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# A simple hyperbolic shear deformation theory for vibration analysis of thick functionally graded rectangular plates resting on elastic foundations

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**Abstract.** A simple hyperbolic shear deformation theory taking into account transverse shear deformation effects is proposed for the free flexural vibration analysis of thick functionally graded plates resting on elastic foundations. By considering further supposition, the present formulation introduces only four unknowns and its governing equations are therefore reduced. Hamilton's principle is employed to obtain equations of motion and Navier-type analytical solutions for simply-supported plates are compared with the available solutions in literature to check the accuracy of the proposed theory. Numerical results are computed to examine the effects of the power-law index and side-to-thickness ratio on the natural frequencies.

Keywords: shear deformation theory; vibration; functionally graded plate

# 1. Introduction

Functionally graded materials (FGMs) are a type of advanced composite materials that contain higher mechanical response than homogeneous material made of identical constituents. Such kind material are obtained by a continuously graded distribution of the volume fractions of the constituents (Koizumi 1997), the FGM is thus suitable for diverse applications, such as thermal coatings of barrier for ceramic engines, gas turbines, nuclear fusions, spacecraft heat shields, heat exchanger tubes, optical thin layers, biomaterial electronics, etc (Akbaş 2015, Bennai *et al.* 2015, Arefi 2015, Ait Atmane *et al.* 2015a, Belkorissat *et al.* 2015, Ebrahimi and Dashti 2015, Darılmaz *et al.* 2015, Bouguenina *et al.* 2015, Boukhari *et al.* 2016, Ebrahimi and Habibi 2016, Hadji *et al.* 2016, Kar *et al.* 2016, Moradi-Dastjerdi 2016, Trinh *et al.* 2016).

Currently, many functionally graded (FG) plate structures which have been employed for engineering fields led to the development of various plate models to accurately examine the static, buckling and vibration responses of FG structures (Benachour *et al.* 2011, Jha *et al.* 2013a, Hadji

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et al. 2014, Belabed et al. 2014, Hebali et al. 2014, Klouche Djedid et al. 2014, Nguyen et al. 2015, Tagrara et al. 2015, Pradhan and Chakraverty 2015, Hadji and Adda Bedia 2015a, b, Kar and Panda 2015, Hamidi et al. 2015, Bennoun et al. 2016, Eltaher et al. 2016). The classical plate theory (CPT) is based on the supposition that straight lines which are normal to the neutral surface before deformation remain straight and normal to the neutral surface after deformation. Since the transverse shear deformation is neglected (Feldman and Aboudi 1997, Mahdavian 2009, Chen et al. 2006, Baferani *et al.* 2011a), it cannot be suitable for the investigating of moderately thick or thick plates in which transverse shear deformation effects are more important. For FG thick and moderately thick plates; the first-order shear deformation theory (FSDT) has been employed (Praveen and Reddy 1998, Croce and Venini 2004, Efraim and Eisenberger 2007, Zhao et al. 2009). In such formulation, in-plane displacements are linearly varied within the thickness and need a shear correction coefficient to correct the unrealistic distribution of the transverse shear stresses and shear strains across the thickness. To avoid the employment of the shear correction coefficient and predict a better distribution of the transverse shear deformation in FG plates, higher-order shear deformation plate theories (HSDTs) have been developed (Reddy 2000, 2011, Pradyumna and Bandyopadhyay 2008, Jha et al. 2013b, Neves et al. 2013, Talha and Singh 2010, Chen et al. 2009, Mantari and Soares 2012, 2013, Houari et al. 2013, Matsunaga 2008, Tounsi et al. 2013, Mahi et al. 2015, Bellifa et al. 2016, Ait Yahia et al. 2015, Al-Basyouni et al. 2015, Bourada et al. 2015, Attia et al. 2015, Merazi et al. 2015, Abdelhak et al. 2015). However, some of these HSDTs are computational costs due to of number of additional variables incorporated to the theory (Pradyumna and Bandyopadhyay 2008, Jha et al. 2013a, b, Neves et al. 2013, Reddy 2011, Talha and Singh 2010). Thus, a simple higher-order shear deformation theory presented in this article is necessary.

This article aims to propose a simple higher-order shear deformation theory for free vibration response of FG plates resting on elastic foundation. By considering a further assumption to the existing higher-order shear deformation model, the proposed theory involves only four unknowns and their governing equations are consequently reduced. Pasternak model is employed to simulate the interactions between the plate and elastic foundation. Equations of motion are obtained via Hamilton's principle. Analytical solutions of simply supported plates are proposed. The computed results are compared with the existing solutions to check the accuracy of present formulation in predicting the vibration behavior of FG plates resting on elastic foundation.

## 2. Theoretical formulations

A FG rectangular plate with length, width and uniform thickness equal to a, b and h respectively is shown in Fig. 1. The FG plate is composed by a mixture of ceramic and metal components whose material characteristics change across the plate thickness with a power law distribution of the volume fractions of the constituents of the two materials as (Akavci 2015, Ahouel *et al.* 2016)

$$E(z) = E_m + \left(E_c - E_m\right) \left(\frac{1}{2} + \frac{z}{h}\right)^p$$
(1a)

$$\rho(z) = \rho_m + \left(\rho_c - \rho_m\right) \left(\frac{1}{2} + \frac{z}{h}\right)^p \tag{1b}$$



Fig. 1 Schematic representation of a rectangular FG plate resting on elastic foundation

where the subscripts *m* and *c* denote the metallic and ceramic components, respectively; and *p* is the power law exponent. The value of *p* equal to zero indicates a fully ceramic plate, whereas infinite *p* represents a fully metallic plate. Since the influences of the variation of Poisson's ratio *v* on the behavior of FG plates are very small (Yang *et al.* 2005, Kitipornchai *et al.* 2006, Bourada *et al.* 2012, Ould Larbi *et al.* 2013, Saidi *et al.* 2013, Bousahla *et al.* 2014, Fekrar *et al.* 2014, Bouchafa *et al.* 2015, Larbi Chaht *et al.* 2015, Sallai *et al.* 2015, Zemri *et al.* 2015, Meradjah *et al.* 2015, Bouderba *et al.* 2016, Laoufi *et al.* 2016), it is supposed to be constant for convenience.

#### 2.1 Kinematics and strains

In this work, further simplifying supposition are made to the conventional HSDT so that the number of unknowns is reduced. The displacement field of the conventional HSDT is given by

$$u(x, y, z, t) = u_0(x, y, t) - z \frac{\partial w_0}{\partial x} + f(z)\theta_x(x, y, t)$$
(2a)

$$v(x, y, z, t) = v_0(x, y, t) - z \frac{\partial w_0}{\partial y} + f(z)\theta_y(x, y, t)$$
(2b)

$$w(x, y, z, t) = w_0(x, y, t)$$
 (2c)

where  $u_0$ ;  $v_0$ ;  $w_0$ ,  $\theta_x$ ,  $\theta_y$  are five unknown displacements of the mid-plane of the plate, f(z) denotes shape function representing the variation of the transverse shear strains and stresses within the thickness. By considering that  $\theta_x = -\partial \varphi(x, y) / \partial x$  and  $\theta_y = -\partial \varphi(x, y) / \partial y$ , the displacement field of the present model can be expressed in a simpler form as (Draiche *et al.* 2014)

$$u(x, y, z, t) = u_0(x, y, t) - z \frac{\partial w_0}{\partial x} - f(z) \frac{\partial \varphi}{\partial x}$$
(3a)

$$v(x, y, z, t) = v_0(x, y, t) - z \frac{\partial w_0}{\partial y} - f(z) \frac{\partial \varphi}{\partial y}$$
(3b)

$$w(x, y, z, t) = w_0(x, y, t)$$
 (3c)

where the shape function f(z) is chosen according to Mahi *et al.* (2015) as

$$f(z) = \frac{h}{2} \tanh\left(2\frac{z}{h}\right) - \frac{4}{3\cosh^{2}(1)}\left(\frac{z^{3}}{h^{2}}\right)$$
(4)

Clearly, the displacement field in Eq. (3) considers only four unknowns ( $u_0$ ,  $v_0$ ,  $w_0$  and  $\varphi$ ). The nonzero strains associated with the displacement field in Eq. (3) are

$$\begin{cases} \varepsilon_{x} \\ \varepsilon_{y} \\ \gamma_{xy} \end{cases} = \begin{cases} \varepsilon_{x}^{0} \\ \varepsilon_{y}^{0} \\ \gamma_{xy}^{0} \end{cases} + z \begin{cases} k_{x}^{b} \\ k_{y}^{b} \\ k_{xy}^{b} \end{cases} + f(z) \begin{cases} k_{x}^{s} \\ k_{y}^{s} \\ k_{xy}^{s} \end{cases}, \quad \begin{cases} \gamma_{yz} \\ \gamma_{xz} \end{cases} = g(z) \begin{cases} \gamma_{yz}^{0} \\ \gamma_{xz}^{0} \end{cases}, \tag{5}$$

where

$$\begin{cases} \varepsilon_{x}^{0} \\ \varepsilon_{y}^{0} \\ \gamma_{xy}^{0} \end{cases} = \begin{cases} \frac{\partial u_{0}}{\partial x} \\ \frac{\partial v_{0}}{\partial x} \\ \frac{\partial v_{0}}{\partial y} + \frac{\partial v_{0}}{\partial x} \end{cases}, \quad \begin{cases} k_{x}^{b} \\ k_{y}^{b} \\ k_{xy}^{b} \end{cases} = \begin{cases} -\frac{\partial^{2} w_{0}}{\partial x^{2}} \\ -\frac{\partial^{2} w_{0}}{\partial y^{2}} \\ -2\frac{\partial^{2} w_{0}}{\partial x \partial y} \end{cases}, \quad \begin{cases} k_{x}^{s} \\ k_{y}^{s} \\ k_{xy}^{s} \end{cases} = \begin{cases} -\frac{\partial^{2} \varphi}{\partial x^{2}} \\ -\frac{\partial^{2} \varphi}{\partial y^{2}} \\ -2\frac{\partial^{2} \varphi}{\partial x \partial y} \end{cases}, \quad \begin{cases} \gamma_{yz}^{0} \\ \gamma_{xz}^{0} \end{cases} = \begin{cases} \frac{\partial \varphi}{\partial y} \\ \frac{\partial \varphi}{\partial x} \end{cases}, \quad (6a)$$

and

$$g(z) = -\frac{df(z)}{dz}$$
(6b)

For elastic and isotropic FGMs, the constitutive relations can be expressed as

$$\begin{cases} \sigma_{x} \\ \sigma_{y} \\ \tau_{xy} \\ \tau_{yz} \\ \tau_{xz} \end{cases} = \begin{bmatrix} C_{11} & C_{12} & 0 & 0 & 0 \\ C_{12} & C_{22} & 0 & 0 & 0 \\ 0 & 0 & C_{66} & 0 & 0 \\ 0 & 0 & 0 & C_{44} & 0 \\ 0 & 0 & 0 & 0 & C_{55} \end{bmatrix} \begin{bmatrix} \varepsilon_{x} \\ \varepsilon_{y} \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{xz} \end{bmatrix}$$
(7)

where  $(\sigma_x, \sigma_y, \tau_{xy}, \tau_{yz}, \tau_{xz})$  and  $(\varepsilon_x, \varepsilon_y, \gamma_{xy}, \gamma_{yz}, \gamma_{xz})$  are the stress and strain components, respectively. Using the material properties defined in Eq. (1), stiffness coefficients,  $C_{ij}$ , can be written as

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$$C_{11} = C_{22} = \frac{E(z)}{1 - v^2}, \quad C_{12} = \frac{v E(z)}{1 - v^2}, \quad C_{44} = C_{55} = C_{66} = \frac{E(z)}{2(1 + v)},$$
 (8)

## 2.2 Equations of motion

Hamilton's principle is herein employed to determine the equations of motion

$$0 = \int_{0}^{t} (\delta U + \delta V - \delta K) dt$$
(9)

where  $\delta U$  is the variation of strain energy;  $\delta V$  is the variation of work done; and  $\delta K$  is the variation of kinetic energy.

The variation of strain energy of the plate is computed by

$$\delta U = \int_{V} \left[ \sigma_x \delta \varepsilon_x + \sigma_y \delta \varepsilon_y + \tau_{xy} \delta \gamma_{xy} + \tau_{yz} \delta \gamma_{yz} + \tau_{xz} \delta \gamma_{xz} \right] dV$$
  
$$= \int_{A} \left[ N_x \delta \varepsilon_x^0 + N_y \delta \varepsilon_y^0 + N_{xy} \delta \gamma_{xy}^0 + M_x^b \delta k_x^b + M_y^b \delta k_y^b + M_{xy}^b \delta k_{xy}^b \right]$$
  
$$+ M_x^s \delta k_x^s + M_y^s \delta k_y^s + M_{xy}^s \delta k_{xy}^s + S_{yz}^s \delta \gamma_{yz}^s + S_{xz}^0 \delta \gamma_{xz}^0 \right] dA = 0$$
 (10)

where A is the top surface and the stress resultants N, M, and S are defined by

$$\left(N_{i}, M_{i}^{b}, M_{i}^{s}\right) = \int_{-h/2}^{h/2} (1, z, f) \sigma_{i} dz, \quad (i = x, y, xy) \text{ and } \left(S_{xz}^{s}, S_{yz}^{s}\right) = \int_{-h/2}^{h/2} g(\tau_{xz}, \tau_{yz}) dz \tag{11}$$

The variation of the potential energy of elastic foundation can be calculated by

$$\delta V = \int_{A} f_e \delta w_0 dA \tag{12}$$

where  $f_e$  is the density of reaction force of foundation. For the Pasternak foundation model (Bounouara *et al.* 2016, Abdelbari *et al.* 2016, Ait Atmane *et al.* 2016, Chikh *et al.* 2016, Bakora and Tounsi 2015, Tebboune *et al.* 2015, Meksi *et al.* 2015, Ait Amar Meziane *et al.* 2014, Zidi *et al.* 2014, Khalfi *et al.* 2014, Bouderba *et al.* 2013)

$$f_e = K_W w - K_{S1} \frac{\partial^2 w}{\partial x^2} - K_{S2} \frac{\partial^2 w}{\partial y^2}$$
(13)

where  $K_W$  is the modulus of subgrade reaction (elastic coefficient of the foundation) and  $K_{S1}$  and  $K_{S2}$  are the shear moduli of the subgrade (shear layer foundation stiffness). If foundation is homogeneous and isotropic, we will get  $K_{S1} = K_{S2} = K_S$ . If the shear layer foundation stiffness is

neglected, Pasternak foundation becomes a Winkler foundation.

The variation of kinetic energy of the plate can be expressed as

$$\begin{split} \delta K &= \int_{V} \left[ \dot{u} \delta \dot{u} + \dot{v} \delta \dot{v} + \dot{w} \delta \dot{w} \right] \rho(z) \, dV \\ &= \int_{A} \left\{ I_{0} \left[ \dot{u}_{0} \delta \dot{u}_{0} + \dot{v}_{0} \delta \dot{v}_{0} + \dot{w}_{0} \delta \dot{w}_{0} \right] \\ &- I_{1} \left( \dot{u}_{0} \frac{\partial \delta \dot{w}_{0}}{\partial x} + \frac{\partial \dot{w}_{0}}{\partial x} \delta \dot{u}_{0} + \dot{v}_{0} \frac{\partial \delta \dot{w}_{0}}{\partial y} + \frac{\partial \dot{w}_{0}}{\partial y} \delta \dot{v}_{0} \right) \\ &- J_{1} \left( \dot{u}_{0} \frac{\partial \delta \dot{\varphi}}{\partial x} + \frac{\partial \dot{\varphi}}{\partial x} \delta \dot{u}_{0} + \dot{v}_{0} \frac{\partial \delta \dot{\varphi}}{\partial y} + \frac{\partial \dot{\varphi}}{\partial y} \delta \dot{v}_{0} \right) \\ &+ I_{2} \left( \frac{\partial \dot{w}_{0}}{\partial x} \frac{\partial \delta \dot{w}_{0}}{\partial x} + \frac{\partial \dot{w}_{0}}{\partial y} \frac{\partial \delta \dot{w}_{0}}{\partial y} \right) + K_{2} \left( \frac{\partial \dot{\varphi}}{\partial x} \frac{\partial \delta \dot{\varphi}}{\partial x} + \frac{\partial \dot{\varphi}}{\partial y} \frac{\partial \delta \dot{\varphi}}{\partial y} \right) \\ &+ J_{2} \left( \frac{\partial \dot{w}_{0}}{\partial x} \frac{\partial \delta \dot{\varphi}}{\partial x} + \frac{\partial \dot{\varphi}}{\partial x} \frac{\partial \delta \dot{w}_{0}}{\partial x} + \frac{\partial \dot{w}_{0}}{\partial y} \frac{\partial \delta \dot{\varphi}}{\partial y} + \frac{\partial \dot{\varphi}}{\partial y} \frac{\partial \delta \dot{w}_{0}}{\partial y} \right) \right\} dA \end{split}$$

where dot-superscript convention indicates the differentiation with respect to the time variable t;  $\rho(z)$  is the mass density given by Eq. (1b); and  $(I_i, J_i, K_i)$  are mass inertias expressed by

$$(I_0, I_1, I_2) = \int_{-h/2}^{h/2} (1, z, z^2) \rho(z) dz$$
(15a)

$$(J_1, J_2, K_2) = \int_{-h/2}^{h/2} (f, z f, f^2) \rho(z) dz$$
(15b)

Substituting Eqs. (10), (12), and (14) into Eq. (9), integrating by parts, and collecting the coefficients of  $\delta u_0$ ,  $\delta v_0$ ,  $\delta w_0$ , and  $\delta \varphi$ ; the following equations of motion are obtained

$$\delta u_{0} : \frac{\partial N_{x}}{\partial x} + \frac{\partial N_{xy}}{\partial y} = I_{0}\ddot{u}_{0} - I_{1}\frac{\partial \ddot{w}_{0}}{\partial x} - J_{1}\frac{\partial \ddot{\varphi}}{\partial x}$$

$$\delta v_{0} : \frac{\partial N_{xy}}{\partial x} + \frac{\partial N_{y}}{\partial y} = I_{0}\ddot{v}_{0} - I_{1}\frac{\partial \ddot{w}_{0}}{\partial y} - J_{1}\frac{\partial \ddot{\varphi}}{\partial y}$$

$$\delta w_{0} : \frac{\partial^{2}M_{x}^{b}}{\partial x^{2}} + 2\frac{\partial^{2}M_{xy}^{b}}{\partial x\partial y} + \frac{\partial^{2}M_{y}^{b}}{\partial y^{2}} - f_{e} = I_{0}\ddot{w}_{0} + I_{1}\left(\frac{\partial \ddot{u}_{0}}{\partial x} + \frac{\partial \ddot{v}_{0}}{\partial y}\right) - I_{2}\nabla^{2}\ddot{w}_{0} - J_{2}\nabla^{2}\ddot{\varphi}$$

$$\delta \varphi : \frac{\partial^{2}M_{x}^{s}}{\partial x^{2}} + 2\frac{\partial^{2}M_{xy}^{s}}{\partial x\partial y} + \frac{\partial^{2}M_{y}^{s}}{\partial y^{2}} + \frac{\partial S_{xz}^{s}}{\partial x} + \frac{\partial S_{yz}^{s}}{\partial y} = J_{1}\left(\frac{\partial \ddot{u}_{0}}{\partial x} + \frac{\partial \ddot{v}_{0}}{\partial y}\right) - J_{2}\nabla^{2}\ddot{w}_{0} - K_{2}\nabla^{2}\ddot{\varphi}$$
(16)

where  $\nabla^2 = \partial^2 / \partial x^2 + \partial^2 / \partial x^2$  is the Laplacian operator in two-dimensional Cartesian coordinate

system.

Substituting Eq. (5) into Eq. (7) and the subsequent results into Eqs. (11), the stress resultants are obtained in terms of strains as following compact form

$$\begin{cases}
N \\
M^{b} \\
M^{s}
\end{cases} = \begin{bmatrix}
A & B & B^{s} \\
B & D & D^{s} \\
B^{s} & D^{s} & H^{s}
\end{bmatrix} \begin{cases}
\varepsilon \\
k^{b} \\
k^{s}
\end{cases}, \quad S = A^{s}\gamma,$$
(17)

in which

$$N = \{N_x, N_y, N_{xy}\}^t, \qquad M^b = \{M_x^b, M_y^b, M_{xy}^b\}^t, \qquad M^s = \{M_x^s, M_y^s, M_{xy}^s\}^t,$$
(18a)

$$\varepsilon = \left\{ \varepsilon_x^0, \varepsilon_y^0, \gamma_{xy}^0 \right\}^t, \qquad k^b = \left\{ k_x^b, k_y^b, k_{xy}^b \right\}^t, \qquad k^s = \left\{ k_x^s, k_y^s, k_{xy}^s \right\}^t, \tag{18b}$$

$$A = \begin{bmatrix} A_{11} & A_{12} & 0 \\ A_{12} & A_{22} & 0 \\ 0 & 0 & A_{66} \end{bmatrix}, \quad B = \begin{bmatrix} B_{11} & B_{12} & 0 \\ B_{12} & B_{22} & 0 \\ 0 & 0 & B_{66} \end{bmatrix}, \quad D = \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{12} & D_{22} & 0 \\ 0 & 0 & D_{66} \end{bmatrix},$$
(18c)

$$B^{s} = \begin{bmatrix} B_{11}^{s} & B_{12}^{s} & 0 \\ B_{12}^{s} & B_{22}^{s} & 0 \\ 0 & 0 & B_{66}^{s} \end{bmatrix}, \quad D^{s} = \begin{bmatrix} D_{11}^{s} & D_{12}^{s} & 0 \\ D_{12}^{s} & D_{22}^{s} & 0 \\ 0 & 0 & D_{66}^{s} \end{bmatrix}, \quad H^{s} = \begin{bmatrix} H_{11}^{s} & H_{12}^{s} & 0 \\ H_{12}^{s} & H_{22}^{s} & 0 \\ 0 & 0 & H_{66}^{s} \end{bmatrix}, \quad (18d)$$

$$S = \left\{ S_{xz}^{s}, S_{yz}^{s} \right\}^{t}, \quad \gamma = \left\{ \gamma_{xz}^{0}, \gamma_{yz}^{0} \right\}^{t}, \quad A^{s} = \begin{bmatrix} A_{44}^{s} & 0\\ 0 & A_{55}^{s} \end{bmatrix}, \quad (18e)$$

and stiffness components are given as

$$\begin{cases}
A_{11} \quad B_{11} \quad D_{11} \quad B_{11}^{s} \quad D_{11}^{s} \quad H_{11}^{s} \\
A_{12} \quad B_{12} \quad D_{12} \quad B_{12}^{s} \quad D_{12}^{s} \quad H_{12}^{s} \\
A_{66} \quad B_{66} \quad D_{66} \quad B_{66}^{s} \quad D_{66}^{s} \quad H_{66}^{s}
\end{cases} = \int_{-h/2}^{h/2} C_{11}(1, z, z^{2}, f(z), z \quad f(z), f^{2}(z)) \begin{cases}
1 \\
\nu \\
\frac{1-\nu}{2}
\end{cases} dz,$$
(19a)

$$\left(A_{22}, B_{22}, D_{22}, B_{22}^{s}, D_{22}^{s}, H_{22}^{s}\right) = \left(A_{11}, B_{11}, D_{11}, B_{11}^{s}, D_{11}^{s}, H_{11}^{s}\right),$$
(19b)

$$A_{44}^{s} = A_{55}^{s} = \int_{-h/2}^{h/2} C_{44} [g(z)]^{2} dz, \qquad (19c)$$

Introducing Eq. (17) into Eq. (16), the equations of motion can be expressed in terms of displacements ( $\delta u_0$ ,  $\delta v_0$ ,  $\delta w_0$ ,  $\delta \varphi$ ) and the appropriate equations take the form

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$$A_{11}d_{11}u_0 + A_{66}d_{22}u_0 + (A_{12} + A_{66})d_{12}v_0 - B_{11}d_{111}w_0 - (B_{12} + 2B_{66})d_{122}w_0 - (B_{12}^s + 2B_{66}^s)d_{122}\varphi - B_{11}^sd_{111}\varphi = I_0\ddot{u}_0 - I_1d_1\ddot{w}_0 - J_1d_1\ddot{\varphi},$$
(20a)

$$A_{22}d_{22}v_0 + A_{66}d_{11}v_0 + (A_{12} + A_{66})d_{12}u_0 - B_{22}d_{222}w_0 - (B_{12} + 2B_{66})d_{112}w_0 - (B_{12}^s + 2B_{66}^s)d_{112}\varphi - B_{22}^sd_{222}\varphi = I_0\ddot{v}_0 - I_1d_2\ddot{w}_0 - J_1d_2\ddot{\varphi},$$
(20b)

$$B_{11}d_{111}u_0 + (B_{12} + 2B_{66})d_{122}u_0 + (B_{12} + 2B_{66})d_{112}v_0 + B_{22}d_{222}v_0 - D_{11}d_{1111}w_0 - 2(D_{12} + 2D_{66})d_{1122}w_0 - D_{22}d_{2222}w_0 - D_{11}^sd_{1111}\varphi - 2(D_{12}^s + 2D_{66}^s)d_{1122}\varphi - D_{22}^sd_{2222}\varphi - f_e = I_0\ddot{w}_0 + I_1(d_1\ddot{u}_0 + d_2\ddot{v}_0) - I_2(d_{11}\ddot{w}_0 + d_{22}\ddot{w}_0) - J_2(d_{11}\ddot{\varphi} + d_{22}\ddot{\varphi})$$
(20c)

$$B_{11}^{s}d_{111}u_{0} + (B_{12}^{s} + 2B_{66}^{s})d_{122}u_{0} + (B_{12}^{s} + 2B_{66}^{s})d_{112}v_{0} + B_{22}^{s}d_{222}v_{0} - D_{11}^{s}d_{1111}w_{0} -2(D_{12}^{s} + 2D_{66}^{s})d_{1122}w_{0} - D_{22}^{s}d_{2222}w_{0} - H_{11}^{s}d_{1111}\varphi -2(H_{12}^{s} + 2H_{66}^{s})d_{1122}\varphi - H_{22}^{s}d_{2222}\varphi + A_{44}^{s}d_{11}\varphi + A_{55}^{s}d_{22}\varphi = J_{1}(d_{1}\ddot{u}_{0} + d_{2}\ddot{v}_{0}) -J_{2}(d_{11}\ddot{w}_{0} + d_{22}\ddot{w}_{0}) - K_{2}(d_{11}\ddot{\varphi} + d_{22}\ddot{\varphi})$$
(20d)

where  $d_{ij}$ ,  $d_{ijl}$  and  $d_{ijlm}$  are the following differential operators

$$d_{ij} = \frac{\partial^2}{\partial x_i \partial x_j}, \quad d_{ijl} = \frac{\partial^3}{\partial x_i \partial x_j \partial x_l}, \quad d_{ijlm} = \frac{\partial^4}{\partial x_i \partial x_j \partial x_l \partial x_m}, \quad d_i = \frac{\partial}{\partial x_i}, \quad (i, j, l, m = 1, 2).$$
(21)

# 2.3 Closed-form solution for simply-supported FG plates

Based on Navier method, the following expansions of generalized displacements are taken to automatically respect the simply supported boundary conditions

$$\begin{cases}
 u_{0} \\
 v_{0} \\
 w_{0} \\
 \phi
 \end{bmatrix} = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \begin{cases}
 U_{mn} e^{i\omega t} \cos(\alpha x) \sin(\beta y) \\
 V_{mn} e^{i\omega t} \sin(\alpha x) \cos(\beta y) \\
 W_{mn} e^{i\omega t} \sin(\alpha x) \sin(\beta y) \\
 X_{mn} e^{i\omega t} \sin(\alpha x) \sin(\beta y)
 \end{bmatrix}$$
(22)

where  $\alpha = m\pi / a$  and  $\beta = n\pi / b$ ,  $\omega$  is the frequency of free vibration of the plate,  $\sqrt{i} = -1$  the imaginary unit.

Substituting Eqs. (22) into Eq. (20) and collecting the displacements and acceleration for any values of m and n, the following problem is obtained

$$\begin{pmatrix} \begin{bmatrix} S_{11} & S_{12} & S_{13} & S_{14} \\ S_{12} & S_{22} & S_{23} & S_{24} \\ S_{13} & S_{23} & S_{33} & S_{34} \\ S_{14} & S_{24} & S_{34} & S_{44} \end{bmatrix} - \omega^2 \begin{bmatrix} m_{11} & 0 & m_{13} & m_{14} \\ 0 & m_{22} & m_{23} & m_{24} \\ m_{13} & m_{23} & m_{33} & m_{34} \\ m_{14} & m_{24} & m_{34} & m_{44} \end{bmatrix} \begin{pmatrix} U_{mn} \\ V_{mn} \\ W_{mn} \\ X_{mn} \end{pmatrix} = \begin{cases} 0 \\ 0 \\ 0 \\ 0 \end{cases}$$
(23)

where

$$S_{11} = A_{11}\alpha^{2} + A_{66}\beta^{2}, \quad S_{12} = \alpha\beta(A_{12} + A_{66}), \quad S_{13} = -\alpha(B_{11}\alpha^{2} + B_{12}\beta^{2} + 2B_{66}\beta^{2}), \\S_{14} = -\alpha(B_{11}^{s}\alpha^{2} + B_{12}^{s}\beta^{2} + 2B_{66}^{s}\beta^{2}), \quad S_{22} = A_{66}\alpha^{2} + A_{22}\beta^{2}, \\S_{23} = -\beta(B_{11}\beta^{2} + B_{12}\alpha^{2} + 2B_{66}\alpha^{2}), \quad S_{24} = -\beta(B_{11}^{s}\beta^{2} + B_{12}^{s}\alpha^{2} + 2B_{66}^{s}\alpha^{2}), \\S_{33} = D_{11}\alpha^{4} + 2(D_{12} + 2D_{66})\alpha^{2}\beta^{2} + D_{22}\beta^{4} + K_{w} + K_{s}(\alpha^{2} + \beta^{2}), \\S_{34} = D_{11}^{s}\alpha^{4} + 2(D_{12}^{s} + 2D_{66}^{s})\alpha^{2}\beta^{2} + D_{22}^{s}\beta^{4}, \\S_{44} = H_{11}^{s}\alpha^{4} + 2(H_{12}^{s} + 2H_{66}^{s})\alpha^{2}\beta^{2} + H_{22}^{s}\beta^{4} + A_{55}^{s}\alpha^{2} + A_{44}^{s}\beta^{2} \\m_{11} = m_{22} = I_{0}, \quad m_{13} = -\alpha I_{1}, \quad m_{14} = -\alpha J_{1}, \quad m_{23} = -\beta I_{1}, \quad m_{24} = -\beta J_{1}, \\m_{33} = I_{0} + I_{2}(\alpha^{2} + \beta^{2}), \quad m_{34} = J_{2}(\alpha^{2} + \beta^{2}), \quad m_{44} = K_{2}(\alpha^{2} + \beta^{2})$$

$$(24)$$

Eq. (24) is a general form for buckling and free vibration analysis of FG plates resting on elastic foundations under in-plane loads. The stability problem can be carried out by neglecting the mass matrix while the free vibration problem is achieved by omitting the in-plane loads.

## 3. Numerical examples and discussions

In this section the accuracy of the present theory for vibrational analysis of simply supported FG plates is verified. FG plates made of a material combination of metal and ceramic (Al/Al<sub>2</sub>O<sub>3</sub>) are considered. The material properties of FG plates are given in Table 1. For convenience, the following non-dimensional parameters are employed

$$\hat{\omega} = \omega h \sqrt{\rho_m / E_m}, \quad \overline{\omega} = \omega \frac{a^2}{h} \sqrt{\rho_c / E_c}, \quad k_w = \frac{K_w a^4}{D_m}, \quad k_s = \frac{K_s a^2}{D_m}, \quad D_m = \frac{E_m h^3}{12(1 - \nu^2)}$$
(25)

Table 2 aims to prove the accuracy of the present formulation in predicting the free vibration response of Al/Al<sub>2</sub>O<sub>3</sub> plate resting on elastic foundations. By considering, different values of thickness ratio h / a, power law exponent p and foundation parameters  $k_w$ ;  $k_s$ , the non-dimensional  $\hat{\omega}$  of square plates are listed in Table 2 and compared with those given by Baferani *et al.* (2011b) using a third-order shear deformation theory (TSDT) and Thai and Choi (2014) using a zeroth-order shear deformation theory (ZSDT). It can be observed that the computed results are in very

Propriétés	Aluminium (Al)	Alumina (Al <sub>2</sub> O <sub>3</sub> )
Young's modulus (GPa)	70	380
Poisson's ratio	0.3	0.3
Mass density kg/m <sup>3</sup>	2702	3800

Table 1 Material properties employed in the FG plates

k <sub>w</sub> k	1	1 /	Theory —		Power law index ( <i>p</i> )					
	$K_{s}$	n / a		0	0.5	1	2	5		
			Ref <sup>(a)</sup>	0.0291	0.0249	0.0227	0.0209	0.0197		
		0.05	Ref <sup>(b)</sup>	0.0291	0.0246	0.0222	0.0202	0.0191		
			Present	0.0291	0.0247	0.0222	0.0202	0.0191		
			Ref <sup>(a)</sup>	0.1134	0.0975	0.0891	0.0819	0.0767		
		0.10	Ref <sup>(b)</sup>	0.1134	0.0963	0.0868	0.0788	0.0740		
0	0		Present	0.1134	0.0964	0.0870	0.0790	0.0740		
0	0		Ref <sup>(a)</sup>	0.2454	0.2121	0.1939	0.1778	0.1648		
		0.15	Ref <sup>(b)</sup>	0.2452	0.2090	0.1885	0.1706	0.1589		
			Present	0.2450	0.2090	0.1880	0.1710	0.1600		
			Ref <sup>(a)</sup>	0.4154	0.3606	0.3299	0.3016	0.2765		
		0.20	Ref <sup>(b)</sup>	0.4150	0.3551	0.3205	0.2892	0.2667		
			Present	0.4152	0.3551	0.3205	0.2892	0.2665		
			Ref <sup>(a)</sup>	0.0406	0.0389	0.0382	0.0380	0.0381		
		0.05	Ref <sup>(b)</sup>	0.0406	0.0386	0.0378	0.0374	0.0377		
			Present	0.0406	0.0386	0.0378	0.0374	0.0376		
			Ref <sup>(a)</sup>	0.1599	0.1540	0.1517	0.1508	0.1515		
		0.10	Ref <sup>(b)</sup>	0.1597	0.1526	0.1494	0.1478	0.1487		
0	100		Present	0.1597	0.1526	0.1494	0.1478	0.1487		
0	100		Ref <sup>(a)</sup>	0.3515	0.3407	0.3365	0.3351	0.3362		
		0.15	Ref <sup>(b)</sup>	0.3512	0.3369	0.3304	0.3269	0.3286		
			Present	0.3513	0.3369	0.3303	0.3270	0.3285		
			Ref <sup>(a)</sup>	0.6080	0.5932	0.5876	0.5861	0.5879		
		0.20	Ref <sup>(b)</sup>	0.6075	0.5857	0.5753	0.5694	0.5722		
			Present	0.6076	0.5856	0.5752	0.5692	0.5720		
			Ref <sup>(a)</sup>	0.0298	0.0258	0.0238	0.0221	0.0210		
		0.05	Ref <sup>(b)</sup>	0.0298	0.0255	0.0233	0.0214	0.0204		
			Present	0.0298	0.0255	0.0232	0.0214	0.0205		
			Ref <sup>(a)</sup>	0.1162	0.1012	0.0933	0.0867	0.0821		
100		0.10	Ref <sup>(b)</sup>	0.1161	0.0999	0.0910	0.0836	0.0795		
	0		Present	0.1162	0.0999	0.0910	0.0837	0.0796		
100	0		Ref <sup>(a)</sup>	0.2519	0.2204	0.2036	0.1889	0.1775		
		0.15	Ref <sup>(b)</sup>	0.2516	0.2173	0.1982	0.1818	0.1716		
			Present	0.2517	0.2173	0.1982	0.1818	0.1716		
			Ref <sup>(a)</sup>	0.4273	0.3758	0.3476	0.3219	0.2999		
		0.20	Ref <sup>(b)</sup>	0.4269	0.3702	0.3381	0.3097	0.2901		
			ZSDT	0.4272	0.3702	0.3380	0.3096	0.2898		

Table 2 Dimensionless fundamental frequency  $\hat{\omega}$  of square plates

$k_w$	1.	h/a	Theory	Power law index ( <i>p</i> )					
	$\kappa_s$	n / u	Theory	0	0.5	1	2	5	
			Ref <sup>(a)</sup>	0.0411	0.0395	0.0388	0.0386	0.0388	
		0.05	Ref <sup>(b)</sup>	0.0411	0.0392	0.0392	0.0381	0.0384	
			Present	0.0411	0.0392	0.0384	0.0381	0.0384	
			Ref <sup>(a)</sup>	0.1619	0.1563	0.1542	0.1535	0.1543	
		0.10	Ref <sup>(b)</sup>	0.1617	0.1549	0.1519	0.1505	0.1515	
100	100		Present	0.1617	0.1549	0.1519	0.1505	0.1515	
100	100		Ref <sup>(a)</sup>	0.3560	0.3460	0.3422	0.3412	0.3427	
		0.15	Ref <sup>(b)</sup>	0.3557	0.3421	0.3359	0.3329	0.3349	
			Present	0.3558	0.3420	0.3360	0.3327	0.3348	
			Ref <sup>(a)</sup>	0.6162	0.6026	0.5978	0.5970	0.5993	
		0.20	Ref <sup>(b)</sup>	0.6156	0.5950	0.5852	0.5800	0.5834	
			Present	0.6156	0.5948	0.5852	0.5800	0.5832	

Table 2 Continued

<sup>(a)</sup> Baferani *et al.* (2011b) <sup>(b)</sup> Thai and Choi (2014)

Table 3 Dimensionless fundamental frequency  $\overline{\omega}$  of rectangular plates ( $k_w = k_s = 100$ )

a / b	a / h	Theory	Power law index ( <i>p</i> )					
			0	0.5	1	2	5	10
0.5	5	Ref <sup>(a)</sup>	11.3952	11.2331	11.1780	11.2018	11.3593	11.4558
		Present	11.3959	11.2335	11.1783	11.2019	11.3587	11.4557
	10	Ref <sup>(a)</sup>	11.7257	11.4992	11.4270	11.4530	11.6243	11.7093
	10	Present	11.7259	11.4993	11.4271	11.4529	11.6239	11.7092
	20	Ref <sup>(a)</sup>	11.8246	11.5780	11.5005	11.5273	11.7054	11.7886
	20	Present	11.8246	11.5781	11.5005	11.5272	11.7053	11.7885
	5	Ref <sup>(a)</sup>	15.3904	14.8757	14.6305	14.5004	14.5843	14.6636
	3	Present	15.3923	14.8768	14.6313	14.5006	14.5830	14.6635
1	10	Ref <sup>(a)</sup>	16.1728	15.4895	15.1887	15.0455	15.1497	15.2045
-	10	Present	16.1735	15.4898	15.1890	15.0455	15.1488	15.2043
	20	Ref <sup>(a)</sup>	16.4249	15.6851	15.3663	15.2209	15.3414	15.3929
	20	Present	16.4251	15.6852	15.3663	15.2209	15.3411	15.3928
2	5	Ref <sup>(a)</sup>	28.6467	26.8009	25.7640	24.9077	24.5036	24.4352
		Present	28.6591	26.8086	25.7703	24.9109	24.4983	24.4367
	10	Ref <sup>(a)</sup>	32.3893	29.7133	28.3322	27.2931	26.8741	26.6994
	10	Present	32.3937	29.7163	28.3346	27.2932	26.8675	26.6951
	20	Ref <sup>(a)</sup>	33.8869	30.8606	29.3467	28.2628	27.9294	27.7426
	20	Present	33.8882	30.8614	29.3474	28.2627	27.9267	27.7419

(a) Thai and Choi (2014)

good agreement with those calculated by TSDT (Baferani *et al.* 2011b) and by ZSDT (Thai and Choi 2014). It is also concluded from Table 2 that the increase of the foundation parameters  $k_w$ ;  $k_s$ , leads to an increase of non-dimensional fundamental frequency. Compared to the Winkler parameter  $k_w$ , the Pasternak foundation parameter  $k_s$  has dominant influence on increasing the non-dimensional frequency.



Fig. 2 Effect of winkler parameter  $(k_w)$  and the length-to-thickness ratio a/h on the natural frequency  $\overline{\omega}$  of Al/Al<sub>2</sub>O<sub>3</sub> square plates  $(p = 3, k_s = 10)$ 



Fig. 3 Effect of pasternak parameter ( $k_s$ ) and the length-to-thickness ratio a/h on the natural frequency  $\overline{\omega}$  of Al/Al<sub>2</sub>O<sub>3</sub> square plates, ( $p = 3, k_w = 10$ )



Fig. 4 Effect of winkler parameter  $(k_w)$  and the power law index p on the natural frequency  $\overline{\omega}$  of Al/Al<sub>2</sub>O<sub>3</sub> square plates, (a / h = 10)



Fig. 5 Effect of pasternak parameter ( $k_s$ ) and the power law index p on the natural frequency  $\overline{\omega}$  of Al/Al<sub>2</sub>O<sub>3</sub> square plates, (a / h = 10 and  $k_w = 10$ )

Table 3 presents non-dimensional fundamental frequencies  $\overline{\omega}$  of FG rectangular plates resting on elastic foundation. In this example, the non-dimensional fundamental frequencies computed by present method are compared with those predicted by Thai and Choi (2014) based on ZSDT. The non-dimensional foundation parameters ( $k_w$ ,  $k_s$ ) are considered to be 100. The reliability of the presented formulation for FG plates can be concluded from Table 3; where the results are in an excellent agreement as values of non-dimensional fundamental frequency are consistent with those



Fig. 6 Effect of the power- law index p on the natural frequency  $\overline{\omega}$  of Al/Al<sub>2</sub>O<sub>3</sub> square plates,  $(a / h = 10 \text{ and } k_w = k_s = 0)$ 

predicted by Thai and Choi (2014).

The variation of non-dimensional frequencies  $\overline{\omega}$  with various values of length-to-thickness ratios are plotted in Figs. 2 and 3 for different values of Winkler parameter  $k_w$  and Pasternak foundation parameter  $k_s$ , respectively. It can be seen from this figure, that for thin plates where a/htakes high values, the frequencies become almost constants. It can be also observed that, increasing value of Winkler and Pasternak parameters cause the increase in the natural frequency. The figures demonstrate also, that Pasternak parameter of foundation has more significant influence than Winkler parameter on the fundamental frequency of plate.

The variations of non-dimensional fundamental frequencies of square FG plates with respect to power law index p and for different values of Winkler and Pasternak parameters are plotted in Figs. 4 and 5, respectively. It is observed from the figures that, increasing value of power law index causes a reduction of the fundamental frequency. It is due to the fact that a higher value of p corresponds to lower value of volume fraction of the ceramic phase, and thus makes the plates become the softer ones.

The variation of non-dimensional frequencies in terms of the power-law index is presented in Fig. 6 for different mode number. It can be observed from this figure that the frequencies diminish with the decrease of the mode number.

## 4. Conclusions

In the current investigation, analytical formulation for free vibration response of FG plates resting on elastic foundation is developed on the supposition that transverse shear displacements vary as a hyperbolic function within the thickness of plate. The proposed model contains only four unknowns and equations of motion are obtained from Hamilton's principle. Navier-type solutions are determined for simply-supported boundary conditions and compared with the existing solutions to check the validity of the proposed theory. The material properties are estimated by power-law form. It has been demonstrated that the present analytical formulation can accurately predict natural frequencies of FG plates resting on elastic foundation.

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