shapes of mode shapes, it is easy to predict the edge support to be clamped, simply supported or free, irrespective of the geometry and power-law exponent used in gradation.

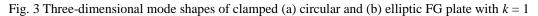
a/b	BC	v	λ_1	λ_2	λ_3	λ_4	λ_5	λ_6
		0.00	8.5001	17.689	29.019	33.093	42.609	51.094
	C	0.25	8.5001	17.689	29.019	33.093	42.609	51.094
	С	0.33	8.5001	17.689	29.019	33.093	42.609	51.094
		0.50	8.5001	17.689	29.019	33.093	42.609	51.094
		0.00	3.6973	11.234	21.009	24.445	33.715	42.139
1.0	S	0.25	4.0439	11.512	21.266	24.694	33.989	42.436
1.0	3	0.33	4.1428	11.596	21.346	24.771	34.075	42.529
		0.50	4.3372	11.766	21.508	24.931	34.253	42.724
		0.00	5.1197	6.8595	11.772	17.272	20.517	31.434
	F	0.25	4.5856	7.3971	10.718	17.605	18.889	31.284
	Г	0.33	4.3783	7.5461	10.286	17.704	18.194	31.238
		0.50	3.8610	7.8330	9.1701	16.348	17.9045	28.823
		0.00	22.779	32.865	46.582	58.129	64.916	73.282
	C	0.25	22.779	32.865	46.582	58.129	64.916	73.282
	С	0.33	22.779	32.865	46.582	58.129	64.916	73.282
		0.50	22.779	32.865	46.582	58.129	64.916	73.282
		0.00	10.522	18.995	31.092	38.111	49.207	51.933
2.0	S	0.25	10.921	19.569	31.785	38.362	50.158	52.234
2.0	3	0.33	11.038	19.735	31.988	38.441	50.442	52.328
		0.50	11.271	20.065	32.392	38.607	51.017	52.524
		0.00	5.874	10.208	15.173	21.075	23.027	28.962
		0.25	5.639	9.0445	14.538	19.085	23.111	27.621
		0.33	5.489	8.6086	14.173	18.289	23.105	26.925
		0.50	5.023	7.5447	13.049	16.258	23.056	24.819

Table 10 Effect of Poisson's ratio (v) on frequency parameters of elliptic FG plate with various BCs for k=1



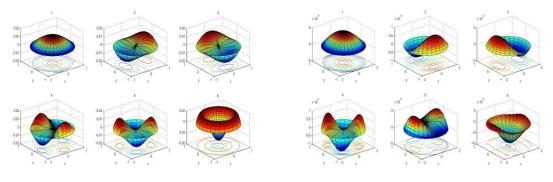
(a) FG circular plate

(b) FG elliptic plate



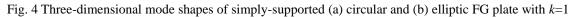
7. Conclusions

The analysis of natural frequencies and mode shapes of functionally graded elliptic and circular plates with various boundary conditions based on classical plate theory is proposed in the present



(a) FG circular plate

(b) FG elliptic plate



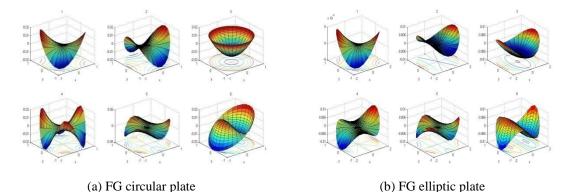


Fig. 5Three-dimensional mode shapes of free (a) circular and (b) elliptic FG plate with k=1

investigation. Generalized eigenfrequency equation for free vibration can be obtained by means of Rayleigh-Ritz method. Trial functions denoting displacement component may handle any boundary condition very easily. Looking into the present modelling and results, one may conclude as follows.

• The aspect ratios (a/b), power-law indices (k) and different material distributions play key factors to study free vibration characteristics of FG elliptic (or circular) plates.

• In Rayleigh-Ritz method, increase in the number of polynomials (*n*) play a crucial role in the convergence of frequency parameters.

• From Tables and Figers, it is observed that the frequencies are increasing with increase in aspect ratios for a fixed power-law index and are decreasing with increase in power-law exponents for a fixed aspect ratio while assuming clamped and simply supported BCs, whereas a fluctuating order of frequencies can be seen in case of free FG elliptic plates.

• Assuming effect of Poisson's ratio (v), one may see that frequencies are independent of v for clamped elliptic plates. But frequencies are increasing with increase in v in case of simply supported and are showing fluctuating behavior while considering free edge condition, keeping both a/b and k fixed.

• Other shear deformation plate theories can also be extended easily following the above analysis.

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