

Influences of porosity on dynamic response of FG plates resting on Winkler/Pasternak/Kerr foundation using quasi 3D HSDT

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Abstract. This work investigates the effect of Winkler/Pasternak/Kerr foundation and porosity on dynamic behavior of FG plates using a simple quasi-3D hyperbolic theory. Four different patterns of porosity variations are considered in this study. The used quasi-3D hyperbolic theory is simple and easy to apply because it considers only four-unknown variables to determine the four coupled vibration responses (axial-shear-flexion-stretching). A detailed parametric study is established to evaluate the influences of gradient index, porosity parameter, stiffness of foundation parameters, mode numbers, and geometry on the natural frequencies of imperfect FG plates.

Keywords: Kerr foundation; porous FGM; quasi-3D plate model; vibration

1. Introduction

Functionally graded materials (FGM) were presented in the mid-1980s to be employed as thermal barrier materials against high temperatures. FGMs are advanced materials made of different constituent materials, such as metals, ceramics or polymers, with variable properties in a given spatial direction. This makes it possible to customize the morphologies and the structured characteristics in this specific spatial direction, which improves the mechanical behavior of these materials in terms of stiffness, toughness, hardness, thermal conductivity and corrosion resistance (Sofiyev and Avcar 2010, Bessaim 2013, Naebe and Shirvanimoghaddam 2016, Ebrahimi *et al.* 2017, Zidi *et al.* 2017, El-Haina *et al.* 2017, Avcar and Mohammed 2018, Zarga *et al.* 2019, Karami *et al.* 2019a, b, Hellal *et al.* 2019). In recent years, the trend to use FG plates for use in modern structures has grown considerably. There are many uses of FG structures in the fields of energy conversion, nuclear power engineering, commodities, civil engineering and aerospace. As a result, advances in numerical analysis of FG structures have attracted a lot of attention. One of the most cost-effective ways to advance in numerical investigation is the development of accurate structural models via refined shear deformation theories.

In recent years, extensive studies on FG plates have

been performed using conventional plate theory (CPT) and first order shear deformation plate theory (FSDT). Despite its simplicity, the CPT ignores shear deformations and rotational inertia, resulting in less accurate results for thick and moderately thick structures. The FSDT considers transverse shear influences via a shear correction factor and is therefore suitable for the investigation of both thin and moderately thick structures (Al-Basyouni *et al.* 2015, Boudierba *et al.* 2016, Avcar 2016, Youcef *et al.* 2018, Draoui *et al.* 2019). However, the appropriate value of the shear correction coefficient depends on the variation of the Poisson's ratio depending on plate thickness, geometry, load and boundary conditions. Higher order shear deformation theories (HSDTs) do not require a shear correction coefficient and offer the reliable accuracy against to CPT and FSDT. However, these theories lead to a large number of equilibrium equations, greatly increasing the complexity of the problem. As a result, simple theories with fewer unknowns are very attractive. In order to decrease the number of variables employed in the equations of motion, and to satisfy shear deformation influences on the lower and upper faces of the structures without using shear correction factor, Shimpi and Patel (2006a) proposed a refined theory with only two unknown variables for the study of isotropic plates, known as refined plate theory (RPT). Then, different validity studies were carried out on the basis of the RPT. These include the study of isotropic (Shimpi and Patel 2006a, Shahsavari and Janghorban 2017), orthotropic (Shimpi and Patel 2006b), FGM (Karami *et al.* 2018a, Zidi *et al.* 2014) and stratified composite plates (Thai and Kim

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2012). However, until now, various RPT models under the effect of different shape functions by dividing the transverse displacement into bending and shearing parts have been proposed for the study of dynamic (Chaabane *et al.* 2019, Zaoui *et al.* 2019, Bourada *et al.* 2019, Abdelaziz *et al.* 2017, Mouffoki *et al.* 2017, Houari *et al.* 2016, Bellifa *et al.* 2016, Attia *et al.* 2015, Karama *et al.* 1998), bending (Hamidi *et al.* 2015, Beldjelili *et al.* 2016, Shahsavari and Janghorban 2017, Abdelaziz *et al.* 2017, Hachemi *et al.* 2017, Kar *et al.* 2017, Bakhadda *et al.* 2018, Attia *et al.* 2018, Younsi *et al.* 2018, Boussoula *et al.* 2019, Meksi *et al.* 2019), wave propagation (Boukhari *et al.* 2016, Benadouda *et al.* 2017, Selmi and Bisharat 2018, Karami *et al.* 2018b, Fourn *et al.* 2018) and buckling (Karama *et al.* 1998, Meziane *et al.* 2014, Bousahla *et al.* 2016, Boudherba *et al.* 2016, Sekkal *et al.* 2017ab, Chikh *et al.* 2017, Menasria *et al.* 2017, Bellifa *et al.* 2017a, b, Tounsi *et al.* 2019) responses of micro and nano-plate structure. Yahia *et al.* (2015) examined the wave behavior of FG plates using RPT with 4-variables in terms of cubic, sinusoidal, hyperbolic, and exponential shear strain shape functions. A refined theory of trigonometric shear deformation (RTSDT) was employed for the thermoelastic bending response of sandwich plates made of FGM by Tounsi *et al.* (2013). Recently, the 4-unknown RPT model using polynomial, exponential and hyperbolic functions is applied to research on shear buckling behavior of nano-plates in a hygro-thermal environment based on the non-local stress gradient theory of Shahsavari *et al.* (2018a).

Often, conventional continuum theories (CPT, FSDT, and HSDTs) neglect the stretching effect of thickness (i.e., $\epsilon_z=0$) because of the assumption of constant transverse displacements in the thickness. Recently, the effect of thickness stretching (ϵ_z) in FG plates using finite element approximations was investigated by Carrera *et al.* (2011) to reach accurate results. Recently, the influence of thickness stretching in FG plates using finite element approximations has been studied by Carrera *et al.* (2011) to obtain accurate results. The thickness stretching influence becomes very valuable for the analysis of thick plates and must therefore be taken into account. On the joint consideration of shear deformation and thickness stretching effects, the many quasi 3D theories, based on higher-order distributions within the thickness for deflections, have been proposed (Thai and Kim 2015). Thai and Kim (2013) have proposed a simple quasi-3D sinusoidal shear-deformation model to study the flexural behavior of FG plates using five unknown variables. Hebbali *et al.* (2014) developed a novel quasi-3D model for the analysis of the bending and dynamic of FG plates. An efficient quasi-3D theory has been proposed for FG plates, by dividing the deflection into flexural, shear and stretching components by Belabed *et al.* (2014). Bousahla *et al.* (2014) presented a new quasi-3D theory based on neutral surface position for static study of advanced composite plates. Bourada *et al.* (2015) developed a simple higher-order shear and normal deformation model for FGM beams. Draiche *et al.* (2016) proposed a quasi-3D shear-deformation model for "laminated composite plates". Thai *et al.* (2014) developed a quasi-3D theory for FG plates by considering a hyperbolic shape function as well as five

variables. A sandwich with FGM core and FGM face sheet as well as a sandwich with FGM core and homogeneous face sheet was examined by a new quasi-3D plate model by Bennoun *et al.* (2016).

During the process of manufacturing FGMs, micro-voids are generated during sintering because of the difference in solidification temperature of the constituents of the material (Zhu *et al.* 2001, Li *et al.* 2003). Micro-void formation sources (known as porosity) include air bubbles entering the matrix during melting or mixing processes and formation of water vapor on the surface of the particles in the process of solidification (Aqida *et al.* 2004). Because of the importance of this topic, several works have been conducted to explore the effects of porosity. For example, Yahia *et al.* (2015) investigated the wave propagation in FG plates with porosities using various HSDTs. A HSDT was employed for the study on dynamic of beams made of porous graded materials by Ait Atmane *et al.* (2015). Gupta and Talha (2017) examined the influence of porosity on free vibration behavior of FG plates in the presence of a thermal influence using a non-polynomial quasi 3D HSDT. Benferhat *et al.* (2016a) analyzed the bending response of FG plate with porosities. Also, Benferhat *et al.* (2016b) studied the effect of porosity on the bending and free vibration response of FG plates resting on Winkler-Pasternak foundations. Benadouda *et al.* (2017) studied the effect of porosities on wave propagation in FG beams using an efficient shear deformation theory. Rad *et al.* (2017) analyzed the static response of non-uniform heterogeneous circular plate with porous material resting on a gradient hybrid foundation involving friction force. Akbaş (2017) presented vibration and static analysis of functionally graded porous plates. Eltaher *et al.* (2018) presented a modified porosity model in analysis of FG porous nano-beams. Shahsavari *et al.* (2018b) studied the effect of porosities on free vibration of FG plates resting on elastic foundation. Faleh *et al.* (2018) discussed the vibration properties of porous FG nanoshells. Akbaş (2018) studied the forced vibration behavior of FG porous deep beams. Karami *et al.* (2018c) investigated the thermal buckling of smart porous FG nanobeam rested on Kerr foundation. Avcar (2019) examined the dynamic response of imperfect sigmoid and power law functionally graded beams. Arshid *et al.* (2019) studied the effect of porosity on free vibration of SPFG circular plates resting on visco-Pasternak elastic foundation. Batou *et al.* (2019) studied the wave dispersion properties in imperfect sigmoid plates using various HSDTs.

In recent years, the study of integrated structures in foundations has attracted a lot of attention. To define the interaction between plate and foundation, various assumptions of foundation models have been proposed (Wanget *al.* 2005). The simplest and oldest assumption of the elastic medium models, which has only one substrate reaction coefficient, is known as the Winkler elastic foundation (Winkler 1867). Despite the ease of implementation, the Winkler model is unable to provide continuity in the foundation because of separate springs (Kolahchi *et al.* 2016). This assumption has been improved by the Pasternak model (Pasternak 1954) by adding a shear

layer above the springs. The Pasternak model comprising a two-parameter substrate (spring and shear layer) is widely employed to explain the mechanical interactions of flexible plates with different distributions of material properties (Akhavan *et al.* 2009, Hsu 2010, Baferani *et al.* 2011, Lü *et al.* 2009, Boudierba *et al.* 2013, Bounouara *et al.* 2016, Sobhy 2013). In the Kerr Foundation (Kneifati 1985), there are no unconcentrated reactions because of an upper spring layer. This means that, in Kerr's model, a shear layer is surrounded by upper and lower spring layers.

In this work, we examine effects of Winkler/Pasternak/Kerr foundation and porosity on dynamic behavior of imperfect FG plates. For this end, a simple quasi-3D hyperbolic shear deformation plate theory is employed with considering the axial, bending, shear, and thickness stretching effects. Four different patterns of porosity variations are considered for describing porosity influence in graded material characteristics. The Kerr foundation is utilized to describe elastic foundation. The proposed model for the FG plates incorporated into the Kerr foundation will provide the best explanation of the embedded plates compared to that of the Winkler and Pasternak foundations.

2. Theoretical formulations

2.1 geometry and concept of functionally graded plate (P-FGM)

In the present study, consider a functionally graded metal-ceramic plates (P-FGM) of length “ a ”, width “ b ” and thickness “ h ” in the reference ($x \times y \times z$), respectively. The material properties of FG-plate such as Young's modulus “ $E(z)$ ”, mass density “ $\rho(z)$ ” and Poisson's ratio “ $\nu(z)$ ” are assumed to vary continuously through the thickness according to the power law distribution as (Tounsi *et al.* 2013, Houari *et al.* 2013, Bousahla *et al.* 2014, Bourada *et al.* 2015, Fahsi *et al.* 2017)

$$P(z) = P_m + (P_c - P_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p \quad (1)$$

Where P_m and P_c are the metal and ceramic materials properties, respectively. p is the material index.

The metal-ceramic FG-plate is supposed resting on elastic foundations (type Winkler, Pasternak and Kerr). The illustrative sketches of the three types of elastic foundations are presented in Fig. 1.

2.2 Porous functionally graded plates

The imperfection in the functionally graded materials can be in the form of the micro voids (porosity) that occur during the manufacturing steps of these materials. The micro voids are due to the difference of solidification temperatures between the two materials that constitute the FGM (Zhu *et al.* 2001). Several formulation models of the distribution of the micro voids in functionally graded structures have been proposed such as even, uneven, and logarithmic-uneven porosities.

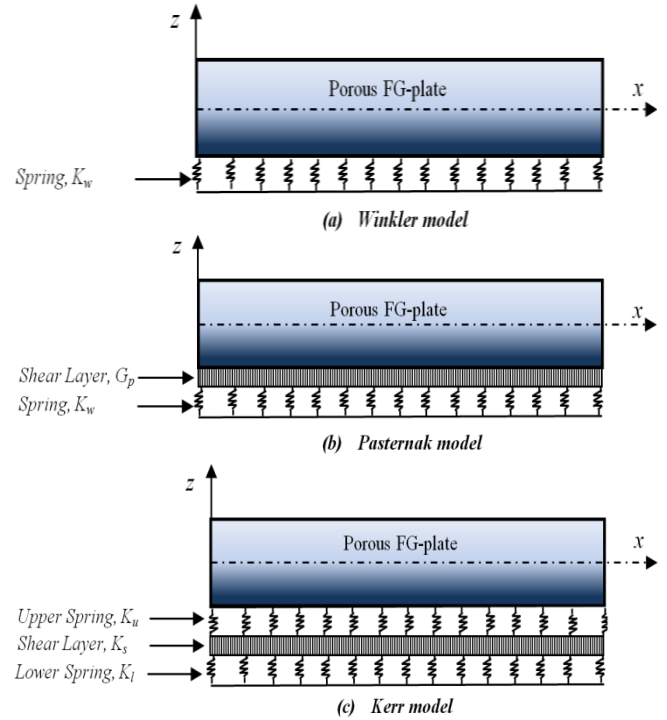


Fig. 1 FG-plates resting on elastic foundations

2.2.1 FG-plate with even porosities

The first model of the porosity distribution was developed by Wattanasakulpong and Ungbhakorn (2014) where the porosity is constant across the thicknesses of the FG-plate (see Fig. 2(a)). The effective materials properties of the FG-plate obtained by introducing the even porosities can be given as

$$P(z) = P_c \left(\left(\frac{1}{2} + \frac{z}{h} \right)^p - \frac{\xi}{2} \right) + P_m \left(1 - \left(\frac{1}{2} + \frac{z}{h} \right)^p - \frac{\xi}{2} \right) \quad (2)$$

Where ξ is the parameter which takes into account the porosity effect.

By applying the Eq. (2) on the effective properties of the FG-plate. The Young's modulus “ $E(z)$ ”, mass density “ $\rho(z)$ ” and Poisson's ratio “ $\nu(z)$ ” formulations can be expressed as (Wattanasakulpong and Ungbhakorn 2014)

$$E(z) = (E_c - E_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + E_m - \frac{\xi}{2} (E_c + E_m) \quad (3a)$$

$$\nu(z) = (\nu_c - \nu_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + \nu_m - \frac{\xi}{2} (\nu_c + \nu_m) \quad (3b)$$

$$\rho(z) = (\rho_c - \rho_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + \rho_m - \frac{\xi}{2} (\rho_c + \rho_m) \quad (3c)$$

2.2.2 FG-plate with uneven porosities

The infiltration of the materials in the intermediate zone of the plate is very difficult (which increases the risk of production of micro-voids). On the other hand, the infiltration of the material is easy in the free surfaces (upper and lower surfaces) of the plate (the risk of the production

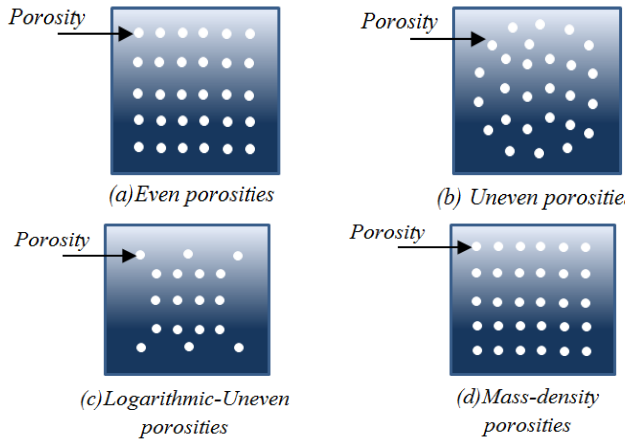


Fig. 2 illustration of different patterns of porosity variations

of the micro voids is low), on the basis of these cases Wattanasakulpong and Ungbhakorn (2014) has developed another model of the porosities distribution (porosity varies across the thickness). The effective material properties with uneven distribution (see Fig. 2(b)) can be written as

$$E(z) = (E_c - E_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + E_m - \frac{\xi}{2} (E_c + E_m) \left(1 - \frac{2|z|}{h} \right) \quad (4a)$$

$$\nu(z) = (\nu_c - \nu_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + \nu_m - \frac{\xi}{2} (\nu_c + \nu_m) \left(1 - \frac{2|z|}{h} \right) \quad (4b)$$

$$\rho(z) = (\rho_c - \rho_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + \rho_m - \frac{\xi}{2} (\rho_c + \rho_m) \left(1 - \frac{2|z|}{h} \right) \quad (4c)$$

2.2.3 FG-plate with logarithmic-uneven porosities

The third model was proposed by Gupta and Talha (2018) where the distribution of the porosity is varying according to a logarithmic function through the thickness of the plate (see Fig. 2(c)). The effective material properties with logarithmic-uneven distribution can be given as

$$E(z) = (E_c - E_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + E_m - \log \left(1 + \frac{\xi}{2} \right) (E_c + E_m) \left(1 - \frac{2|z|}{h} \right) \quad (5a)$$

$$\nu(z) = (\nu_c - \nu_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + \nu_m - \log \left(1 + \frac{\xi}{2} \right) (\nu_c + \nu_m) \left(1 - \frac{2|z|}{h} \right) \quad (5b)$$

$$\rho(z) = (\rho_c - \rho_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + \rho_m - \log \left(1 + \frac{\xi}{2} \right) (\rho_c + \rho_m) \left(1 - \frac{2|z|}{h} \right) \quad (5c)$$

2.2.4 FG-plate with mass-density porosities

In the present investigation, the fourth model of the porosity is based on the true and the apparent mass density. The formulations of the true and the apparent mass density can be written as

$$m_0 = \int_h \rho(z) dz \text{ at } \xi = 0 \text{ and } m = \int_h \rho(z) dz \text{ at } \xi > 0 \quad (6)$$

With

$$\rho(z) = (\rho_c - \rho_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + \rho_m - \frac{\xi}{2} (\rho_c + \rho_m) \quad (7)$$

where “ m_0 ” and “ m ” are the true and the apparent mass density.

By considering that the elasticity modulus depend on the density of the material, the expression of the Young modulus proposed by Eltaher *et al.* (2018) can be given as

$$E(z) = (E_c - E_m) \left(\frac{z}{h} + \frac{1}{2} \right)^p + E_m - \frac{m_0 - m}{m_0} (E_c + E_m) \quad (8)$$

2.3 Displacement field and strains:

By introducing the stretching effect on the higher order shear deformation theory (Hebali *et al.* 2016, Merdaci *et al.* 2016, Bourada *et al.* 201, 2018, Elmossouess *et al.* 2017, AitSidhoum *et al.* 2017, Besseghier *et al.* 2017) and keeps the same number of unknowns (Four variables) that is reduced compared to conventional quasi-3D theory. The displacement field of the six unknown quasi-3D theory can be given as

$$u(x, y, z) = u_0(x, y) - z \frac{\partial w_0}{\partial x} + f(z) \phi_x(x, y) \quad (9a)$$

$$v(x, y, z) = v_0(x, y) - z \frac{\partial w_0}{\partial y} + f(z) \phi_y(x, y) \quad (9b)$$

$$w(x, y, z) = w_0(x, y) + g(z) \phi_z(x, y) \quad (9c)$$

The present four unknown hyperbolic quasi-3D shear deformation theory is assumed in the following form (Abualnour *et al.* 2018, Benchohra *et al.* 2018, Bouhadra *et al.* 2018, Boukhelif *et al.* 2019, Boulefrakh *et al.* 2019, Bouanati *et al.* 2019, Khiloun *et al.* 2019, Bendaho *et al.* 2019)

$$u(x, y, z, t) = u_0(x, y, t) - z \frac{\partial w_0}{\partial x} + k_1 f(z) \int \theta(x, y, t) dx \quad (10a)$$

$$v(x, y, z, t) = v_0(x, y, t) - z \frac{\partial w_0}{\partial y} + k_2 f(z) \int \theta(x, y, t) dy \quad (10b)$$

$$w(x, y, z, t) = w_0(x, y, t) + g(z) \theta(x, y, t) \quad (10c)$$

With

$$f(z) = - \left[\frac{3\pi}{2} z \operatorname{sech}^2 \left(\frac{1}{2} \right) \right] + \frac{3\pi}{2} h \tanh \left(\frac{z}{h} \right) \quad (11)$$

$$\text{and } g(z) = \frac{2}{15} \frac{\partial f(z)}{\partial z}$$

Where the terms (u_0, v_0, w_0 and θ) are four unknown displacements of the mid-plane of the plate and the coefficient “ k_1 ” and “ k_2 ” depends on the geometry.

Based on the kinematic of Eq. (10), the strain-displacement expressions can be obtained as

$$\begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix} = \begin{Bmatrix} \varepsilon_x^0 \\ \varepsilon_y^0 \\ \gamma_{xy}^0 \end{Bmatrix} + z \begin{Bmatrix} k_x^b \\ k_y^b \\ k_{xy}^b \end{Bmatrix} + f(z) \begin{Bmatrix} k_x^s \\ k_y^s \\ k_{xy}^s \end{Bmatrix}, \quad (12)$$

$$\begin{Bmatrix} \gamma_{yz} \\ \gamma_{xz} \end{Bmatrix} = f'(z) \begin{Bmatrix} \gamma_{yz}^0 \\ \gamma_{xz}^0 \end{Bmatrix} + g(z) \begin{Bmatrix} \gamma_{yz}^1 \\ \gamma_{xz}^1 \end{Bmatrix}, \quad \varepsilon_z = g'(z) \varepsilon_z^0$$

with

$$\begin{Bmatrix} \varepsilon_x^0 \\ \varepsilon_y^0 \\ \gamma_{xy}^0 \end{Bmatrix} = \begin{Bmatrix} \frac{\partial u_0}{\partial x} \\ \frac{\partial v_0}{\partial x} \\ \frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \end{Bmatrix}, \quad \begin{Bmatrix} k_x^b \\ k_y^b \\ k_{xy}^b \end{Bmatrix} = \begin{Bmatrix} -\frac{\partial^2 w_0}{\partial x^2} \\ -\frac{\partial^2 w_0}{\partial y^2} \\ -2\frac{\partial^2 w_0}{\partial x \partial y} \end{Bmatrix}, \quad (13a)$$

$$\begin{Bmatrix} k_x^s \\ k_y^s \\ k_{xy}^s \end{Bmatrix} = \begin{Bmatrix} k_1 \theta \\ k_2 \theta \\ k_1 \frac{\partial}{\partial y} \int \theta dx + k_2 \frac{\partial}{\partial x} \int \theta dy \end{Bmatrix}$$

$$\begin{Bmatrix} \gamma_{yz}^0 \\ \gamma_{xz}^0 \end{Bmatrix} = \begin{Bmatrix} k_2 \int \theta dy \\ k_1 \int \theta dx \end{Bmatrix}, \quad \begin{Bmatrix} \gamma_{yz}^1 \\ \gamma_{xz}^1 \end{Bmatrix} = \begin{Bmatrix} \frac{\partial \theta}{\partial y} \\ \frac{\partial \theta}{\partial x} \end{Bmatrix}, \quad (13b)$$

$$\varepsilon_z^0 = \theta \quad \text{and} \quad g'(z) = \frac{dg(z)}{dz}$$

The undetermined integrals $\int \theta dx$; $\int \theta dy$; $\frac{\partial}{\partial y} \int \theta dx$;

$\frac{\partial}{\partial x} \int \theta dy$ mentioned in the previous equations shall be resolved via Navier solution and can be obtained as follows

$$\int \theta dx = A' \frac{\partial \theta}{\partial x}, \quad \int \theta dy = B' \frac{\partial \theta}{\partial y}, \quad (14)$$

$$\frac{\partial}{\partial y} \int \theta dx = A' \frac{\partial^2 \theta}{\partial x \partial y}, \quad \frac{\partial}{\partial x} \int \theta dy = B' \frac{\partial^2 \theta}{\partial x \partial y}$$

where the coefficients “A” and “B” are defined according to the type of method used. In this case using Navier solution, the terms A' , B' , k_1 and k_2 are obtained as follows

$$A' = -\frac{1}{\alpha^2}, \quad B' = -\frac{1}{\beta^2}, \quad k_1 = -\alpha^2, \quad k_2 = -\beta^2 \quad (15)$$

where α and β are defined in expression 41.

For functionally graded material, the linear constitution relations (stress-strain) can be written as

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{Bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & 0 & 0 & 0 \\ C_{12} & C_{22} & C_{23} & 0 & 0 & 0 \\ C_{13} & C_{23} & C_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & C_{66} & 0 & 0 \\ 0 & 0 & 0 & 0 & C_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & C_{44} \end{bmatrix} \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \varepsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix} \quad (16)$$

where “ σ and τ ” are the normal and shear stresses and “ ε and γ ” are the strain components. The elastic constants expressions C_{ij} in terms of engineering are given below (Hebali *et al.* 2014, Benahmed *et al.* 2017, Shahsavari *et al.* 2018b, Ait Sidhoum *et al.* 2018):

• If the stretching effect is negligible “ $\varepsilon_z=0$ ”, the 2D elastic constants “ C_{ij} ” can be defined as

$$C_{11} = C_{22} = \frac{E(z)}{(1-\nu(z)^2)}, \quad (17)$$

$$C_{12} = \frac{\nu E(z)}{(1-\nu(z)^2)}, \quad (18)$$

$$C_{44} = C_{55} = C_{66} = \frac{E(z)}{2(1+\nu(z))}, \quad (19)$$

• If the stretching effect is considered “ $\varepsilon_z \neq 0$ ”, the 3D elastic constants “ C_{ij} ” can be expressed as

$$C_{11} = C_{22} = C_{33} = \frac{1-\nu(z)}{\nu(z)} \lambda(z), \quad (20)$$

$$C_{12} = C_{13} = C_{23} = \lambda(z), \quad (21)$$

$$C_{44} = C_{55} = C_{66} = \mu(z), \quad (22)$$

With

$$\lambda(z) = \frac{\nu(z)E(z)}{(1-2\nu(z))(1+\nu(z))} \quad (23)$$

$$\text{And } \mu(z) = G(z) = \frac{E(z)}{2(1+\nu(z))}$$

where $\mu(z)$ Lamé’s coefficients.

2.4 Equations of motion

The equations of motion of the free vibration analysis of simply supported FG-plate resting on elastic foundation can be derived by employing the Hamilton’s energy principle (HEP). The analytical form of the principle (HEP) can be expressed as follow (Mahi *et al.* 2015, Bennai *et al.* 2015, Zemri *et al.* 2015)

$$\int_0^t (\delta U + \delta U_F - \delta K) dt = 0 \quad (24)$$

where δU , δU_F and δK are the virtual strain energy, the strain energy induced by elastic foundations and the variation of kinetic energy, respectively.

The variation of the virtual strain energy (δU) of the FG-plate can be rewritten as

$$\delta U = \int_V \left[\sigma_x \delta \varepsilon_x + \sigma_y \delta \varepsilon_y + \sigma_z \delta \varepsilon_z + \tau_{xy} \delta \gamma_{xy} + \tau_{yz} \delta \gamma_{yz} + \tau_{xz} \delta \gamma_{xz} \right] dV$$

$$= \int_\Omega \left[N_x \delta \varepsilon_x^0 + N_y \delta \varepsilon_y^0 + N_z \delta \varepsilon_z^0 + N_{xy} \delta \gamma_{xy}^0 + M_x^b \delta k_x^b + M_y^b \delta k_y^b + M_{xy}^b \delta k_{xy}^b + M_x^s \delta k_x^s + M_y^s \delta k_y^s + M_{xy}^s \delta k_{xy}^s + Q_{yz}^s \delta \gamma_{yz}^0 + S_{yz}^s \delta \gamma_{yz}^1 + Q_{xz}^s \delta \gamma_{xz}^0 + S_{xz}^s \delta \gamma_{xz}^1 \right] dA \quad (25)$$

Where A is the top surface and “ N, M, S and Q ” are the stress resultants, with

$$(N_i, M_i^b, M_i^s) = \int_{-h/2}^{h/2} (1, z, f) \sigma_i dz, \quad (i = x, y, xy); \quad (26a)$$

$$N_z = \int_{-h/2}^{h/2} g'(z) \sigma_z dz$$

$$(S_{xz}^s, S_{yz}^s) = \int_{-h/2}^{h/2} g(z) (\tau_{xz}, \tau_{yz}) dz, \quad (26b)$$

$$(Q_{xz}^s, Q_{yz}^s) = \int_{-h/2}^{h/2} f'(z) (\tau_{xz}, \tau_{yz}) dz,$$

The elastic foundation models employed in the present investigation are illustrated in the Fig. 1. The strain energy induced by the elastic foundations (Winkler, Pasternak and Kerr) can be defined as

$$\delta U_F = - \int_V \{ U_{Winkler} + U_{Pasternak} + U_{Kerr} \} dV \quad (27)$$

$$= -(q_{Winkler} + q_{Pasternak} + q_{Kerr})$$

In the case of the present four unknown's quasi-3D plate theory, the distributed load cited in the Eq. (27) can be defined by:

Winkler model

This model contains a single parameter (which represents independent springs) and can be expressed as

$$q_{Winkler} = K_w w_0 \quad (28)$$

Where K_w is the constant transverse stiffness coefficient of the elastic medium (so-called spring constant).

Pasternak model

This model contains two elastic parameters, the first is the same of the Winkler (springs) and the second is a shear layer parameter (shear action) which depicts the interaction between the spring parts (Shahsavari et al. 2018b), and the Pasternak reaction can be defined as

$$q_{Pasternak} = K_w w_0 - G_p \nabla^2 w_0 \quad (29)$$

Where G_p is the shear stiffness and ∇^2 represent the rectangular Cartesian coordinates, the Laplace differential operator is defined as

$$\nabla^2 = \partial^2 / \partial x^2 + \partial^2 / \partial y^2 \quad (30)$$

To obtain the Winkler foundation model from the Pasternak model just put $G_p=0$ in Eq. (29).

Kerr model

This model is composed of three elastic layers, independent upper and lower layers modeled by springs (with stiffness's K_u and K_l , respectively) and an intermediate shear layer with stiffness K_s (Shahsavari et al. 2018b). The distributed reaction of this last model (Kerr foundation) is defined as

$$\left(q_{Kerr} - \left(\frac{K_s K_u}{K_l + K_u} \right) \nabla^2 q_{Kerr} \right) = \left(\frac{K_l K_u}{K_l + K_u} \right) w_0 - \left(\frac{K_s K_u}{K_l + K_u} \right) \nabla^2 w_0 \quad (31)$$

This type of Kerr foundation is taken into account for the first time for the present displacement field of quasi-3D plate theories with only four unknowns.

The expression of the variation of kinetic energy of the FG-plate can be written as (Belkorissat et al. 2015, Larbi Chaht et al. 2015, Belabed et al. 2018, Bouafia et al. 2018)

$$\delta K = \int_V [\dot{u} \delta \dot{u} + \dot{v} \delta \dot{v} + \dot{w} \delta \dot{w}] \rho(z) dV$$

$$= \int_A \left\{ I_0 [\dot{u}_0 \delta \dot{u}_0 + \dot{v}_0 \delta \dot{v}_0 + \dot{w}_0 \delta \dot{w}_0] \right.$$

$$- I_1 \left(\dot{u}_0 \frac{\partial \delta \dot{w}_0}{\partial x} + \frac{\partial \dot{w}_0}{\partial x} \delta \dot{u}_0 + \dot{v}_0 \frac{\partial \delta \dot{w}_0}{\partial y} + \frac{\partial \dot{w}_0}{\partial y} \delta \dot{v}_0 \right)$$

$$+ J_1 \left((k_1 A') \left(\dot{u}_0 \frac{\partial \delta \dot{\theta}}{\partial x} + \frac{\partial \dot{\theta}}{\partial x} \delta \dot{u}_0 \right) + (k_2 B') \left(\dot{v}_0 \frac{\partial \delta \dot{\theta}}{\partial y} + \frac{\partial \dot{\theta}}{\partial y} \delta \dot{v}_0 \right) + J_1^{st} (\dot{w}_0 \delta \dot{\theta} + \dot{\theta} \delta \dot{w}_0) \right)$$

$$+ I_2 \left(\frac{\partial \dot{w}_0}{\partial x} \frac{\partial \delta \dot{w}_0}{\partial x} + \frac{\partial \dot{w}_0}{\partial y} \frac{\partial \delta \dot{w}_0}{\partial y} \right) + K_2 \left((k_1 A')^2 \left(\frac{\partial \dot{\theta}}{\partial x} \frac{\partial \delta \dot{\theta}}{\partial x} \right) + (k_2 B')^2 \left(\frac{\partial \dot{\theta}}{\partial y} \frac{\partial \delta \dot{\theta}}{\partial y} \right) \right)$$

$$- J_2 \left((k_1 A') \left(\frac{\partial \dot{w}_0}{\partial x} \frac{\partial \delta \dot{\theta}}{\partial x} + \frac{\partial \dot{\theta}}{\partial x} \frac{\partial \delta \dot{w}_0}{\partial x} \right) + (k_2 B') \left(\frac{\partial \dot{w}_0}{\partial y} \frac{\partial \delta \dot{\theta}}{\partial y} + \frac{\partial \dot{\theta}}{\partial y} \frac{\partial \delta \dot{w}_0}{\partial y} \right) + K_2^{st} \dot{\theta} \delta \dot{\theta} \right) \} dA \quad (32)$$

where (I_i , J_i and K_i) are mass inertias of the FG-plate and dot-superscript convention indicate the differentiation with respect to the time variable t .

$$(I_0, I_1, I_2) = \int_{-h/2}^{h/2} (1, z, z^2) \rho(z) dz \quad (33a)$$

$$(J_1, J_1^{st}, J_2, K_2, K_2^{st}) = \int_{-h/2}^{h/2} (f, g, zf, f^2, g^2) \rho(z) dz \quad (33b)$$

By substituting the virtual strain energy (Eq. (25)), The strain energy induced by elastic foundations (Eq. (27)) and the variation of kinetic energy (Eq. (32)) into expression of

Hamilton energy principle (Eq. (24)), integrating by part and separate the terms of displacement (δu_0 ; δv_0 ; δw_0 and $\delta \theta$). The equations of motion can be obtained as follow

$$\begin{aligned}
 \delta u_0: \quad & \frac{\partial N_x}{\partial x} + \frac{\partial N_{xy}}{\partial y} = I_0 \ddot{u}_0 - I_1 \frac{\partial \ddot{w}_0}{\partial x} + J_1 k_1 A' \frac{\partial \ddot{\theta}}{\partial x} \\
 \delta v_0: \quad & \frac{\partial N_{xy}}{\partial x} + \frac{\partial N_y}{\partial y} = I_0 \ddot{v}_0 - I_1 \frac{\partial \ddot{w}_0}{\partial y} + J_1 k_2 B' \frac{\partial \ddot{\theta}}{\partial y} \\
 \delta w_0: \quad & \frac{\partial^2 M_x^b}{\partial x^2} + 2 \frac{\partial^2 M_{xy}^b}{\partial x \partial y} + \frac{\partial^2 M_y^b}{\partial y^2} - q_{Winkler} - \\
 & q_{Pasternak} - q_{Kerr} = I_0 \ddot{w}_0 + I_1 \left(\frac{\partial \ddot{u}_0}{\partial x} + \frac{\partial \ddot{v}_0}{\partial y} \right) + \\
 & J_2 \left(k_1 A' \frac{\partial^2 \ddot{\theta}}{\partial x^2} + k_2 B' \frac{\partial^2 \ddot{\theta}}{\partial y^2} \right) - I_2 \nabla^2 \ddot{w}_0 + J_0 I_1^s \ddot{\theta} \\
 \delta \theta: \quad & -k_1 A' \frac{\partial^2 M_x^s}{\partial x^2} - k_2 B' \frac{\partial^2 M_y^s}{\partial y^2} - \\
 & (k_1 A' + k_2 B') \frac{\partial^2 M_{xy}^s}{\partial x \partial y} + k_1 A' \frac{\partial Q_{xz}}{\partial x} + \\
 & k_2 B' \frac{\partial Q_{yz}}{\partial y} - N_z + \frac{\partial S_{xz}^s}{\partial x} + \frac{\partial S_{yz}^s}{\partial y} = \\
 & -J_1 \left(k_1 A' \frac{\partial \ddot{u}_0}{\partial x} + k_2 B' \frac{\partial \ddot{v}_0}{\partial y} \right) + \\
 & J_2 \left(k_1 A' \frac{\partial^2 \ddot{w}_0}{\partial x^2} + k_2 B' \frac{\partial^2 \ddot{w}_0}{\partial y^2} \right) \\
 & -K_2 \left((k_1 A')^2 \frac{\partial^2 \ddot{\theta}}{\partial x^2} + (k_2 B')^2 \frac{\partial^2 \ddot{\theta}}{\partial y^2} \right) + J_1^s \ddot{w}_0 + K_2^s \ddot{\theta}
 \end{aligned} \quad (34)$$

Substituting Eq. (12) into Eq. (16) and the obtained results into Eq. (26), the stress resultants N, M, Q and S are obtained in terms of strains ε , k^b , k^s and γ as follow

$$\begin{Bmatrix} N \\ M^b \\ M^s \end{Bmatrix} = \begin{bmatrix} A & B & B^s \\ B & D & D^s \\ B^s & D^s & H^s \end{bmatrix} \begin{Bmatrix} \varepsilon \\ k^b \\ k^s \end{Bmatrix} + \varepsilon^0 \begin{Bmatrix} L \\ L^a \\ R \end{Bmatrix}, \quad (35a)$$

$$\begin{Bmatrix} Q \\ S \end{Bmatrix} = \begin{bmatrix} F^s & X^s \\ X^s & A^s \end{bmatrix} \begin{Bmatrix} \gamma^0 \\ \gamma^1 \end{Bmatrix}$$

$$N_z = L(\varepsilon_x^0 + \varepsilon_y^0) + L^a(k_x^b + k_y^b) + R(k_x^s + k_y^s) + R^a \varepsilon_z^0 \quad (35b)$$

in which

$$N = \{N_x, N_y, N_{xy}\}^t, \quad M^b = \{M_x^b, M_y^b, M_{xy}^b\}^t, \quad (36a)$$

$$M^s = \{M_x^s, M_y^s, M_{xy}^s\}^t,$$

$$\varepsilon = \{\varepsilon_x^0, \varepsilon_y^0, \gamma_{xy}^0\}^t, \quad k^b = \{k_x^b, k_y^b, k_{xy}^b\}^t, \quad (36b)$$

$$k^s = \{k_x^s, k_y^s, k_{xy}^s\}^t,$$

$$A = \begin{bmatrix} A_{11} & A_{12} & 0 \\ A_{12} & A_{22} & 0 \\ 0 & 0 & A_{66} \end{bmatrix}, \quad B = \begin{bmatrix} B_{11} & B_{12} & 0 \\ B_{12} & B_{22} & 0 \\ 0 & 0 & B_{66} \end{bmatrix},$$

$$D = \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{12} & D_{22} & 0 \\ 0 & 0 & D_{66} \end{bmatrix}, \quad (36c)$$

$$B^s = \begin{bmatrix} B_{11}^s & B_{12}^s & 0 \\ B_{12}^s & B_{22}^s & 0 \\ 0 & 0 & B_{66}^s \end{bmatrix}, \quad D^s = \begin{bmatrix} D_{11}^s & D_{12}^s & 0 \\ D_{12}^s & D_{22}^s & 0 \\ 0 & 0 & D_{66}^s \end{bmatrix}, \quad (36d)$$

$$H^s = \begin{bmatrix} H_{11}^s & H_{12}^s & 0 \\ H_{12}^s & H_{22}^s & 0 \\ 0 & 0 & H_{66}^s \end{bmatrix},$$

$$Q = \{Q_{xz}^s, Q_{yz}^s\}^t, \quad S = \{S_{xz}^s, S_{yz}^s\}^t, \quad (36e)$$

$$\gamma^0 = \{\gamma_{xz}^0, \gamma_{yz}^0\}^t, \quad \gamma^1 = \{\gamma_{xz}^1, \gamma_{yz}^1\}^t$$

$$F^s = \begin{bmatrix} F_{55}^s & 0 \\ 0 & F_{44}^s \end{bmatrix}, \quad X^s = \begin{bmatrix} X_{55}^s & 0 \\ 0 & X_{44}^s \end{bmatrix}, \quad (36f)$$

$$A^s = \begin{bmatrix} A_{55}^s & 0 \\ 0 & A_{44}^s \end{bmatrix}$$

$$\begin{Bmatrix} L \\ L^a \\ R \\ R^a \end{Bmatrix} = \int_z \lambda(z) \begin{Bmatrix} 1 \\ z \\ f(z) \\ g'(z) \frac{1-\nu(z)}{\nu(z)} \end{Bmatrix} g'(z) dz \quad (36g)$$

and stiffness components $A, B, D, B^s, D^s, H^s, F^s, X^s$ and A^s are given as

$$\begin{Bmatrix} A_{11} & B_{11} & D_{11} & B_{11}^s & D_{11}^s & H_{11}^s \\ A_{12} & B_{12} & D_{12} & B_{12}^s & D_{12}^s & H_{12}^s \\ A_{66} & B_{66} & D_{66} & B_{66}^s & D_{66}^s & H_{66}^s \end{Bmatrix} =$$

$$\int_z \lambda(z) \begin{Bmatrix} 1-\nu(z) \\ \nu(z) \\ 1 \\ 1-2\nu(z) \\ 2\nu(z) \end{Bmatrix} dz \quad (37a)$$

$$(A_{22}, B_{22}, D_{22}, B_{22}^s, D_{22}^s, H_{22}^s) = (A_{11}, B_{11}, D_{11}, B_{11}^s, D_{11}^s, H_{11}^s) \quad (37b)$$

$$(F_{44}^s, X_{44}^s, A_{44}^s) = \int_{-h/2}^{h/2} \frac{E(z)}{2(1+\nu(z))} \left([f'(z)]^2, f'(z)g(z), g^2(z) \right) dz \quad (37c)$$

$$(F_{55}^s, X_{55}^s, A_{55}^s) = (F_{44}^s, X_{44}^s, A_{44}^s) \quad (37d)$$

By substituting Eqs. (35) into Eq. (34), the equation of motion can be expressed in terms of displacements (u_0, v_0, w_0, θ) and the appropriate equations are obtained as

$$A_{11}d_{11}u_0 + A_{66}d_{22}u_0 + (A_{12} + A_{66})d_{12}v_0 - B_{11}d_{11}w_0 - (B_{12} + 2B_{66})d_{12}w_0 + (B_{66}^s(k_1A' + k_2B') + B_{12}^sk_2B')d_{122}\theta + B_{11}^sk_1A'd_{111}\theta + Ld_1\theta = I_0\ddot{u}_0 - I_1d_1\ddot{w}_0 + J_1k_1A'd_1\ddot{\theta} \quad (38a)$$

$$A_{22}d_{22}v_0 + A_{66}d_{11}v_0 + (A_{12} + A_{66})d_{12}u_0 - B_{22}d_{22}w_0 - (B_{12} + 2B_{66})d_{11}w_0 + (B_{66}^s(k_1A' + k_2B') + B_{12}^sk_1A')d_{112}\theta - B_{22}^sk_2B'd_{222}\theta + Ld_2\theta = I_0\ddot{v}_0 - I_1d_2\ddot{w}_0 + J_1k_2B'd_2\ddot{\theta} \quad (38b)$$

$$B_{11}d_{11}u_0 + (B_{12} + 2B_{66})d_{12}u_0 + (B_{12} + 2B_{66})d_{112}v_0 + B_{22}d_{22}v_0 - D_{11}d_{111}w_0 - 2(D_{12} + 2D_{66})d_{112}w_0 - D_{22}d_{222}w_0 + D_{11}^sk_1A'd_{111}\theta + (D_{12}^s + 2D_{66}^s)(k_1A' + k_2B')d_{1122}\theta + D_{22}^sk_2B'd_{2222}\theta + L^a(d_{11}\theta + d_{22}\theta) - (K_w w_0 - G_p(d_{11}w_0 + d_{22}w_0)) - \left(\left(\frac{K_l K_u}{K_l + K_u} \right) w_0 - \left(\frac{K_s K_u}{K_l + K_u} \right) (d_{11}w_0 + d_{22}w_0) \right) = I_0\ddot{w}_0 \quad (38c)$$

$$+ I_1(d_1\ddot{u}_0 + d_2\ddot{v}_0) - I_2(d_{11}\ddot{w}_0 + d_{22}\ddot{w}_0) + J_2(k_1A'd_{11}\ddot{\theta} + k_2B'd_{22}\ddot{\theta}) + J_1^{st}\ddot{\theta} - k_1A'B_{11}^sd_{111}u_0 - (B_{12}^sk_2B' + B_{66}^s(k_1A' + k_2B'))d_{122}u_0 - (B_{12}^sk_1A' + B_{66}^s(k_1A' + k_2B'))d_{112}v_0 - B_{22}^sk_2B'd_{222}v_0 + D_{11}^sk_1A'd_{1111}w_0 + (D_{12}^s + 2D_{66}^s)(k_1A' + k_2B')d_{1122}w_0 + D_{22}^sk_2B'd_{2222}w_0 - H_{11}^s(k_1A')^2d_{1111}\theta - H_{22}^s(k_2B')^2d_{2222}\theta - (2H_{12}^sk_1A'k_2B' + (k_1A' + k_2B')^2H_{66}^s)d_{1122}\theta + ((k_1A')^2F_{55}^s + 2k_1A'X_{55}^s + A_{55}^s)d_{11}\theta + ((k_2B')^2F_{44}^s + 2k_2B'X_{44}^s + A_{44}^s)d_{22}\theta - 2R(k_1A'd_{11}\theta + k_2B'd_{22}\theta) - L(d_1u_0 + d_2v_0) + L^a(d_{11}w_0 + d_{22}w_0) - R^a\theta = -J_1(k_1A'd_1\ddot{u}_0 + k_2B'd_2\ddot{v}_0) + J_2(k_1A'd_{11}\ddot{w}_0 + k_2B'd_{22}\ddot{w}_0) - K_2((k_1A')^2d_{11}\ddot{\theta} + (k_2B')^2d_{22}\ddot{\theta}) + J_1^{st}\ddot{w}_0 + K_2^{st}\ddot{\theta} \quad (38d)$$

Where the following differential operators (d_{ij} , d_{ijl} and d_{ijlm}) are given as

$$d_{ij} = \frac{\partial^2}{\partial x_i \partial x_j}, \quad d_{ijl} = \frac{\partial^3}{\partial x_i \partial x_j \partial x_l}, \quad d_{ijlm} = \frac{\partial^4}{\partial x_i \partial x_j \partial x_l \partial x_m}, \quad (39)$$

$$d_i = \frac{\partial}{\partial x_i}, \quad (i, j, l, m = 1, 2).$$

2.5 Closed-form solutions

In this investigation, Navier solution method is employed to solve the equation of motion of Eq. (38) and assured the boundary conditions (simply supported). The

Navier method can be expressed in double trigonometric functions as follow

$$\begin{Bmatrix} u_0 \\ v_0 \\ w_0 \\ \theta \end{Bmatrix} = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \begin{Bmatrix} U_{mn} e^{i\omega t} \cos(\alpha x) \sin(\beta y) \\ V_{mn} e^{i\omega t} \sin(\alpha x) \cos(\beta y) \\ W_{mn} e^{i\omega t} \sin(\alpha x) \sin(\beta y) \\ X_{mn} e^{i\omega t} \sin(\alpha x) \sin(\beta y) \end{Bmatrix} \quad (40)$$

where ω is the frequency of free vibration of the FG-plate, $\sqrt{-1}$ the imaginary unit with

$$\alpha = m\pi / a \quad \text{and} \quad \beta = n\pi / b \quad (41)$$

Substituting Eq. (40) into Eq. (38), the following equation is obtained

$$\begin{Bmatrix} S_{11} & S_{12} & S_{13} & S_{15} \\ S_{12} & S_{22} & S_{23} & S_{24} \\ S_{13} & S_{23} & S_{33} & S_{34} \\ S_{14} & S_{24} & S_{34} & S_{44} \end{Bmatrix} - \omega^2 \begin{Bmatrix} m_{11} & 0 & m_{13} & m_{14} \\ 0 & m_{22} & m_{23} & m_{24} \\ m_{13} & m_{23} & m_{33} & m_{34} \\ m_{14} & m_{24} & m_{34} & m_{44} \end{Bmatrix} \begin{Bmatrix} U_{mn} \\ V_{mn} \\ W_{mn} \\ X_{mn} \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \quad (42)$$

Where

$$\begin{aligned} S_{11} &= (\alpha^2 A_{11} + \beta^2 A_{66}) \\ S_{12} &= \alpha\beta(A_{12} + A_{66}) \\ S_{13} &= -\alpha^3 B_{11} - \alpha\beta^2(B_{12} + 2B_{66}) \\ S_{14} &= \alpha((k_2B'B_{12}^s + (k_1A' + k_2B')B_{66}^s)\beta^2 + k_1A'B_{11}^s\alpha^2 - L) \\ S_{22} &= (\alpha^2 A_{66} + \beta^2 A_{22}) \\ S_{23} &= -\alpha^2\beta(B_{12} + 2B_{66}) - \beta^3 B_{22} \\ S_{24} &= \beta((k_1A'B_{12}^s + (k_1A' + k_2B')B_{66}^s)\alpha^2 + k_2B'B_{22}^s\beta^2 - L) \\ S_{33} &= (\alpha^4 D_{11} + \beta^4 D_{22} + 2\alpha^2\beta^2(D_{12} + 2D_{66})) + \\ &K_w + G_p(\alpha^2 + \beta^2) + \left(\frac{K_l K_u}{K_l + K_u} \right) + \left(\frac{K_s K_u}{K_l + K_u} \right)(\alpha^2 + \beta^2) \\ S_{34} &= -(\alpha^4 k_1A'D_{11}^s + \beta^4 k_2B'D_{22}^s) - \\ &\alpha^2\beta^2(k_1A' + k_2B')(D_{12}^s + 2D_{66}^s) + L^a(\alpha^2 + \beta^2) \\ S_{44} &= \alpha^4(k_1A')^2 H_{11}^s + \beta^4(k_2B')^2 H_{22}^s + \\ &(2k_1k_2A'B'H_{12}^s + (k_1A' + k_2B')^2 H_{66}^s)\alpha^2\beta^2 \\ &+ \alpha^2((k_1A')^2 F_{55}^s + 2k_1A'X_{55}^s + A_{55}^s) + \\ &\beta^2((k_2B')^2 F_{44}^s + 2k_2B'X_{44}^s + A_{44}^s) \\ &- 2R(k_1A'\alpha^2 + k_2B'\beta^2) + R^a \end{aligned} \quad (43)$$

and

$$\begin{aligned} m_{11} &= I_0, \quad m_{13} = -\alpha I_1, \quad m_{14} = J_1 k_1 A' \alpha, \\ m_{22} &= I_0, \quad m_{23} = -\beta I_1, \quad m_{24} = k_2 B' \beta J_1, \\ m_{33} &= I_0 + I_2(\alpha^2 + \beta^2), \\ m_{34} &= -J_2(k_1 A' \alpha^2 + k_2 B' \beta^2) + J_1^{st}, \\ m_{44} &= K_2((k_1 A')^2 \alpha^2 + (k_2 B')^2 \beta^2) + K_2^{st} \end{aligned} \quad (44)$$

Table 1 Non-dimensional fundamental frequency “ $\hat{\omega}$ ” of isotropic square plate ($a=10$, $\nu=0.3$, $h/a=0.1$, $E=30 \times 10^6$)

Model	Var.	Mode (m, n)					
		(1, 1)	(1, 2)	(2, 2)	(2, 3)	(3, 3)	(2, 4) (1, 5)
Zhou <i>et al.</i> (2002)	-	0.0932	0.2226	0.3421	0.5239	0.6889	0.7511 0.9268
Jha <i>et al.</i> (2013)	10	0.0932	0.2226	0.3421	0.5240	0.6892	0.7515 0.9275
Akavci and Tanrikulu (2015)	6	0.0932	0.2227	0.3424	0.5247	0.6902	0.7526 0.9290
Benahmed <i>et al.</i> (2017)	5	0.0932	0.2229	0.3425	0.5248	0.6904	0.7528 0.9294
Farzam-Rad <i>et al.</i> (2017)	5	0.0932	0.2227	0.3423	0.5243	0.6896	0.7520 0.9284
Shahsavari <i>et al.</i> (2018b)	5	0.0932	0.2226	0.3421	0.5240	0.6892	0.7514 0.9274
Present	4	0.0934	0.2234	0.3436	0.5264	0.6924	0.7548 0.9318

3. Numerical results and discussion

In this part, the dynamic behavior analysis of simply supported FG-plate reposed on elastic foundations (Winkler/Pasternak/Kerr) is investigated. Several numerical results of frequency parameters for isotropic perfect and imperfect FG plate are presented in explicit tables and graphs.

For comparison, the following non-dimensional foundation parameters and fundamental natural frequencies are employed (Benahmed *et al.* 2017, Wattanasakulpong and Ungbhakorn 2014)

Frequency parameters:

$$\hat{\omega} = \omega h \sqrt{\rho/G}, \quad \tilde{\omega} = \omega h \sqrt{\rho_m/E_m}, \quad \bar{\omega} = \omega \frac{a^2}{h} \sqrt{\rho_m/E_m}, \quad (45)$$

Foundation parameters:

$$\bar{K}_w = \frac{K_w a^4}{D_{11}}, \quad \bar{G}_p = \frac{G_p a^2}{D_{11}}, \quad (46)$$

$$\bar{K}_t = \frac{K_t a^4}{D_{11}}, \quad \bar{K}_u = \frac{K_u a^4}{D_{11}}, \quad \bar{K}_s = \frac{K_s a^2}{D_{11}}$$

$$\text{With } D_{11} = (E_m h^3) / (12(1 - \nu_m^2)).$$

3.1 Isotropic plate

The Table 1, shows the non-dimensional fundamental frequency “ $\hat{\omega}$ ” of simply supported isotropic square plate with ($h/a=0.1$ and $E=30.10^6$). The obtained results are

Table 2 Non-dimensional fundamental frequency “ $\tilde{\omega}$ ” of FG-plates

h/a	Model	$b/a=1$			$b/a=2$		
		$p=0$	$p=1$	$p=2$	$p=0$	$p=1$	$p=2$
0.1	Jin <i>et al.</i> (2014)	0.1135	0.0870	0.0789	0.0719	0.0550	0.0499
	Mantari <i>et al.</i> (2014)	0.1135	0.0882	0.0806	0.0718	0.0557	0.0510
	Farzam-Rad <i>et al.</i> (2017)	0.1136	0.0882	0.0806	0.0719	0.0558	0.0510
	Shahsavari <i>et al.</i> (2018b)	0.1135	0.0882	0.0806	0.0718	0.0557	0.0510
	Present	0.1138	0.0884	0.0807	0.0720	0.0558	0.0510
0.2	Jin <i>et al.</i> (2014)	0.4169	0.3222	0.2905	0.2713	0.2088	0.1888
	Mantari <i>et al.</i> (2014)	0.4168	0.3260	0.2961	0.2712	0.2115	0.1926
	Farzam-Rad <i>et al.</i> (2017)	0.4170	0.3262	0.2961	0.2714	0.2116	0.1926
	Shahsavari <i>et al.</i> (2018b)	0.4168	0.3260	0.2961	0.2720	0.2115	0.1926
	Present	0.4185	0.3272	0.2966	0.2724	0.2124	0.1929
0.5	Jin <i>et al.</i> (2014)	1.8470	1.4687	1.3095	0.9570	0.7937	0.7149
	Mantari <i>et al.</i> (2014)	1.8505	1.4774	1.3219	1.3040	1.0346	0.9293
	Farzam-Rad <i>et al.</i> (2017)	1.8528	1.4788	1.3226	0.9570	0.7961	0.7193
	Shahsavari <i>et al.</i> (2018b)	1.8503	1.4772	1.3218	1.3039	1.0345	0.9293
	Present	1.8588	1.4836	1.3254	1.3110	1.0400	0.9318

Table 3 Non-dimensional fundamental frequencies “ $\bar{\omega}$ ” of square FG-plates resting on Winkler-Pasternak foundations ($p=2.3$, $E_c/E_m=10$, $h/a=0.1$)

\bar{G}_p	Model	ε_z	Var.	\bar{K}_w			
				0	10	100	1000
0	Lü <i>et al.</i> (2009)	$\neq 0$	-	5.1295	5.1520	5.3498	7.0281
	Benahmed <i>et al.</i> (2017)	$\neq 0$	5	5.1638	5.1871	5.3923	7.1262
	Thai and Choi (2011)	$= 0$	4	5.2385	5.2605	5.4548	7.1116
	Shahsavari <i>et al.</i> (2018b)	$\neq 0$	5	5.1556	5.1791	5.3855	7.1285
	Present	$\neq 0$	4	5.1637	5.1870	5.3920	7.1250
10	Lü <i>et al.</i> (2009)	$\neq 0$	-	5.5560	5.5767	5.7600	7.3450
	Benahmed <i>et al.</i> (2017)	$\neq 0$	5	5.6059	5.6274	5.8171	7.4527
	Thai and Choi (2011)	$= 0$	4	5.6576	5.6780	5.8584	7.4257
	Shahsavari <i>et al.</i> (2018b)	$\neq 0$	5	5.6004	5.6220	5.8127	7.4565
	Present	$\neq 0$	4	5.6055	5.6269	5.8165	7.4514
25	Lü <i>et al.</i> (2009)	$\neq 0$	-	6.1404	6.1591	6.3255	7.7962
	Benahmed <i>et al.</i> (2017)	$\neq 0$	5	6.2103	6.2297	6.4015	7.9172
	Thai and Choi (2011)	$= 0$	4	6.2336	6.2521	6.4164	7.8734
	Shahsavari <i>et al.</i> (2018b)	$\neq 0$	5	6.2080	6.2275	6.4002	7.9230
	Present	$\neq 0$	4	6.2095	6.2289	6.4006	7.9157

Table 4 Non-dimensional fundamental frequencies “ $\tilde{\omega}$ ” of square isotropic and FG-plates resting on Winkler-Pasternak foundations

\bar{K}_w	\bar{G}_p	h/a	Model	P				
				0	0.5	1	2	5
0	0	0.05	Benahmed <i>et al.</i> (2017)	0.0291	-	0.0226	0.0207	-
			Baferani <i>et al.</i> (2011)	0.0290	0.0249	0.0227	0.0209	0.0197
			Shahsavari <i>et al.</i> (2018b)	0.0291	0.0248	0.0226	0.0206	0.0195
			Present	0.0291	0.0248	0.0226	0.0207	0.0195
		0.1	Benahmed <i>et al.</i> (2017)	0.1136	-	0.0883	0.0807	-
			Baferani <i>et al.</i> (2011)	0.1134	0.0975	0.0891	0.0819	0.0767
			Shahsavari <i>et al.</i> (2018b)	0.1135	0.0970	0.0882	0.0806	0.0755
			Present	0.1137	0.0973	0.0883	0.0806	0.0756
		0.15	Benahmed <i>et al.</i> (2017)	0.2461	-	0.1918	0.1748	-
			Baferani <i>et al.</i> (2011)	0.2454	0.2121	0.1939	0.1778	0.1648
			Shahsavari <i>et al.</i> (2018b)	0.2459	0.2109	0.1916	0.1746	0.1622
			Present	0.2466	0.2116	0.1921	0.1748	0.1624
		0.2	Benahmed <i>et al.</i> (2017)	0.4174	-	0.3264	0.2965	-
			Baferani <i>et al.</i> (2011)	0.4154	0.3606	0.3299	0.3016	0.2765
			Shahsavari <i>et al.</i> (2018b)	0.4168	0.3586	0.3260	0.2961	0.2722
			Present	0.4184	0.3602	0.3272	0.2966	0.2726
100	0	0.05	Benahmed <i>et al.</i> (2017)	0.0298	-	0.0236	0.0218	-
			Baferani <i>et al.</i> (2011)	0.0298	0.0258	0.0238	0.0221	0.0210
			Shahsavari <i>et al.</i> (2018b)	0.0298	0.0257	0.0236	0.0218	0.0208
			Present	0.0298	0.0257	0.0236	0.0218	0.0208
		0.1	Benahmed <i>et al.</i> (2017)	0.1164	-	0.0924	0.0854	-
			Baferani <i>et al.</i> (2011)	0.1162	0.1012	0.0933	0.0867	0.0821
			Shahsavari <i>et al.</i> (2018b)	0.1163	0.1006	0.0923	0.0853	0.0809
			Present	0.1165	0.1008	0.0924	0.0854	0.0809
		0.15	Benahmed <i>et al.</i> (2017)	0.2524	-	0.2011	0.1855	-
			Baferani <i>et al.</i> (2011)	0.2519	0.2204	0.2036	0.1889	0.1775
			Shahsavari <i>et al.</i> (2018b)	0.2522	0.2190	0.2010	0.1855	0.1745
			Present	0.2528	0.2196	0.2014	0.1856	0.1746
		0.2	Benahmed <i>et al.</i> (2017)	0.4286	-	0.3431	0.3158	-
			Baferani <i>et al.</i> (2011)	0.4273	0.3758	0.3476	0.3219	0.2999
			Shahsavari <i>et al.</i> (2018b)	0.4284	0.3734	0.3431	0.3159	0.2950
			Present	0.4298	0.3748	0.3438	0.3158	0.2948
100	100	0.05	Benahmed <i>et al.</i> (2017)	0.0411	-	0.0386	0.0383	-
			Baferani <i>et al.</i> (2011)	0.0411	0.0395	0.0388	0.0386	0.0388
			Shahsavari <i>et al.</i> (2018b)	0.0411	0.0393	0.0386	0.0383	0.0385
			Present	0.0411	0.0393	0.0386	0.0383	0.0385
		0.1	Benahmed <i>et al.</i> (2017)	0.1614	-	0.1521	0.1509	-
			Baferani <i>et al.</i> (2011)	0.1619	0.1563	0.1542	0.1535	0.1543
			Shahsavari <i>et al.</i> (2018b)	0.1616	0.1551	0.1525	0.1512	0.1521
			Present	0.1615	0.1550	0.1523	0.1510	0.1516
		0.15	Benahmed <i>et al.</i> (2017)	0.3537	-	0.3349	0.3323	-
			Baferani <i>et al.</i> (2011)	0.3560	0.3460	0.3422	0.3412	0.3427
			Shahsavari <i>et al.</i> (2018b)	0.3551	0.3421	0.3367	0.3342	0.3358
			Present	0.3544	0.3414	0.3358	0.3328	0.3336
		0.2	Benahmed <i>et al.</i> (2017)	0.6089	-	0.5794	0.5752	-
			Baferani <i>et al.</i> (2011)	0.6162	0.6026	0.5978	0.5970	0.5993
			Shahsavari <i>et al.</i> (2018b)	0.6137	0.5940	0.5856	0.5815	0.5843
			Present	0.6118	0.5920	0.5828	0.5776	0.5784

compared with those given by exact 3D solution developed by Zhou *et al.* (2002) and the existing quasi-3D theories in the literature such as ten variables quasi-3D theory published by Jha *et al.* (2013), six variables quasi-3D theory developed by Akavci and Tanrikulu (2015), and five variables quasi-3D theories of (Benahmed *et al.* 2017, Farzam-Rad *et al.* 2017, Shahsavari *et al.* 2018b). From the table, it can be seen that the present theory with only four unknown is in good agreement with exact 3D solution and

others Quasi-3D theories.

3.2 Functionally graded plate (FGM)

The second part of the results is reserved for functionally graded plate. The properties of materials used in the FG-plate are the alumina Al_2O_3 (ceramic) with Young's modulus $E_c=380$ GPa and density $E_c=3800$ kg/m³ and the second material is aluminum Al with Young's

Table 5 Non-dimensional fundamental frequencies “ $\bar{\omega}$ ” of square isotropic and FG-plates resting on Kerr foundation ($\bar{K}_l = 100$)

\bar{K}_u	\bar{K}_s	h/a	Model	Isotropic plate		FG plate			
				Ceramic	Metal	$P=0.5$	$P=1.0$	$P=2.0$	$P=5.0$
100	0	0.05	Shahsavari <i>et al.</i> (2018b)	0.0294	0.0157	0.0253	0.0231	0.0212	0.0202
			Present	0.0295	0.0158	0.0253	0.0231	0.0213	0.0202
		0.1	Shahsavari <i>et al.</i> (2018b)	0.1149	0.0615	0.0988	0.0903	0.0830	0.0783
			Present	0.1151	0.0616	0.0991	0.0904	0.0831	0.0783
		0.15	Shahsavari <i>et al.</i> (2018b)	0.2491	0.1337	0.2149	0.1964	0.1801	0.1685
			Present	0.2498	0.1340	0.2156	0.1969	0.1803	0.1686
		0.2	Shahsavari <i>et al.</i> (2018b)	0.4226	0.2278	0.3661	0.3347	0.3061	0.2838
			Present	0.4242	0.2282	0.3676	0.3356	0.3064	0.2840
		0.05	Shahsavari <i>et al.</i> (2018b)	0.0356	0.0285	0.0329	0.0316	0.0308	0.0305
			Present	0.0356	0.0284	0.0329	0.0316	0.0308	0.0305
100	100	0.1	Shahsavari <i>et al.</i> (2018b)	0.1396	0.1125	0.1294	0.1245	0.1212	0.1201
			Present	0.1397	0.1123	0.1294	0.1245	0.1210	0.1198
		0.15	Shahsavari <i>et al.</i> (2018b)	0.3054	0.2487	0.2824	0.2740	0.2666	0.2637
			Present	0.3054	0.2476	0.2840	0.2736	0.2658	0.2624
		0.2	Shahsavari <i>et al.</i> (2018b)	0.5246	0.4332	0.4906	0.4739	0.4615	0.4560
			Present	0.5242	0.4306	0.4900	0.4726	0.4592	0.4522
		0.05	Shahsavari <i>et al.</i> (2018b)	0.0375	0.0317	0.0351	0.0341	0.0335	0.0334
			Present	0.0375	0.0317	0.0351	0.0341	0.0334	0.0334
		0.1	Shahsavari <i>et al.</i> (2018b)	0.1473	0.1255	0.1385	0.1345	0.1320	0.1316
			Present	0.1473	0.1252	0.1385	0.1344	0.1318	0.1313
200	100	0.15	Shahsavari <i>et al.</i> (2018)	0.3228	0.2779	0.3047	0.2964	0.2909	0.2897
			Present	0.3226	0.2766	0.3044	0.2958	0.2898	0.2882
		0.2	Shahsavari <i>et al.</i> (2018b)	0.5559	0.4850	0.5273	0.5139	0.5047	0.5024
			Present	0.5550	0.4816	0.5262	0.5120	0.5018	0.4978
		0.05	Shahsavari <i>et al.</i> (2018b)	0.0440	0.0419	0.0427	0.0423	0.0422	0.0426
			Present	0.0440	0.0419	0.0427	0.0422	0.0422	0.0426
		0.1	Shahsavari <i>et al.</i> (2018b)	0.1735	0.1660	0.1687	0.1670	0.1668	0.1684
			Present	0.1733	0.1655	0.1685	0.1667	0.1664	0.1679
		0.15	Shahsavari <i>et al.</i> (2018b)	0.3819	0.3686	0.3728	0.3694	0.3689	0.3725
			Present	0.3810	0.3664	0.3718	0.3682	0.3672	0.3700
200	200	0.2	Shahsavari <i>et al.</i> (2018b)	0.6617	0.5511	0.6484	0.6436	0.6431	0.6494
			Present	0.6590	0.5510	0.6454	0.6404	0.6386	0.6424

modulus $E_m=70$ GPa and density $\rho_m=2702$ Kg/m³ and the poisson's ratio's is $\nu=0.3$ for the both materials (ceramic and metal).

3.2.1 Perfect simply supported FG-plate

The Table 2 illustrates the variation of the non-dimensional frequency parameter “ $\bar{\omega}$ ” of simply supported square “ $b/a=1$ ” and rectangular “ $b/a=2$ ” FG-plate as function of the geometry ratio “ h/a ” and material index “ p ”. The current results computed using with present four variables quasi-3D theory are compared with those obtained by quasi-3D theories of (Mantari *et al.* 2014, Farzam-Rad *et al.* 2017, Shahsavari *et al.* 2018b) and the exact 3D solution developed by Jin *et al.* (2014). It can be observed from the Table that a good agreement is confirmed between the present results and those obtained by the other quasi-3D and exact 3D theories and this for moderately thick “ $h/a=0.1$ ”, thick “ $h/a=0.2$ ” and very thick “ $h/a=0.5$ ” square FG-plate. A small difference is noticed between the current results and those obtained via exact 3D solution and quasi 3D theory of Farzam-Rad *et al.* (2017) for a very thick rectangular FG-plate. It can also be remarkable that the non-dimensional frequency parameter “ $\bar{\omega}$ ” is in direct correlation relation with the geometry ratio “ h/a ” and in

inverse relation with material index “ p ”.

3.2.2 Perfect FG-plates resting on elastic foundation (Winkler-Pasternak- Kerr)

The Table 3 presents the values of the non-dimensional fundamental frequencies “ $\bar{\omega}$ ” of FG square plates resting on Winkler-Pasternak foundations with ($p=2.3$, $E_c/E_m=10$ and $h/a=0.1$). The computed frequencies using the current model are compared with those given by a quasi-3D theories with five unknowns proposed by Benahmed *et al.* (2017) and Shahsavari *et al.* (2018b), refined plate theory developed by Thai and Choi (2011) and the exact solution of Lü *et al.* (2009). From the obtained results it can be observed that the current model gives almost the same results as the five variables quasi-3D theories and exact 3D solution. It is obvious also that the increase in the values of spring constant “ \bar{K}_w ” and shear layer parameter “ \bar{G}_p ” leads to an increase in the non-dimensional fundamental frequencies, so we can conclude that the presence of the foundation makes the plate stiffer.

The Table 4 illustrates the variations of the non-dimensional fundamental frequencies “ $\bar{\omega}$ ” of FG square plates versus the geometry ratios “ h/a ”, material index “ p ”

Table 6 Variations of frequency parameters " $\bar{\omega}$ " of perfect and imperfect FG-square plates versus the Winkler-Pasternak foundation stiffness ($p=1$)

(\bar{K}_w, \bar{G}_p)	h/a	ζ	Even porosity	Uneven porosity	Logarithmic-uneven porosity	Mass-density porosity	Perfect
(0,0)	0.05	0.05	8.8888	9.0368	9.0368	8.6248	9.030
		0.1	8.7352	9.0456	9.0456	8.1224	
		0.15	8.5656	9.0552	9.0544	7.4713	
		0.2	8.3728	9.0656	9.0640	6.5652	
	0.1	0.05	8.6992	8.8408	8.8402	8.4432	8.836
		0.1	8.5520	8.8464	8.8458	7.9568	
		0.15	8.3898	8.8532	8.8526	7.3262	
		0.2	8.2058	8.8606	8.8594	6.4470	
	0.15	0.05	8.4131	8.5429	8.5426	8.1670	8.541
		0.1	8.2761	8.5456	8.5456	7.7040	
		0.15	8.1248	8.5494	8.5492	7.1036	
		0.2	7.9539	8.5542	8.5534	6.2656	
	0.2	0.05	8.0635	8.1795	8.1795	7.8280	8.180
		0.1	7.9385	8.1800	8.1800	7.3930	
		0.15	7.8015	8.1815	8.1810	6.8285	
		0.2	7.6450	8.1835	8.1830	6.0395	
(100,0)	0.05	0.05	9.3248	9.4560	9.4560	9.0736	9.439
		0.1	9.2032	9.4752	9.4744	8.6240	
		0.15	9.0696	9.4960	9.4936	8.0440	
		0.2	8.9192	9.5176	9.5136	7.2491	
	0.1	0.05	9.1372	9.2608	9.2602	8.8936	9.246
		0.1	9.0216	9.2770	9.2764	8.4588	
		0.15	8.8948	9.2950	9.2932	7.8990	
		0.2	8.7532	9.3142	9.3106	7.1298	
	0.15	0.05	8.8538	8.9654	8.9654	8.6202	8.954
		0.1	8.7480	8.9797	8.9788	8.2082	
		0.15	8.6328	8.9939	8.9930	7.6783	
		0.2	8.5037	9.0108	9.0072	6.9482	
	0.2	0.05	8.5095	8.6080	8.6080	8.2865	8.598
		0.1	8.4160	8.6195	8.6190	7.9025	
		0.15	8.3140	8.6320	8.6305	7.4075	
		0.2	8.1995	8.6455	8.6430	6.7240	
(100,100)	0.05	0.05	15.6210	15.5700	15.5680	15.4710	15.435
		0.1	15.8270	15.7100	15.7030	15.4960	
		0.15	16.0580	15.8570	15.8410	15.4990	
		0.2	16.3140	16.0100	15.9810	15.4610	
	0.1	0.05	15.4150	15.3570	15.3560	15.2690	15.226
		0.1	15.6250	15.4950	15.4880	15.3030	
		0.15	15.8600	15.6390	15.6230	15.3160	
		0.2	16.1210	15.7900	15.7610	15.2900	
	0.15	0.05	15.1180	15.0520	15.0500	14.9780	14.924
		0.1	15.3340	15.1860	15.1800	15.0240	
		0.15	15.5730	15.3260	15.3100	15.0520	
		0.2	15.8380	15.4730	15.4450	15.0390	
	0.2	0.05	14.7730	14.6990	14.6980	14.6420	14.574
		0.1	14.9930	14.8300	14.8230	14.7020	
		0.15	15.2360	14.9660	14.9510	14.7430	
		0.2	15.5060	15.1080	15.0820	14.7430	

and stiffness parameters of Winkler-Pasternak foundation (\bar{K}_w, \bar{G}_p). The current results are compared with those presented by Benahmed *et al.* (2017), Baferani *et al.* (2011) and Shahsavari *et al.* (2018b). It can be seen from the table that a good agreement is confirmed between the present results and those of the models existing in the literature. It can be also remarkable that the non-dimensional fundamental frequencies " $\bar{\omega}$ " are in direct correlation relation with the geometry ratios " h/a ". It is noticed also

that the power index " p " has a slight influence on the fundamental frequencies " $\bar{\omega}$ ". It is confirmed from the results that the presence of the elastic foundation leads to an increase in the frequency parameter " $\bar{\omega}$ ".

The Table 5 shows the effect of Kerr foundation on the non-dimensional fundamental frequencies " $\bar{\omega}$ " of isotropic (all ceramic and all metallic) and FG square plates for various values of power index " p ", thickness ratios " h/a ", upper spring and shear layer parameters (\bar{K}_u, \bar{K}_s). The

Table 7 Variations of frequency parameters “ $\bar{\omega}$ ” of perfect and imperfect FG-square plates versus the Kerr foundation stiffness ($p=1$, $\bar{K}_l = 100$)

(\bar{K}_u, \bar{K}_s)	h/a	ζ	Even porosity	Uneven porosity	Logarithmic-uneven porosity	Mass-density porosity	Perfect
(100,0)	0.05	0.05	9.1096	9.2488	9.2488	8.8520	9.2368
		0.1	8.9720	9.2632	9.2624	8.3768	
		0.15	8.8208	9.2784	9.2768	7.7630	
		0.2	8.6504	9.2944	9.2912	6.9156	
	0.1	0.05	8.9210	9.0532	9.0526	8.6712	9.0434
		0.1	8.7898	9.0644	9.0638	8.2114	
		0.15	8.6464	9.0770	9.0750	7.6182	
		0.2	8.4842	9.0906	9.0874	6.7974	
	0.15	0.05	8.6365	8.7568	8.7568	8.3968	8.7496
		0.1	8.5152	8.7651	8.7648	7.9602	
		0.15	8.3827	8.7748	8.7737	7.3966	
		0.2	8.2333	8.7855	8.7833	6.6157	
	0.2	0.05	8.2895	8.3965	8.3965	8.0605	8.3915
		0.1	8.1810	8.4025	8.4020	7.6520	
		0.15	8.0620	8.4100	8.4090	7.1240	
		0.2	7.9270	8.4175	8.4160	6.3910	
(100,100)	0.05	0.05	12.7090	12.7300	12.7290	12.5250	12.645
		0.1	12.7830	12.8180	12.8140	12.3710	
		0.15	12.8690	12.9120	12.9020	12.1660	
		0.2	12.9660	13.0100	12.9910	11.8780	
	0.1	0.05	12.5160	12.5310	12.5290	12.3380	12.449
		0.1	12.5960	12.6170	12.6130	12.1960	
		0.15	12.6880	12.7080	12.6980	12.0060	
		0.2	12.7920	12.8030	12.7850	11.7340	
	0.15	0.05	12.2340	12.2380	12.2370	12.0640	12.159
		0.1	12.3210	12.3220	12.3180	11.9400	
		0.15	12.4210	12.4100	12.4000	11.7700	
		0.2	12.5330	12.5020	12.4850	11.5220	
	0.2	0.05	11.9020	11.8960	11.8940	11.7410	11.818
		0.1	11.9980	11.9760	11.9720	11.6380	
		0.15	12.1060	12.0620	12.0520	11.4920	
		0.2	12.2260	12.1510	12.1340	11.2700	
(200,100)	0.05	0.05	13.7480	13.7420	13.7410	13.5780	13.639
		0.1	13.8720	13.8500	13.8440	13.4940	
		0.15	14.0130	13.9620	13.9500	13.3700	
		0.2	14.1700	14.0820	14.0580	13.1810	
	0.1	0.05	13.5510	13.5380	13.5370	13.3870	13.438
		0.1	13.6810	13.6440	13.6380	13.3130	
		0.15	13.8260	13.7550	13.7420	13.2020	
		0.2	13.9890	13.8700	13.8480	13.0280	
	0.15	0.05	13.2650	13.2430	13.2420	13.1080	13.146
		0.1	13.4010	13.3450	13.3410	13.0490	
		0.15	13.5530	13.4530	13.4410	12.9570	
		0.2	13.7240	13.5660	13.5440	12.8030	
	0.2	0.05	12.9300	12.8980	12.8970	12.7820	12.804
		0.1	13.0720	12.9980	12.9920	12.7410	
		0.15	13.2320	13.1020	13.0900	12.6690	
		0.2	13.4090	13.2110	13.1900	12.5360	

results computed using the current four variable quasi-3D theory are almost identical to those given via quasi-3D theory (with five unknown) of Shahsavari *et al.* (2018b).

It can be seen from the table that the non-dimensional fundamental frequencies “ $\bar{\omega}$ ” increase with increasing of upper spring and shear layer parameters (\bar{K}_u, \bar{K}_s). It can also be noted that the presence of the upper spring “ \bar{K}_u ” in the foundation of Kerr makes the plate stiffer, we can also confirm that the power-law index “ p ” has a slight influence

on the results. The biggest values of the non-dimensional frequency are obtained for fully ceramic plate and this is due to the high rigidity of ceramic.

3.2.3 Perfect and imperfect FG-plates resting on Winkler-Pasternak-Kerr elastic foundation

In this section, the presence of the porosity in the material that makes up the FG-plate is considered. Four model of distribution of the micro-voids is examined and presented in the following.

Table 7 Continued

(\bar{K}_u, \bar{K}_s)	h/a	ξ	Even porosity	Uneven porosity	Logarithmic-uneven porosity	Mass-density porosity	Perfect
(200,200)	0.05	0.05	17.138	17.0540	17.0520	17.0020	16.895
		0.1	17.406	17.2190	17.2100	17.1050	
		0.15	17.703	17.3910	17.3730	17.1980	
		0.2	18.033	17.5720	17.5380	17.2640	
	0.1	0.05	16.922	16.8320	16.8300	16.7890	16.677
		0.1	17.195	16.9950	16.9870	16.9000	
		0.15	17.495	17.1650	17.1460	17.0010	
		0.2	17.828	17.3420	17.3080	17.0770	
	0.15	0.05	16.614	16.5170	16.5140	16.4860	16.365
		0.1	16.889	16.6750	16.6670	16.6070	
		0.15	17.193	16.8400	16.8220	16.7190	
		0.2	17.53	17.0140	16.9800	16.8050	
	0.2	0.05	16.259	16.1540	16.1520	16.1380	16.006
		0.1	16.537	16.3080	16.3010	16.2700	
		0.15	16.843	16.4700	16.4520	16.3920	
		0.2	17.18	16.6380	16.6060	16.4820	

The Table 6 presents the effects of the imperfection (porosity) and the Winkler-Pasternak foundation on the frequency parameters " $\bar{\omega}$ " of the thin, moderately thick and thick square FG-plates with " $p=1$ ". It can be observed from the tabulated results that the increase in volume fraction porosity of the uneven and logarithmic-uneven porosities model has a slight effect on the values of frequency parameters " $\bar{\omega}$ " but the mass-density porosities model has a significant influence when the parameter of porosity " ξ " increases.

It can also be noted from the table that the frequency parameter $\bar{\omega}$ is in relation inverse with the porosity volume fraction " ξ " of even and mass-density porosities models but in the case of uneven and logarithmic-uneven porosities the frequency parameter $\bar{\omega}$ increase with increasing of " ξ " even it exceeds the frequency parameter of the perfect plate. It is concluded again that the higher values of the frequency $\bar{\omega}$ is obtained for the plates resting on elastic foundation with $(\bar{K}_w, \bar{G}_p = 100)$.

The variations of the frequency parameters $\bar{\omega}$ of perfect and imperfect FG square plates as function of the Kerr foundation stiffness is presented in the Table 7. The power law index is considered equal one " $p=1$ ", and the lower spring stiffness " $\bar{K}_l = 100$ ".

The current results are computed with various distributions of the porosities through the thickness of the plate (even, uneven, logarithmic uneven and mass density porosities). It can be noted from the obtained results that the increase in the Kerr foundation stiffness (\bar{K}_u, \bar{K}_s) leads to an increase in the frequency $\bar{\omega}$. For the even and masse density models, it is confirmed that the frequency parameter $\bar{\omega}$ decreases with increasing of the porosity volume fraction " ξ " but for uneven and logarithmic models this is reversed.

Fig. 3 illustrates the variation of the non-dimensional frequency " $\bar{\omega}$ " of the perfect and imperfect FG-plates resting on the elastic foundation versus the thickness ratio " a/h " and the spring constant " \bar{K}_w " with $(\bar{G}_p = 10 \text{ and } \xi = 0.05)$. It can be seen from the plotted

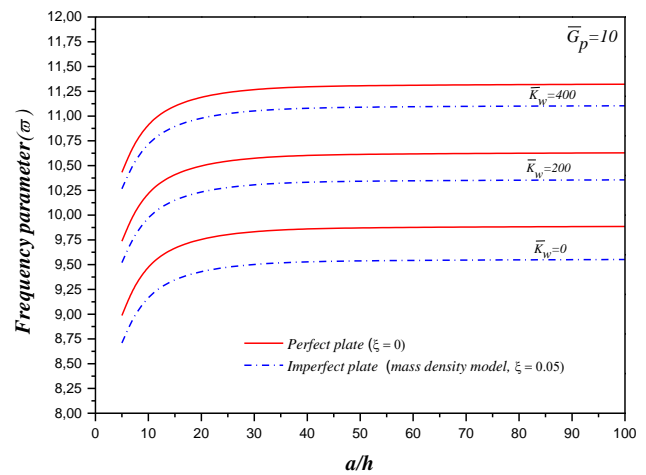


Fig. 3 The variation of the non-dimensional frequency " $\bar{\omega}$ " of the perfect and imperfect FG-plates versus the thickness ratio " a/h " and the spring constant " \bar{K}_w " with $(\bar{G}_p = 10, p = 1 \text{ and } \xi = 0.05)$

graphs that the non-dimensional frequency " $\bar{\omega}$ " increase with increasing of the geometry ratio " a/h " and values of the Winkler constant " \bar{K}_w ".

It is clear in the graphs that the existence of the porosity (mass density model) leads to a decrease in the values of the non-dimensional frequency " $\bar{\omega}$ ".

The effect of the shear layer parameters " \bar{G}_p " and the geometry ratio " a/h " on the non-dimensional frequency " $\bar{\omega}$ " of perfect and imperfect FG-plates is plotted in the Fig. 4. The value of the spring constant is considered " $\bar{K}_w = 100$ ". The plotted curves of imperfect FG-plate are computed via mass density model with " $\xi = 0.05$ ". From the graphs it can be noted that the increase of the shear layer parameters " \bar{G}_p " leads to an increase in the values of non-dimensional frequency " $\bar{\omega}$ ". For the great values of " \bar{G}_p ", it is remarkable that the frequency results of the perfect and imperfect plate converge.

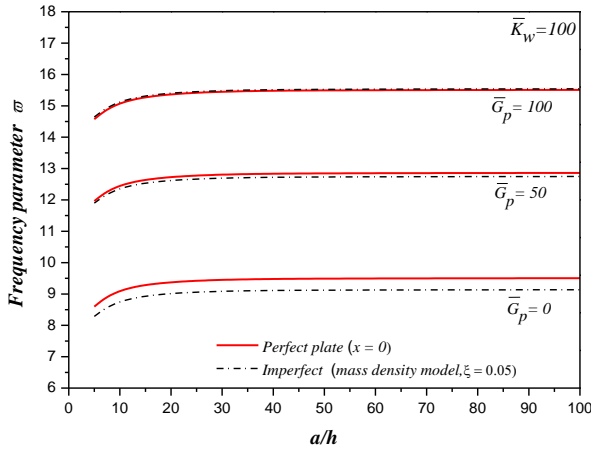


Fig. 4 The effect of the shear layer parameters " \bar{G}_p " and the geometry ratio " a/h " on the non-dimensional frequency " $\bar{\omega}$ " of perfect and imperfect FG-plates with ($\bar{K}_w=100$ and $p=1$)

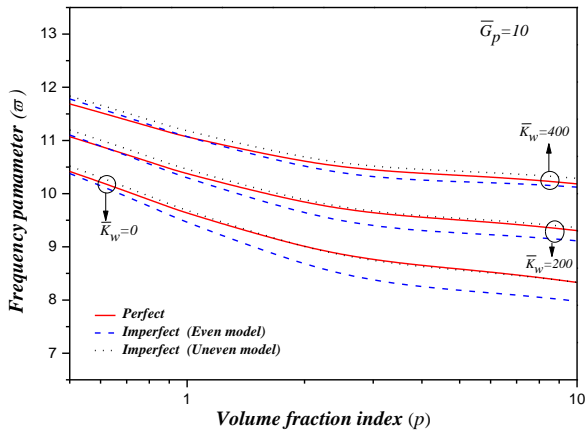


Fig. 5 The variation of the non-dimensional frequency " $\bar{\omega}$ " of perfect and imperfect plate versus the power index p and the spring constant " \bar{K}_w " with ($\bar{G}_p=10$, $a/h=10$ and $\zeta=0.1$)

The variation of the non-dimensional frequency " $\bar{\omega}$ " of perfect and imperfect plate versus the power index p and the spring constant " \bar{K}_w " is presented in Fig. 5. The shear layer parameters is taken " $\bar{G}_p=10$ " and porosity volume fraction " $\zeta=0.1$ ". It can be seen from the obtained curves that the values of the non-dimensional frequency " $\bar{\omega}$ " decrease with the increase of the power index " p ". The greater values of the frequency parameter are obtained for the FG-plate resting on elastic foundation with ($\bar{K}_w=400, \bar{G}_p=10$).

Fig. 6 shows the effect of the power index " p " and shear layer parameters " \bar{G}_p " on the non-dimensional frequency " $\bar{\omega}$ " of perfect and imperfect FG-plates with ($\bar{K}_w=100$ and $\zeta=0.1$). From the plotted graphs it can be concluded that the non-dimensional frequency " $\bar{\omega}$ " is in direct correlation relation with the shear layer parameters " \bar{G}_p " and in

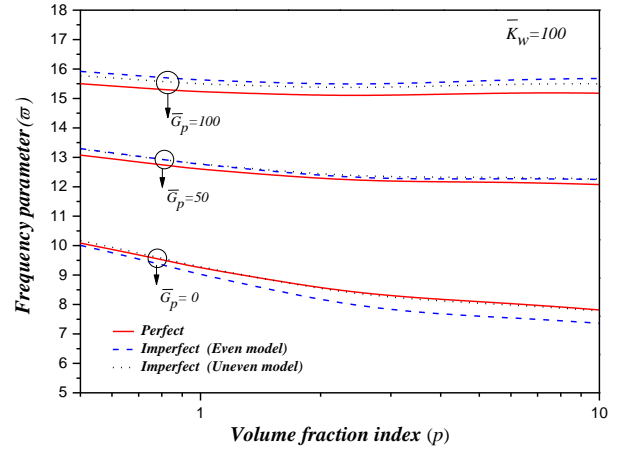


Fig. 6 The effect of the power index " p " and shear layer parameters " \bar{G}_p " on the non-dimensional frequency " $\bar{\omega}$ " of perfect and imperfect FG-plates with ($\bar{K}_w=100$, $a/h=10$ and $\zeta=0.1$)

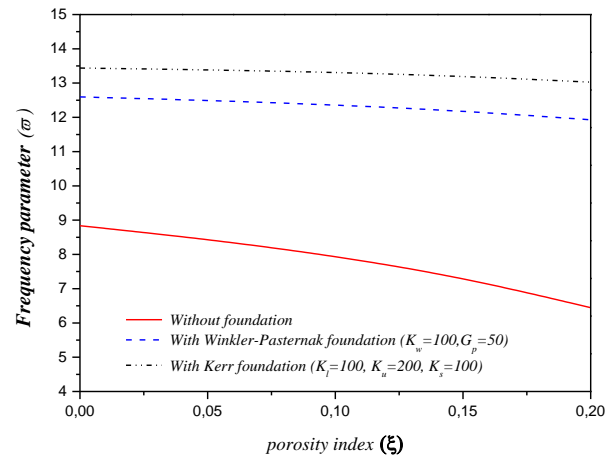


Fig. 7 The effect of the elastic foundation and the porosity index " ζ " on the frequency parameter " $\bar{\omega}$ " with ($a/h=10$ and $p=1$)

inverse relation with power index " p ".

Fig. 7 illustrates the effect of the elastic foundation and the porosity index " ζ " on the frequency parameter " $\bar{\omega}$ ". From the figure it can be seen that the elastic foundation has a significant effect on the frequency " $\bar{\omega}$ ". It is clear also that the presence of the upper spring in the elastic foundation (Kerr foundation) gives the greater values of frequency " $\bar{\omega}$ " and this is due that the rigidity of the FG-plate increase. We can also conclude that the frequency " $\bar{\omega}$ " decrease with increasing of the porosity volume fraction " ζ ".

4. Conclusions

In this paper, a hyperbolic four variables quasi-3D theory has been presented for dynamic behavior analysis of perfect and imperfect FG-plate resting on Winkler-Pasternak-Kerr elastic foundation. Four different models of

porosity distributions are considered for describing porosity effect in graded material characteristics. The equations of motion of the present problem are derived using the Hamilton's energy principle and solved via Navier solutions. The accuracy and efficiency of the present model are ascertained by comparing it with other theories, and excellent agreement was observed in all examples. We can finally conclude that the presence of the micro voids in the material has a significant effect on the frequency parameter of simply supported FG-plate. An improvement of the present formulation will be considered in the future work to consider other type of materials (Daouadji 2017, Klouche et al. 2017, Yeghnem et al. 2017, Karami et al. 2017, Khetir et al. 2017, Panjehpour et al. 2018, Behera and Kumari 2018, Shahadat et al. 2018, Karami et al. 2018d, Ayat et al. 2018, Kaci et al. 2018, Kadari et al. 2018, Cherif et al. 2018, Bouadi et al. 2018, Karami et al. 2018e, f, Yazid et al. 2018, Zine et al. 2018, Bensaid et al. 2018, Mokhtar et al. 2018, Hussain and Naeem 2019, Draiche et al. 2019, Rajabi and Mohammadimehr 2019, Fadoun 2019, Benmansour et al. 2019, Bensattalah et al. 2019, Berghouti et al. 2019, Medani et al. 2019, Boutaleb et al. 2019, Karami et al. 2019c, Hussain et al. 2019).

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